



**WHOLE HOUSE VENTILATION**

BEDROOMS	6
HEATED SQUARE FOOTAGE	5,713.6 SF
CFM = (0.01 * 5713.6 SF) + (7.5 * (5+1 BEDROOMS))	
AIRFLOW (CFM)	102.1 CFM MIN.

a. WHOLE HOUSE VENTILATION SHALL BE PROVIDED BY ERV/HRV W/ INTEGRAL FANS, PROVIDING MIN. 103 CFM RUNNING CONTINUOUSLY PER 2018 IRC TABLES M1505.4.3 (1&2). FAN SHALL BE LESS THAN .35 WATT PER CFM AND RUN CONTINUOUSLY, AND HAVE A SONE RATING OF LESS THAN 1.0. VENTILATION SHALL BE ABLE TO OPERATE INDEPENDENTLY OF HEATING SYSTEM.

b. SYSTEM SHALL HAVE A 9" SMOOTH FRESH AIR DUCT W/ LOUVER & SCREEN CONNECTED TO THE RETURN AIR STREAM 4' UPSTREAM OF THE AIR HANDLER AND INSULATED W/ R-4 MIN IN HEATED AREAS. ALL SUPPLY DUCTS IN CONDITIONED SPACE SHALL BE INSULATED TO MIN. R-4 PER IRC M1507.3.5.2.

c. SHALL HAVE A FILTER WITH A MERV OF AT LEAST 8 INSTALLED IN AN EASILY ACCESSIBLE LOCATION.

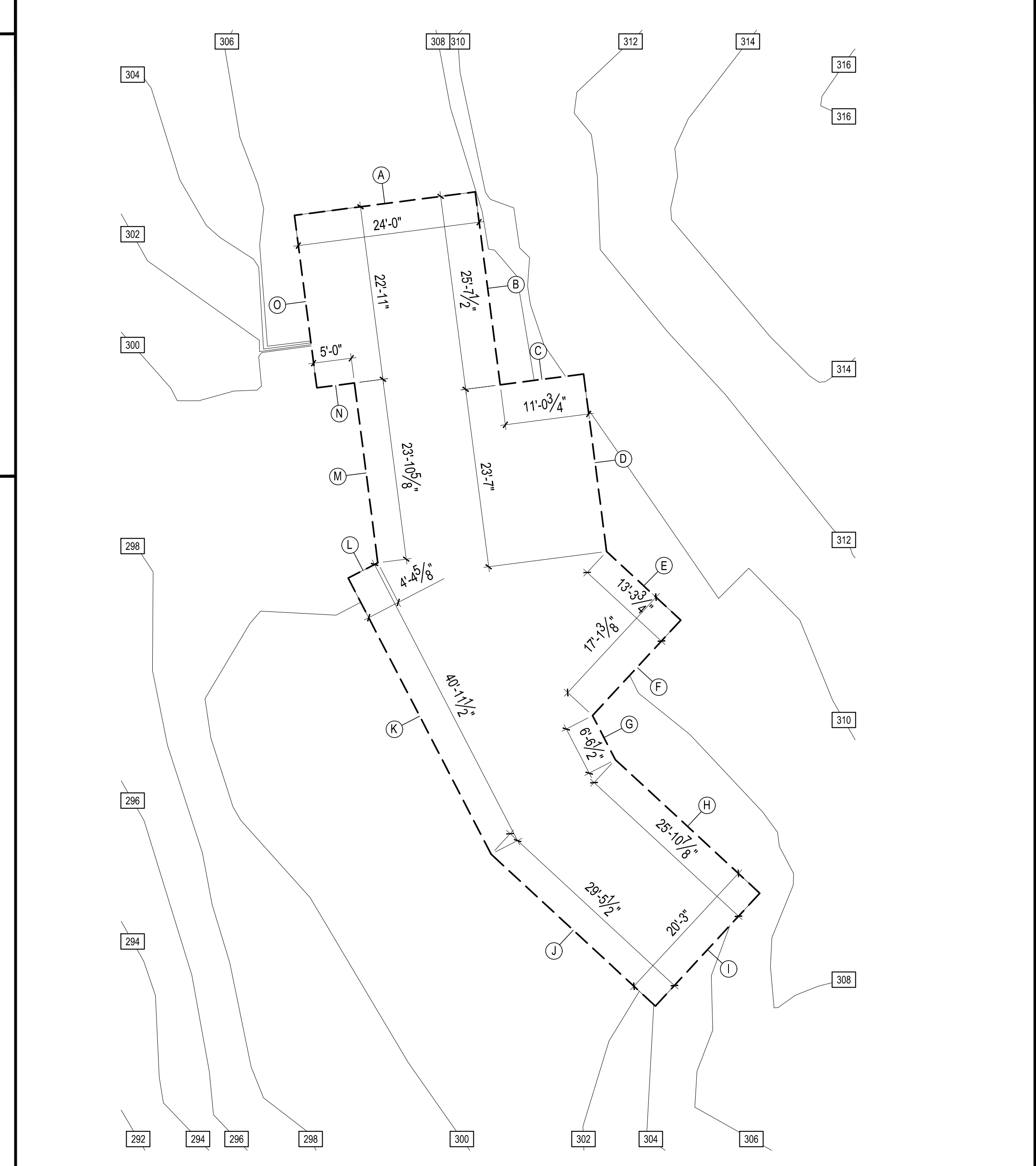
d. FRESH AIR VENT SHALL BE LOCATED AWAY FROM SOURCES OF ODORS OR FUMES, MIN. 10' FROM PLUMBING OR APPLIANCE VENTS, AWAY FROM ROOMS W/ FUEL BURNING APPLIANCES, AND OUT OF ATTICS, CRAWL SPACES, AND GARAGES.

e. AIRFLOW FOR WHOLE HOUSE VENTILATION FAN SHALL BE PROVIDED BY UNDERCUTTING INTERIOR DOORS 1/2" ABOVE FINISHED FLOOR, TYP.

f. WHOLE HOUSE VENTILATION SHALL BE TESTED, BALANCED AND VERIFIED AND A WRITTEN REPORT SHALL BE POSTED AND PROVIDED THE BUILDING OFFICIAL AND CERTIFICATION COMPLETED PER WSEC SECTIONS M1505.4.1.8 AND M1505.4.1.7.

g. AN EXHAUST FAN WHOLE HOUSE VENTILATION IS NOT ALLOWED WITH AN ERV SYSTEM.

h. HRV/ERV SHALL HAVE A MINIMUM HRE OF .80



**A.B.E. CALCULATIONS**

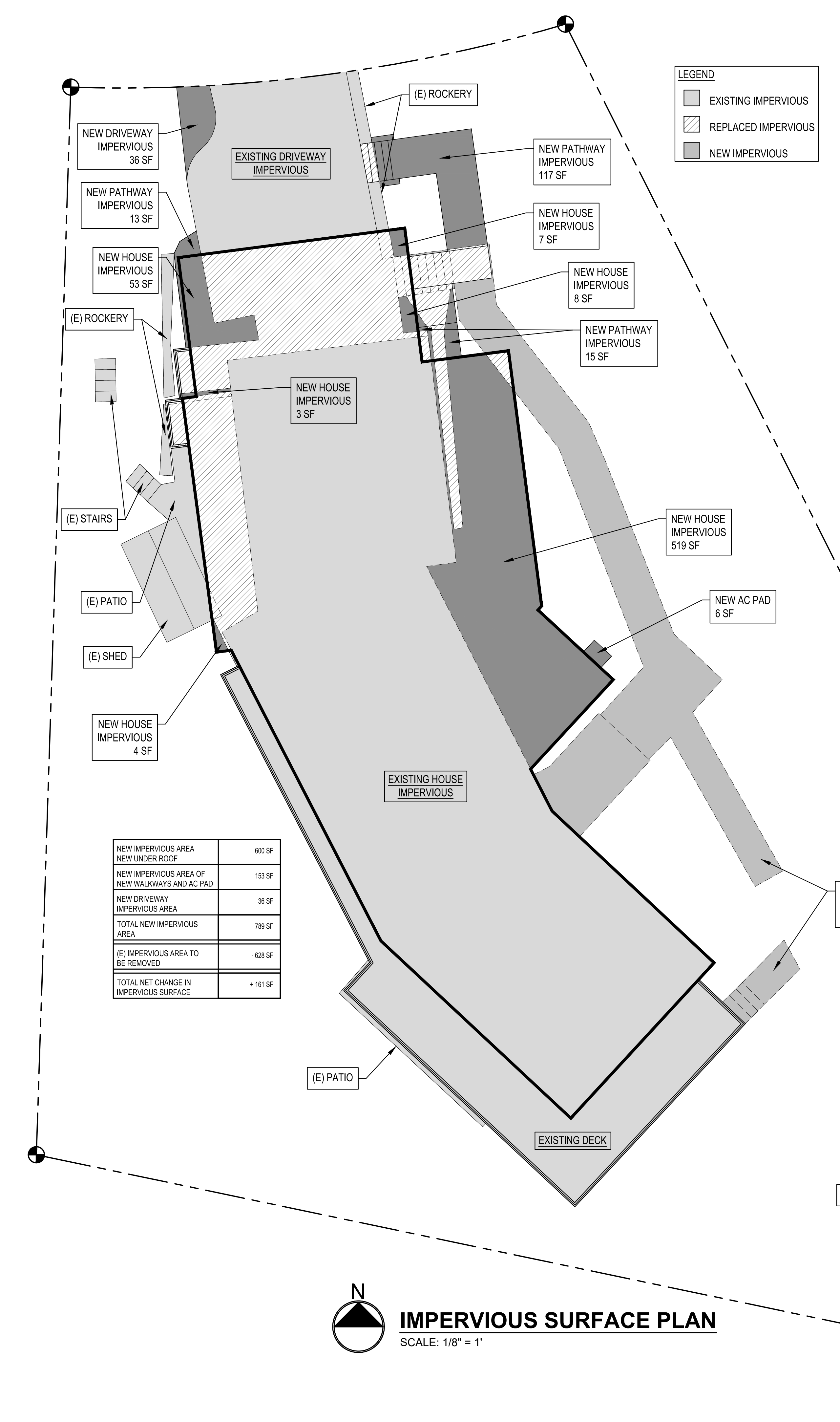
AVERAGE BUILDING ELEVATION			
	Wall Length	Elevation Pt.	Wall Length X Elev. Pt.
A	24.00	307.20	7372.80
B	25.60	307.80	7879.68
C	11.10	308.70	3426.57
D	23.60	309.10	7294.76
E	13.30	308.40	4101.72
F	17.10	308.20	5270.22
G	6.50	307.20	1996.80
H	25.90	307.20	7956.48
I	20.30	305.70	6205.71
J	29.50	301.80	8903.10
K	41.00	301.40	12357.40
L	4.40	300.00	1320.00
M	23.90	300.00	7170.00
N	5.00	300.00	1500.00
O	22.90	306.20	7011.98
<b>TOTAL</b>	<b>294.10</b>	<b>4578.90</b>	<b>89767.22</b>
89767.22	<b>305.23</b>	Average Building Elevation	
294.10			

**2018 WSEC CREDITS**

CREDITS REQUIRED:  
 ADDITIONS MORE THAN 1,500 SF BUT LESS THAN 5,000 SF.

2,425.5 SF NEW CONDITIONS SPACE 6.0 CREDITS  
 TOTAL CREDITS REQUIRED 6.0 CREDITS

CREDITS	OPTION	DESCRIPTION
1.0	1	HEAT PUMP
0.5	1.3	VERTICAL FENESTRATION U=28 FLOOR R-38 OR R-10 FOR SLAB ON GRADE
1.5	3.4	HIGH EFFICIENCY HVAC
0.5	4.1	HIGH EFFICIENCY HVAC DISTRIBUTION SYSTEM
2.0	5.5	EFFICIENT WATER HEATING
0.5	7.1	APPLIANCE PACKAGE
TOTAL CREDITS SELECTED		
6.0		



**LEGEND**

[Solid Grey]	EXISTING IMPERVIOUS
[Hatched]	REPLACED IMPERVIOUS
[Dark Grey]	NEW IMPERVIOUS

NEW IMPERVIOUS AREA NEW UNDER ROOF	600 SF
NEW IMPERVIOUS AREA OF NEW WALKWAYS AND AC PAD	153 SF
NEW DRIVEWAY IMPERVIOUS AREA	36 SF
TOTAL NEW IMPERVIOUS AREA	789 SF
(E) IMPERVIOUS AREA TO BE REMOVED	-628 SF
TOTAL NET CHANGE IN IMPERVIOUS SURFACE	+161 SF

**IMPERVIOUS SURFACE PLAN**  
 SCALE: 1/8" = 1'



### TREE PROTECTION AREA (TPZ)

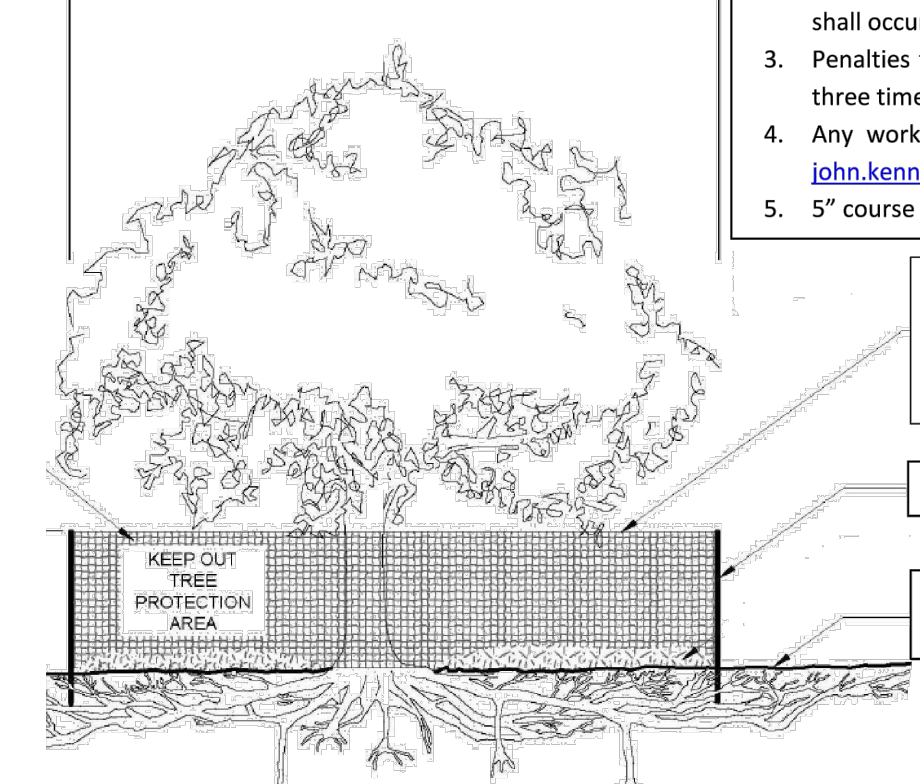
**KEEP OUT!**

**DO NOT REMOVE OR ADJUST THE APPROVED LOCATION OF THIS TREE PROTECTION AREA**

Trees enclosed by this fence are protected and are subject to the conditions of the tree permit. Violation of tree conditions may lead to:

1. Correction Notices or Stop Work Orders until compliance is achieved
2. RE Inspection Fees/financial penalties
3. Arborist reports recommending mitigation

Crown drip line or other limit of Tree Protection area. See Site/Utility Plan for fence alignment.



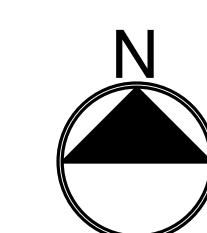
- Notes
1. No pruning shall be performed unless under the direction of the Project Arborist. Including limbing trees up.
  2. No grading, excavation, storage (materials, equipment, vehicles, etc.), or other unpermitted activity shall occur inside the protective fencing.
  3. Penalties for damaging by root damage/compaction or removing a saved tree may be a fine up to three times the value of the tree plus restoration (MICC 19.10.160).
  4. Any work in approved TPZ must be with the permission of the City Arborist (206) 275-7713, [john.kenney@mercergov.org](mailto:john.kenney@mercergov.org).
  5. 5" course woodchips within the tree protection zone, but not against the tree trunk.

Tree protection fence: 6' chain link fence, solidly anchored into the ground, or if authorized High-density polyethylene fencing with 3.5" x 1.5" openings; color orange. Steel posts installed at 8' o.c.

2" x 6" steel posts or approved equal

Maintain existing grade with the tree protection fence unless otherwise indication on the plans

Any Work in the protected area must be with the permission of the City Arborist [john.kenney@mercergov.org](mailto:john.kenney@mercergov.org)



### EXCAVATION PLAN

SCALE: 1/8" = 1'

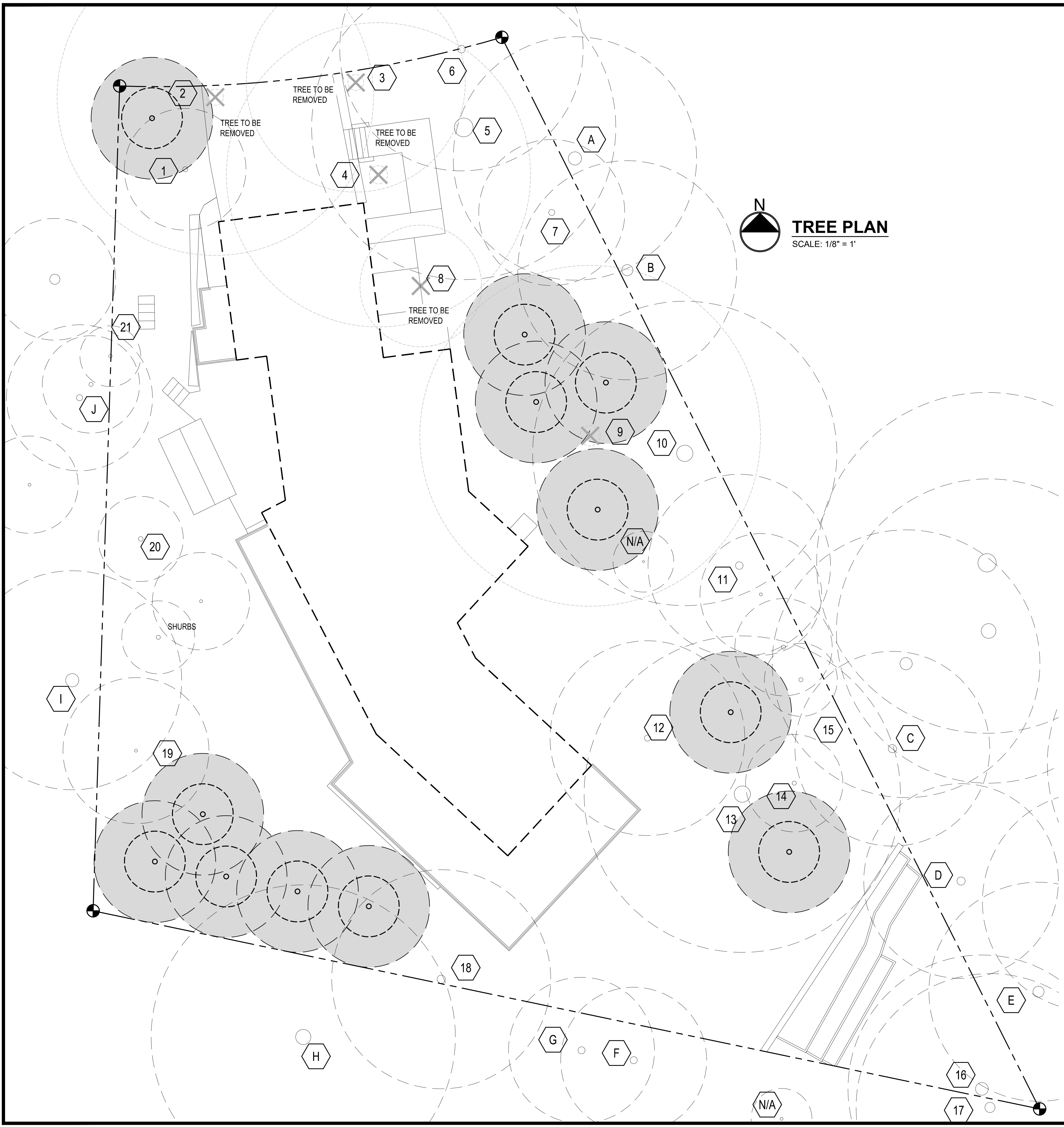
LEGEND	
	INNER TREE ROOT ZONE
	TREE ROOT ZONE INSIDE EXCAVATION
	EXCAVATION
	NEW WORK

# EXCAVATION PLAN

REVISIONS:	
▲ CORRECTION 1 - 6/25/2024	
▲	
▲	
▲	
▲	
PLOT DATE:	6/25/2024
DRAWN BY:	JM
CHECKED BY:	BJS
SHEET	

SCALE: IF SHEET IS LESS THAN 24" x 36" IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY  
 CORRECTION 1 SET 6/25/2024

**A1.2**



**TREE PLAN**  
SCALE: 1/8" = 1'

**PLAN NOTES**

**REPLACEMENT TREE**

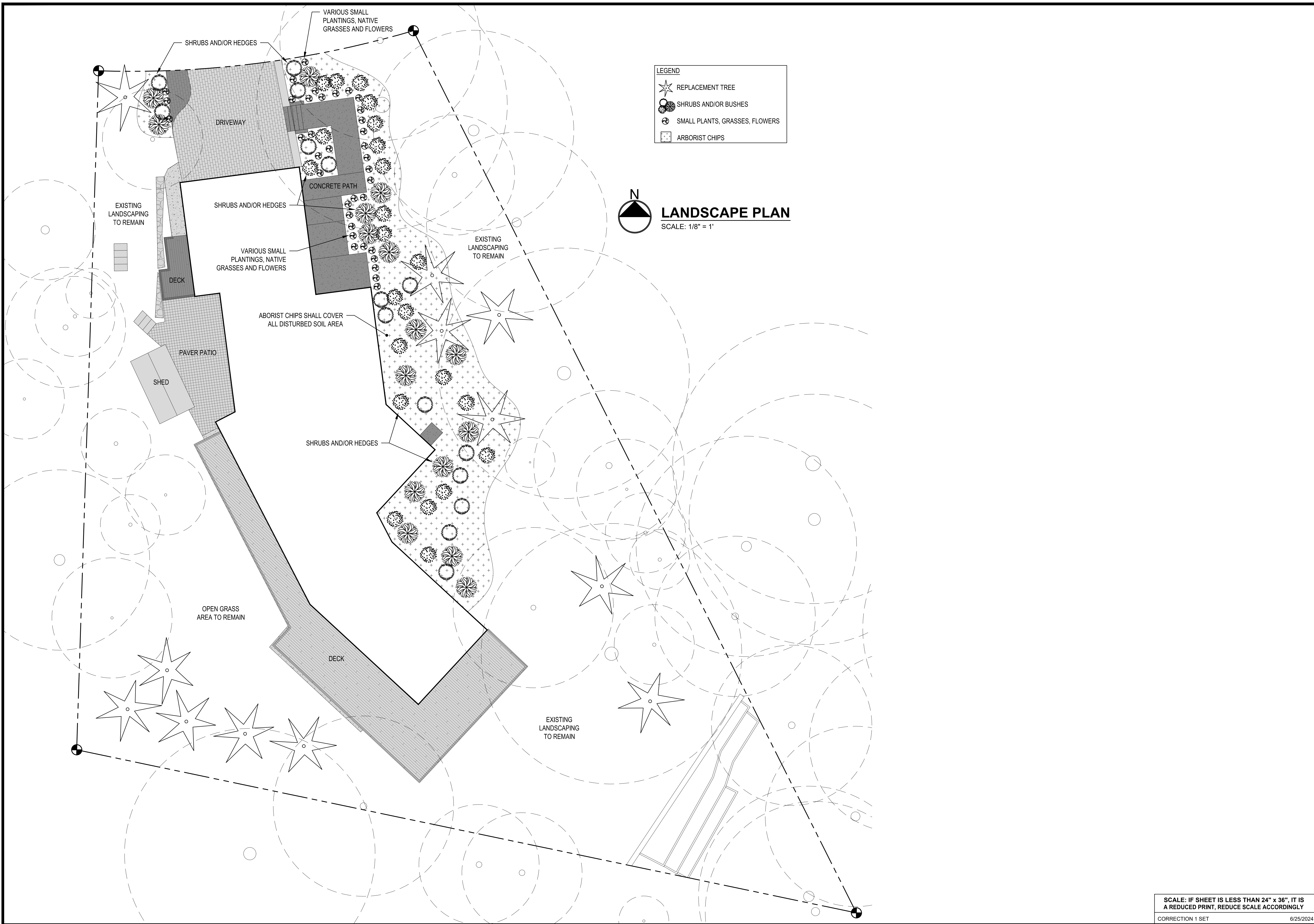
MIN 10'-0" RADIUS FROM ANY STRUCTURE, FENCE, UTILITIES, OR TREES

A TOTAL OF 12 REPLACEMENT TREES ARE REQUIRED.

HALF (6) OF THE REPLACEMENT TREES ARE REQUIRED TO BE PACIFIC NORTHWEST NATIVE TREE SPECIES.

DRIP IRRIGATION WILL BE PROVIDED AROUND ALL NEWLY PLANTED TREES. FLOW WILL ENSURE SATURATION OF ROOT ZONE. DURING DRY SEASON, JUNE 15-OCT 31, IRRIGATION WILL RUN 8 TIMES PER MONTH

REVISIONS:	
▲ CORRECTION 1 - 6/25/2024	
▲	
▲	
▲	
▲	
▲	
PLOT DATE:	6/25/2024
DRAWN BY:	JM
CHECKED BY:	BJS
SHEET	



**LEGEND**

- REPLACEMENT TREE
- SHRUBS AND/OR BUSHES
- SMALL PLANTS, GRASSES, FLOWERS
- ARBORIST CHIPS

**LANDSCAPE PLAN**  
SCALE: 1/8" = 1'

REVISIONS:	
△ CORRECTION 1 - 6/25/2024	
△	
△	
△	
△	
△	
PLOT DATE:	6/25/2024
DRAWN BY:	JM
CHECKED BY:	BJS
SHEET	

SCALE: IF SHEET IS LESS THAN 24" x 36" IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY  
CORRECTION 1 SET 6/25/2024

**A1.4**

**LEGAL DESCRIPTION**

(PER STATUTORY WARRANTY DEED RECORDING #20130730002745)  
 LOT 8, ISLAND POINT NO. 2, ACCORDING TO THE PLAT THEREOF,  
 RECORDED IN VOLUME 79 OF PLATS, PAGE(S) 18 AND 19, IN KING  
 COUNTY, WASHINGTON.

**BASIS OF BEARINGS**

N 83°59'24" E BETWEEN FOUND CENTERLINE MONUMENTATION  
 ALONG SE 82ND ST - CALCULATED PER R1

**REFERENCES**

R1. ISLAND POINT NO. 2 PLAT, VOL. 79, PG. 18,  
 RECORDS OF KING COUNTY, WASHINGTON.

**VERTICAL DATUM**

NAVD88 PER CITY OF MERCER ISLAND BENCHMARK #4360  
 DB ID: 47498  
 ELEV: 299.86

**SURVEYOR'S NOTES**

1. THE TOPOGRAPHIC SURVEY SHOWN HEREON WAS PERFORMED IN MARCH OF 2021. THE FIELD DATA WAS COLLECTED AND RECORDED ON MAGNETIC MEDIA THROUGH AN ELECTRONIC THEODOLITE. THE DATA FILE IS ARCHIVED ON DISC OR CD. WRITTEN FIELD NOTES MAY NOT EXIST. CONTOURS ARE SHOWN FOR CONVENIENCE ONLY. DESIGN SHOULD RELY ON SPOT ELEVATIONS.
2. ALL MONUMENTS SHOWN HEREON WERE LOCATED DURING THE COURSE OF THIS SURVEY UNLESS OTHERWISE NOTED.
3. THE TYPES AND LOCATIONS OF ANY UTILITIES SHOWN ON THIS DRAWING ARE BASED ON INFORMATION PROVIDED TO US, BY OTHERS OR GENERAL INFORMATION READILY AVAILABLE IN THE PUBLIC DOMAIN INCLUDING, AS APPLICABLE, IDENTIFYING MARKINGS PLACED BY UTILITY LOCATE SERVICES AND OBSERVED BY TERRANE IN THE FIELD. AS SUCH, THE UTILITY INFORMATION SHOWN ON THESE DRAWINGS ARE FOR INFORMATIONAL PURPOSES ONLY AND SHOULD NOT BE RELIED ON FOR DESIGN OR CONSTRUCTION PURPOSES; TERRANE IS NOT RESPONSIBLE OR LIABLE FOR THE ACCURACY OR COMPLETENESS OF THIS UTILITY INFORMATION. FOR THE ACCURATE LOCATION AND TYPE OF UTILITIES NECESSARY FOR DESIGN AND CONSTRUCTION, PLEASE CONTACT THE SITE OWNER AND THE LOCAL UTILITY LOCATE SERVICE (800-424-5555).
4. SUBJECT PROPERTY TAX PARCEL NO. 3625600080.
5. SUBJECT PROPERTY AREA PER THIS SURVEY IS 16,386± S.F. (0.38 ACRES)
6. THIS SURVEY WAS PERFORMED WITHOUT THE BENEFIT OF A TITLE REPORT. EASEMENTS AND OTHER ENCUMBRANCES MAY EXIST THAT ARE NOT SHOWN HEREON.
7. FIELD DATA FOR THIS SURVEY WAS OBTAINED BY DIRECT FIELD MEASUREMENTS WITH A CALIBRATED ELECTRONIC 5-SECOND TOTAL STATION AND/OR SURVEY GRADE GPS OBSERVATIONS. ALL ANGULAR AND LINEAR RELATIONSHIPS ARE ACCURATE AND MEET THE STANDARDS SET BY WAC 332-130-090.

**TOPOGRAPHIC & BOUNDARY SURVEY**

**LEGEND**

	AREA DRAIN		NAIL AS NOTED
	ASPHALT SURFACE		MONUMENT IN CASE (FOUND)
	BRICK SURFACE		PAVER SURFACE
	BUILDING		POWER METER
	CENTERLINE ROW		POWER (UNDERGROUND)
	CONCRETE SURFACE		REBAR AS NOTED (FOUND)
	RETAINING WALL		ROCKERY
	DECK		SEWER LINE
	FENCE LINE (CHAIN LINK)		SEWER MANHOLE
	FENCE LINE (WOOD)		STORM DRAIN LINE
	GAS LINE		TELEPHONE SENTRY
	GAS METER		TREE (AS NOTED)
	INLET (TYPE 1)		WATER LINE
	IRON PIPE (FOUND)		WATER METER
	LUMINAIRE		AC UNIT
	STEEP SLOPE AREA		

**VICINITY MAP**  
N.T.S.

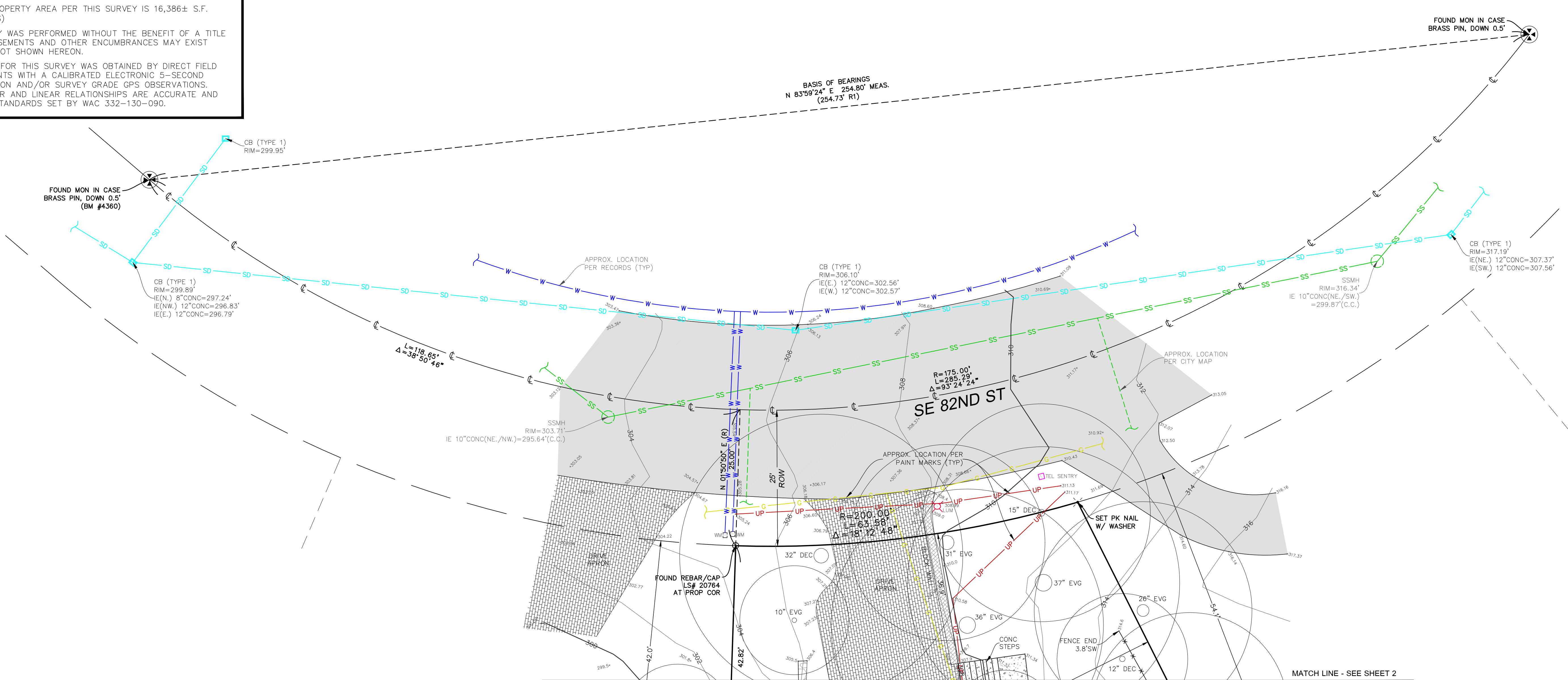
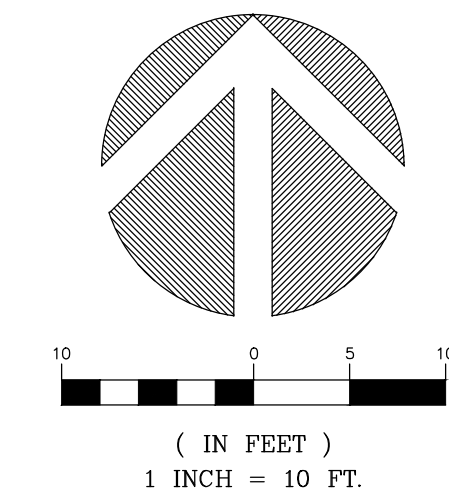


**STEEP SLOPE/BUFFER DISCLAIMER:**

THE LOCATION AND EXTENT OF STEEP SLOPES SHOWN ON THIS DRAWING ARE FOR INFORMATIONAL PURPOSES ONLY AND CANNOT BE RELIED ON FOR DESIGN AND/OR CONSTRUCTION. THE PITCH, LOCATION, AND EXTENT ARE BASED SOLELY ON OUR GENERAL OBSERVATIONS ON SITE AND OUR CURSORY REVIEW OF READILY AVAILABLE PUBLIC DOCUMENTS; AS SUCH, TERRANE CANNOT BE LIABLE OR RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF ANY STEEP SLOPE INFORMATION. ULTIMATELY, THE LIMITS AND EXTENT OF ANY STEEP SLOPES ASSOCIATED WITH ANY SETBACKS OR OTHER DESIGN OR CONSTRUCTION PARAMETERS MUST BE DISCUSSED AND APPROVED BY THE REVIEWING AGENCY BEFORE ANY CONSTRUCTION CAN OCCUR.

**INDEXING INFORMATION**

NW 1/4		NW 1/4	
SECTION: 31			
TOWNSHIP: 24N		RANGE: 05E, W.M.	
COUNTY: KING			



TOPOGRAPHIC & BOUNDARY SURVEY  
 PARCEL NO. 3625600080

RAWSON RESIDENCE  
 8413 SE 82ND ST  
 MERCER ISLAND, WA 98040



**Terrane**  
 10801 Main Street, Suite 102, Bellevue, WA 98004  
 phone 425.458.4488 support@terrane.net  
 www.terrane.net

JOB NUMBER: 210217  
 DATE: 03/11/21  
 DRAFTED BY: RSN  
 CHECKED BY: JGM/CSP  
 SCALE: 1" = 10'

**REVISION HISTORY**

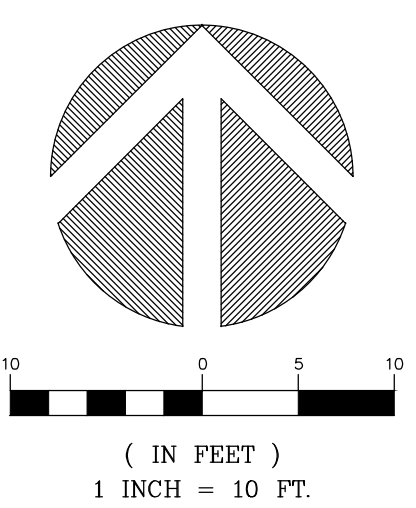
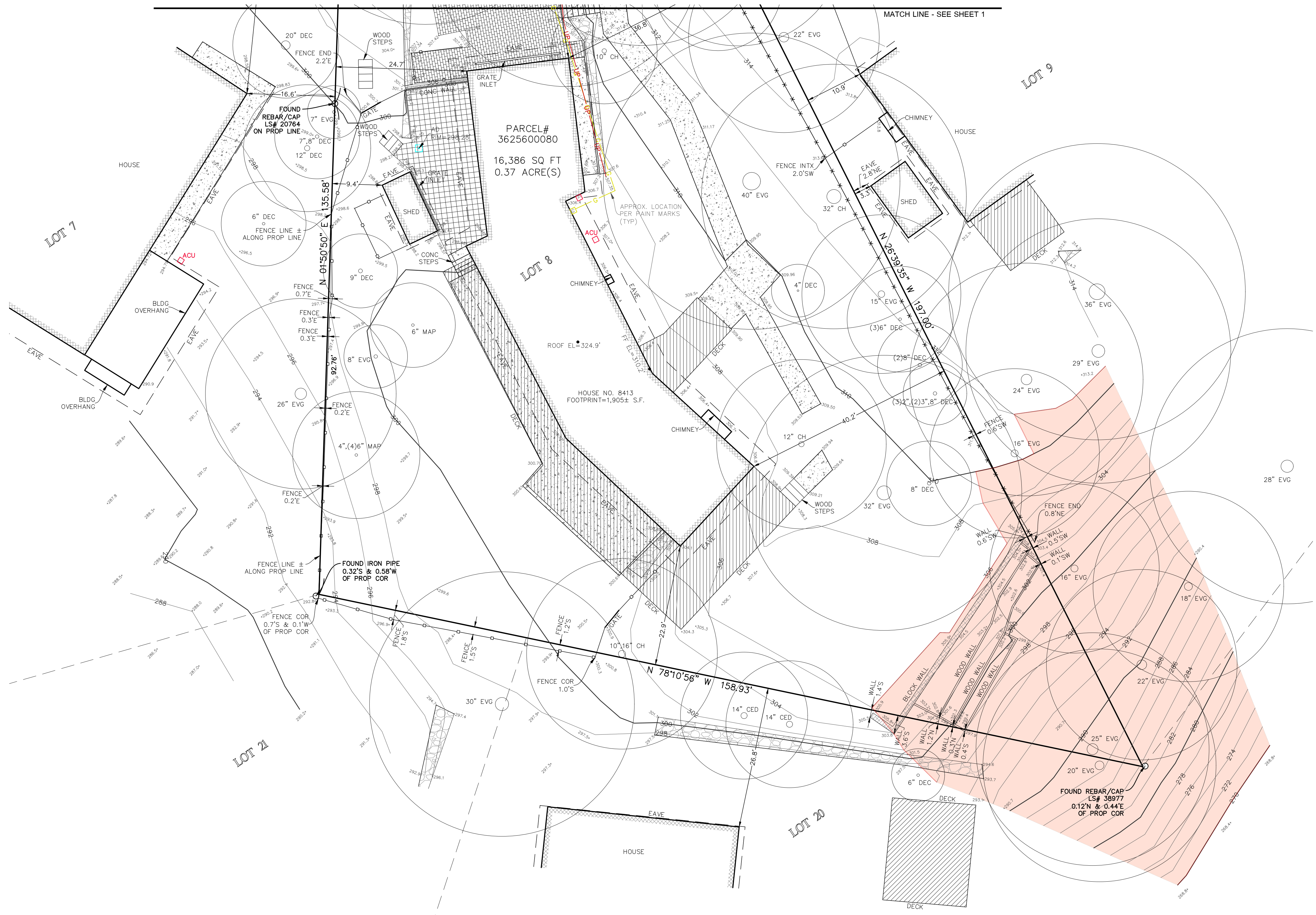

SHEET NUMBER  
 1 OF 2

# TOPOGRAPHIC & BOUNDARY SURVEY

## LEGEND

	AREA DRAIN		NAIL AS NOTED
	ASPHALT SURFACE		MONUMENT IN CASE (FOUND)
	BRICK SURFACE		PAVER SURFACE
	BUILDING		POWER METER
	CENTERLINE ROW		POWER (UNDERGROUND)
	CONCRETE SURFACE		REBAR AS NOTED (FOUND)
	RETAINING WALL		ROCKERY
	DECK		SEWER LINE
	FENCE LINE (CHAIN LINK)		SEWER MANHOLE
	FENCE LINE (WOOD)		STORM DRAIN LINE
	GAS LINE		TELEPHONE SENTRY
	GAS METER		TREE (AS NOTED)
	INLET (TYPE 1)		WATER LINE
	IRON PIPE (FOUND)		WATER METER
	LUMINAIRE		AC UNIT
	STEEP SLOPE AREA		

INDEXING INFORMATION	
	NW 1/4 NW 1/4
	SECTION: 31
	TOWNSHIP: 24N
	RANGE: 05E, W.M.
	COUNTY: KING



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measure success

TOPOGRAPHIC & BOUNDARY SURVEY

PARCEL NO. 3625600080

RAWSON RESIDENCE  
 8413 SE 82ND ST  
 MERCER ISLAND, WA 98040



**Terrane**  
 10801 Main Street, Suite 102, Bellevue, WA 98004  
 phone 425.458.4488 support@terrane.net  
[www.terrane.net](http://www.terrane.net)

JOB NUMBER:	210217
DATE:	03/11/21
DRAFTED BY:	RSN
CHECKED BY:	JGM/CSP
SCALE:	1"= 10'

REVISION HISTORY	

**EROSION AND SEDIMENTATION CONTROL GENERAL NOTES:**

- NOT USED
- NOT USED
- PERIMETER PROTECTION MAY BE USED AS THE SOLE FORM OR TREATMENT WHEN THE FLOWPATH MEETS THE CRITERIA LISTED BELOW. IF THESE ARE NOT MET, PERIMETER PROTECTION SHALL ONLY BE USED AS A BACKUP TO A SEDIMENT TRAP OR POND.

AVERAGE SLOPE	SLOPE PERCENT	FLOWPATH LENGTH
1.5H:1V OR LESS	67% OR LESS	100 FEET
2H:1V OR LESS	50% OR LESS	115 FEET
4H:1V OR LESS	25% OR LESS	150 FEET
6H:1V OR LESS	16.7% OR LESS	200 FEET
10H:1V OR LESS	10% OR LESS	250 FEET

- THE CONTRACTOR SHALL STABILIZE DENUDED AREAS AND SOIL STOCKPILES AS FOLLOWS:  
DENUDED AREAS SHALL BE COVERED BY MULCH, SOD, PLASTIC, OR OTHER BMP'S APPROVED BY THE ENGINEER. WHERE POSSIBLE NATURAL VEGETATION SHALL BE MAINTAINED FOR EROSION AND SEDIMENT CONTROL.
- AS CONSTRUCTION PROGRESSES AND SEASONAL CONDITIONS DICTATE, THE EROSION CONTROL FACILITIES SHALL BE MAINTAINED AND/OR ALTERED AS REQUIRED TO ENSURE CONTINUING EROSION/SEDIMENT CONTROL.
- EVERY EFFORT SHALL BE MADE TO CLOSE UTILITY TRENCHES BY THE END OF THE DAY AND MATERIAL EXCAVATED DURING UNDERGROUND UTILITY CONSTRUCTION SHALL BE PLACED ON THE UPHILL SIDE OF TRENCHES (WHERE CONSISTENT WITH SAFETY AND SPACE CONSIDERATIONS).
- ALL TEMPORARY EROSION AND SEDIMENT CONTROL BMP'S SHALL BE MAINTAINED IN A SATISFACTORY CONDITION UNTIL SUCH TIME THAT CLEARING AND/OR CONSTRUCTION IS COMPLETED, PERMANENT DRAINAGE FACILITIES ARE IN OPERATION, AND THE POTENTIAL FOR EROSION HAS PASSED.
- AT A MINIMUM, EROSION AND SEDIMENT CONTROL FACILITIES SHALL BE MAINTAINED MONTHLY, OR FOLLOWING EACH RUNOFF-PRODUCING STORM, TO ENSURE PROPER OPERATION OF ALL EROSION AND SEDIMENT CONTROL FACILITIES. SEDIMENT SHALL BE REMOVED FROM BMP'S WHEN IT REACHES D-FOOT DEPTH.
- THE PUBLIC RIGHT-OF-WAY SHALL BE KEPT CLEAN. TRACKING OF MUD AND DEBRIS FROM THE SITE WILL NOT BE ALLOWED. FAILURE TO COMPLY WITH THIS CONDITION MAY RESULT IN ALL WORK ON SITE BEING STOPPED.
- THE WASHINGTON STATE CLEAN AIR ACT REQUIRES THE USE OF ALL KNOWN AVAILABLE, AND REASONABLE MEANS OF CONTROLLING AIR POLLUTION, INCLUDING DUST. DUST CAN BE CONTROLLED BY WETTING EXPOSED SOILS, WASHING TRUCK WHEELS BEFORE THEY LEAVE THE SITE, AND INSTALLING AND MAINTAINING ROCK CONSTRUCTION ENTRANCES. CONSTRUCTION VEHICLE TRACK-OUT IS A MAJOR SOURCE OF DUST AND ANY EVIDENCE OF TRACK-OUT CAN TRIGGER FINES FROM THE DEPARTMENT OF ECOLOGY OF THE PUGET SOUND AIR POLLUTION CONTROL AGENCY.
- NOT USED
- THE CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION AND SEDIMENTATION CONTROL BMP'S WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION OR AFTER THEY ARE NO LONGER NECESSARY.

**PRIOR TO BEGINNING CLEARING OR GRADING**

- INSTALL THE SLIT FENCE AS INDICATED ON THE SITE PLAN & SHEET C1.0
- PLACE A THICK LAYER OF STRAW OR MULCH ON ALL AREAS OF BARE SOIL OUTSIDE OF THE PLANNED NEW CONSTRUCTION. THIS IS PARTICULARLY IMPORTANT IN THE SOUTH, LOW END OF THE LOT.
- INSTALL PRE MANUFACTURED SILT SOCKS IN THE TWO EXISTING CATCH BASINS LOCATED SOUTH & EAST OF THE SITE. THIS CATCH BASIN PROTECTION MUST BE CHECKED PERIODICALLY, & CLEANED AS NECESSARY, TO PREVENT THE SILT SOCKS FROM BECOMING OVERLOADED WITH SILT & DEBRIS FROM SURFACE RUNOFF.
- CONSTRUCT A STABILIZED CONSTRUCTION ENTRANCE, AS SHOWN ON SHEET C1.0 OF THE DRAWINGS, WHEREVER TRUCKS WILL DRIVE OFF AF PAVED SURFACES TO IMPORT OR EXPORT DEBRIS & SOIL.

**DURING GRADING & CONSTRUCTION**

- COVER ANY SOIL STOCKPILES WITH PLASTIC SHEETING THAT IS STAKED OR WEIGHTED TO PREVENT IT FROM BLOWING AWAY.
- ALLOW NO RUNOFF FROM THE EXCAVATION FOR THE SOUTHERN ADDITION TO FLOW ACROSS THE GROUND SURFACE TOWARD THE SOUTH. THIS MAY REQUIRE CREATING A SOIL BERM ALONG THE SOUTHERN EDGE OF THE EXCAVATION. IF SILTY RUNOFF COLLECTS IN THE EXCAVATION, IT MAY NEED TO BE PUMPED TO A TEMPORARY HOLDING TANK FOR DISPOSAL OFF SITE.
- FOLLOWING CONSTRUCTION OF THE FOUNDATION WALLS, PROCEED IMMEDIATELY WITH INSTALLATION OF DRAINAGE & WATER PROOFING, THEN COMPLETION OF BACKFILLING.
- SPREAD STRAW OR MULCH AGAIN ON ALL BARE SOIL OUTSIDE OF THE BACKFILLED FOUNDATIONS, UNLESS PERMANENT LANDSCAPING & VEGETATION WILL BE IMMEDIATELY ESTABLISHED.

**COVER MEASURES**

COVER METHODS INCLUDE THE USE OF MULCH, EROSION CONTROL NETS AND BLANKETS, PLASTIC COVERING, SEEDING, AND SODDING. MULCH AND PLASTIC SHEETING ARE PRIMARILY INTENDED TO PROTECT DISTURBED AREAS FOR A SHORT PERIOD OF TIME, TYPICALLY DAYS TO A FEW MONTHS. SEEDING AND SODDING ARE MEASURES FOR AREAS THAT ARE TO REMAIN UNWORKED FOR MONTHS.

TEMPORARY EROSION CONTROL SEED MIX:	% WEIGHT	% PURITY	% GERMINATION
ANNUAL OR PERENNIAL RYE (LOLIUM MULTIFLORUM OR LOLIUM PERENNE)	40	98	90
REDFTOP OR COLONIAL BENTGRASS (AGROSTIS ALBA OR AGROSTIS TENUIIS)	10	92	85

PERMANENT SEED MIX:	% WEIGHT	% PURITY	% GERMINATION	REMARKS
PERENNIAL RYE BLEND (LOLIUM PERENNE)	70	98	90	THIS MIX IS PROVIDED AS JUST ONE RECOMMENDED POSSIBILITY. LOCAL SUPPLIERS SHOULD BE CONSULTED FOR THEIR RECOMMENDATIONS BECAUSE THE APPROPRIATE MIX DEPENDS ON A VARIETY OF FACTORS, INCLUDING EXPOSURE, SOIL TYPE, SLOPE, AND EXPECTED FOOT TRAFFIC.
CHEWINGS AND RED FESCUE BLEND (FESTUCA RUBRA VAR. COMMUTATA OR FESTUCA RUBRA)	30	98	90	

MULCH STANDARDS AND GUIDELINES:			
MULCH MATERIAL	QUALITY STANDARDS	APPLICATION RATES	REMARKS
STRAW	AIR-DRIED; FREE FROM UNDESIRABLE SEED AND COARSE MATERIAL.	2"-3" THICK; 2-3 BALES PER 1000 SF OR 2-3 TONS PER ACRE	COST-EFFECTIVE PROTECTION WHEN APPLIED WITH ADEQUATE THICKNESS. HAND-APPLICATION GENERALLY REQUIRES GREATER THICKNESS THAN BLOWN STRAW. STRAW SHOULD BE CRIMPED TO AVOID WIND BLOW. THE THICKNESS OF STRAW MAY BE REDUCED BY HALF WHEN USED IN CONJUNCTION WITH SEEDING.
CHIPPED SITE VEGETATION	AVERAGE SIZE SHALL BE SEVERAL INCHES.	2" MINIMUM THICKNESS	THIS IS A COST-EFFECTIVE WAY TO DISPOSE OF DEBRIS FROM CLEARING AND GRUBBING, AND IT ELIMINATES THE PROBLEMS ASSOCIATED WITH BURNING. GENERALLY, IT SHOULD NOT BE USED ON SLOPES ABOVE APPROXIMATELY 10% BECAUSE OF ITS TENDENCY TO BE TRANSPORTED BY RUNOFF. IT IS NOT RECOMMENDED WITHIN 200 FEET OF SURFACE WATERS. IF SEEDING IS EXPECTED SHORTLY AFTER MULCH, THE DECOMPOSITION OF THE CHIPPED VEGETATION MAY TIE UP NUTRIENTS IMPORTANT TO GRASS ESTABLISHMENT.

**CONSTRUCTION SEQUENCE:**

- SCHEDULE THE PRE-CONSTRUCTION MEETING.
- FLAG OR FENCE ALL CRITICAL AREAS AND CLEARING LIMITS.
- POST A SIGN WITH THE NAME AND PHONE NUMBER OF THE E.S.C. SUPERVISOR.
- GRADE AND INSTALL CONSTRUCTION ENTRANCE(S).
- INSTALL PERIMETER PROTECTION (SILT FENCE, BRUSH BARRIER, ETC.).
- CONSTRUCT SEDIMENT PONDS AND TRAPS, IF REQUIRED.
- GRADE AND STABILIZE CONSTRUCTION ROADS.
- CONSTRUCT SURFACE WATER CONTROLS (INTERCEPTOR DIKES, PIPE SLOPE DRAINS, ETC.) SIMULTANEOUSLY WITH CLEARING AND GRADING FOR PROJECT DEVELOPMENT.
- INSTALL UTILITIES.
- MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH LOCAL STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.
- RELOCATE SURFACE WATER CONTROLS OR EROSION CONTROL MEASURES, OR INSTALL NEW MEASURES SO THAT AS SITE CONDITIONS CHANGE, THE EROSION AND SEDIMENT CONTROL IS ALWAYS IN ACCORDANCE WITH THE ACCEPTED STANDARD BMP'S.
- COVER ALL AREAS THAT WILL BE UNWORKED FOR MORE THAN SEVEN DAYS DURING THE DRY SEASON OR TWO DAYS DURING THE WET SEASON WITH STRAW, WOOD FIBER MULCH, COMPOST, PLASTIC SHEETING, OR EQUIVALENT.
- STABILIZE ALL AREAS WITHIN SEVEN DAYS OF REACHING FINAL GRADE.
- SEED OR SOD ANY AREAS OF THE PROJECT, STABILIZE ALL DISTURBED AREA AND REMOVE BMP'S IFF APPROPRIATE
- UPON COMPLETION OF THE PROJECT, STABILIZE ALL DISTURBED AREAS AND REMOVE BMP'S IF APPROPRIATE.

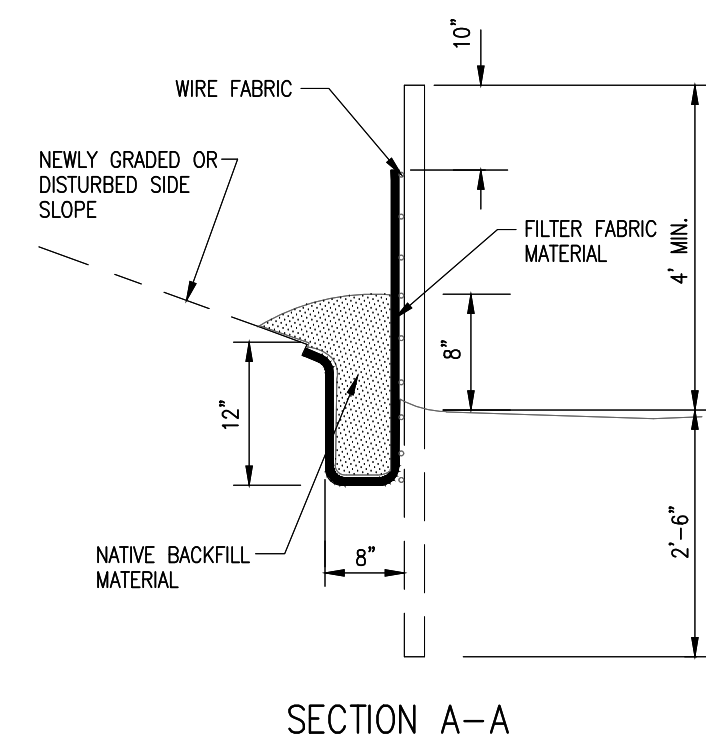
**COVER MEASURES**

COVER METHODS INCLUDE THE USE OF MULCH, EROSION CONTROL NETS AND BLANKETS, PLASTIC COVERING, SEEDING, AND SODDING. MULCH AND PLASTIC SHEETING ARE PRIMARILY INTENDED TO PROTECT DISTURBED AREAS FOR A SHORT PERIOD OF TIME, TYPICALLY DAYS TO A FEW MONTHS. SEEDING AND SODDING ARE MEASURES FOR AREAS THAT ARE TO REMAIN UNWORKED FOR MONTHS.

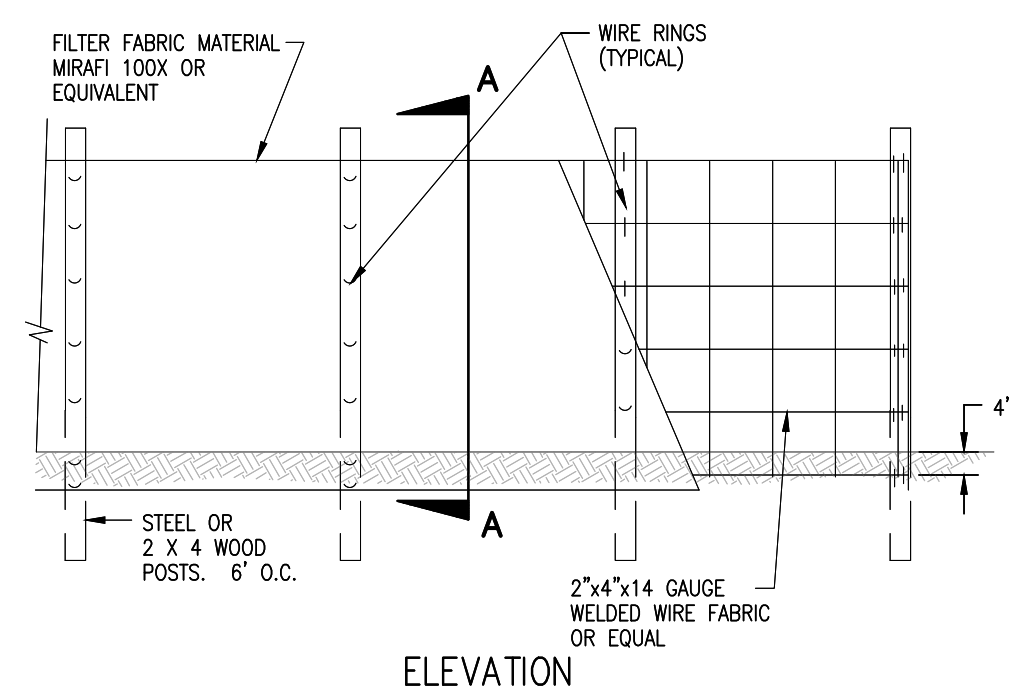
TEMPORARY EROSION CONTROL SEED MIX:	% WEIGHT	% PURITY	% GERMINATION
ANNUAL OR PERENNIAL RYE (LOLIUM MULTIFLORUM OR LOLIUM PERENNE)	40	98	90
REDFTOP OR COLONIAL BENTGRASS (AGROSTIS ALBA OR AGROSTIS TENUIIS)	10	92	85

PERMANENT SEED MIX:	% WEIGHT	% PURITY	% GERMINATION	REMARKS
PERENNIAL RYE BLEND (LOLIUM PERENNE)	70	98	90	THIS MIX IS PROVIDED AS JUST ONE RECOMMENDED POSSIBILITY. LOCAL SUPPLIERS SHOULD BE CONSULTED FOR THEIR RECOMMENDATIONS BECAUSE THE APPROPRIATE MIX DEPENDS ON A VARIETY OF FACTORS, INCLUDING EXPOSURE, SOIL TYPE, SLOPE, AND EXPECTED FOOT TRAFFIC.
CHEWINGS AND RED FESCUE BLEND (FESTUCA RUBRA VAR. COMMUTATA OR FESTUCA RUBRA)	30	98	90	

MULCH STANDARDS AND GUIDELINES:			
MULCH MATERIAL	QUALITY STANDARDS	APPLICATION RATES	REMARKS
STRAW	AIR-DRIED; FREE FROM UNDESIRABLE SEED AND COARSE MATERIAL.	2"-3" THICK; 2-3 BALES PER 1000 SF OR 2-3 TONS PER ACRE	COST-EFFECTIVE PROTECTION WHEN APPLIED WITH ADEQUATE THICKNESS. HAND-APPLICATION GENERALLY REQUIRES GREATER THICKNESS THAN BLOWN STRAW. STRAW SHOULD BE CRIMPED TO AVOID WIND BLOW. THE THICKNESS OF STRAW MAY BE REDUCED BY HALF WHEN USED IN CONJUNCTION WITH SEEDING.
CHIPPED SITE VEGETATION	AVERAGE SIZE SHALL BE SEVERAL INCHES.	2" MINIMUM THICKNESS	THIS IS A COST-EFFECTIVE WAY TO DISPOSE OF DEBRIS FROM CLEARING AND GRUBBING, AND IT ELIMINATES THE PROBLEMS ASSOCIATED WITH BURNING. GENERALLY, IT SHOULD NOT BE USED ON SLOPES ABOVE APPROXIMATELY 10% BECAUSE OF ITS TENDENCY TO BE TRANSPORTED BY RUNOFF. IT IS NOT RECOMMENDED WITHIN 200 FEET OF SURFACE WATERS. IF SEEDING IS EXPECTED SHORTLY AFTER MULCH, THE DECOMPOSITION OF THE CHIPPED VEGETATION MAY TIE UP NUTRIENTS IMPORTANT TO GRASS ESTABLISHMENT.

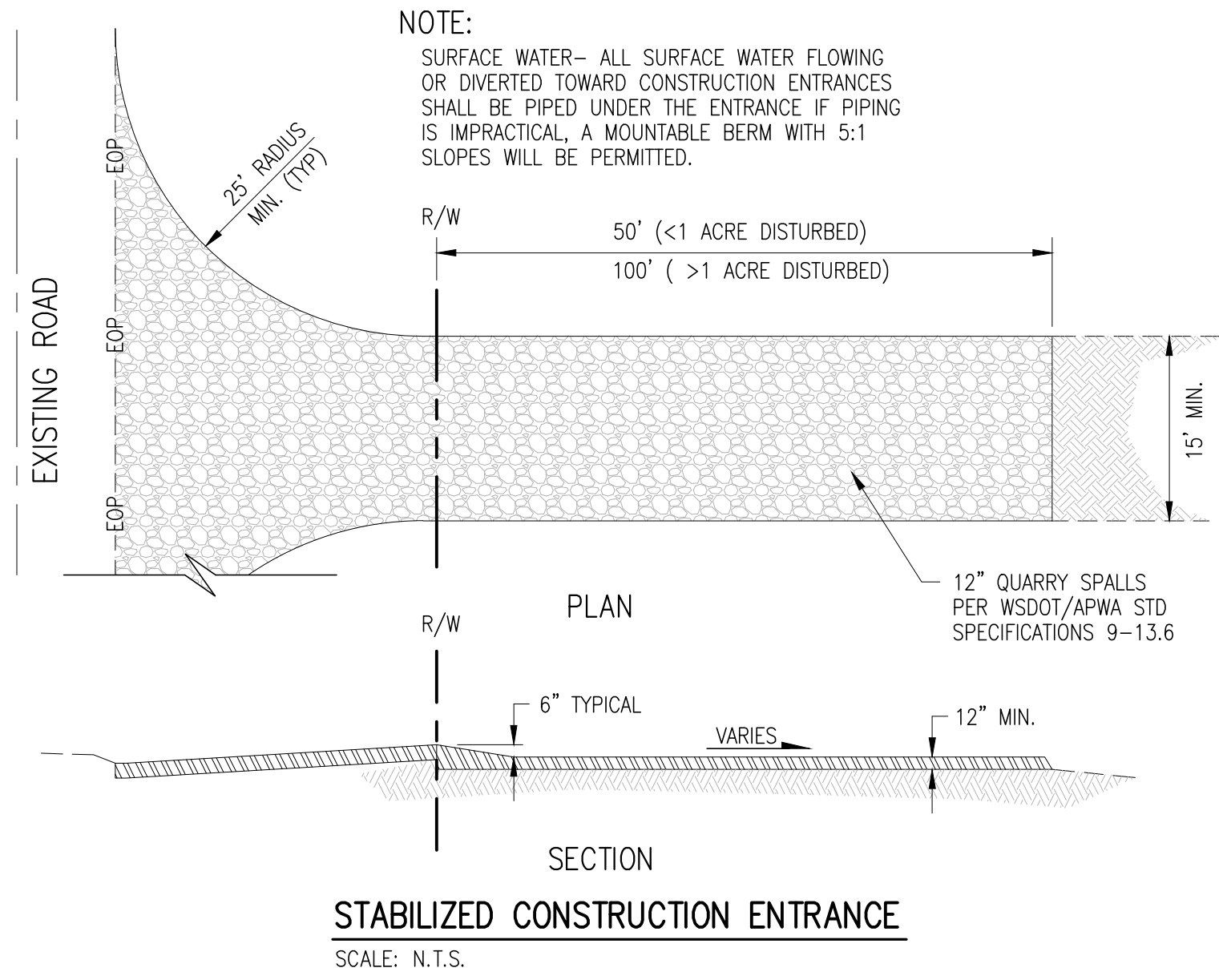


- NOTES:**
- INSTALL THE SILT FENCE FIRST. AFTER THE SILT FENCE HAS BEEN INSTALLED, CONSTRUCT BERM AND TRENCH.
  - THE FILTER FABRIC SHALL BE PURCHASED IN A CONTINUOUS ROLL OUT TO THE LENGTH OF THE BARRIER TO AVOID THE USE OF JOINTS. WHEN JOINTS ARE NECESSARY, THE FILTER FABRIC SHALL BE SPICED TOGETHER ONLY AT A SUPPORT POST, WITH A MINIMUM 6 INCH OVERLAP, AND BOTH ENDS SHALL BE SECURELY FASTENED TO THE POST.
  - POSTS SHALL BE SPACED A MAXIMUM OF 6 FEET APART AND DRIVEN SECURELY INTO THE GROUND A MINIMUM OF 30 INCHES (WHERE PHYSICALLY POSSIBLE).
  - A TRENCH SHALL BE EXCAVATED APPROXIMATELY 8 INCHES WIDE AND 12 INCHES DEEP ALONG THE LINE OF POSTS AND UPSLOPE FROM THE BARRIER. THE TRENCH SHALL BE CONSTRUCTED TO FOLLOW THE CONTOUR. 20 INCHES OF THE FILTER FABRIC SHALL EXTEND INTO THE TRENCH AND THE TRENCH SHALL BE BACKFILLED WITH COMPACTED NATIVE SOIL MATERIAL.
  - THE FILTER FABRIC FENCE SHALL NOT EXTEND MORE THAN 36 INCHES ABOVE THE ORIGINAL GROUND SURFACE. FILTER FABRIC SHALL NOT BE STAPLED TO EXISTING TREES.
  - WHEN SILT FILM FILTER FABRIC IS USED, A WIRE MESH SUPPORT FENCE SHALL DUTY WIRE STAPLES AT LEAST 1 INCH LONG, THE WIRES, OR HOG RINGS. THE WIRE SHALL EXTEND INTO THE TRENCH A MINIMUM OF 4 INCHES. SILT FILM FILTER SHALL BE WIRED TO THE FENCE.
  - WHEN EXTRA-STRENGTH OR MONOFILAMENT FABRIC IS USED, THE WIRE MESH SUPPORT FENCE MAY BE ELIMINATED. IN THIS CASE, THE FILTER FABRIC IS STAPLED OR WIRED DIRECTLY TO THE POSTS. EXTRA CARE SHOULD BE USED WHEN JOINING OR OVERLAPPING THESE STIFFER FABRICS.



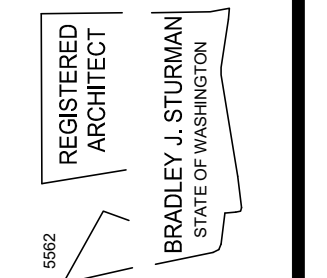
**SILT FENCE DETAIL**

SCALE: N.T.S.



**STABILIZED CONSTRUCTION ENTRANCE**

SCALE: N.T.S.



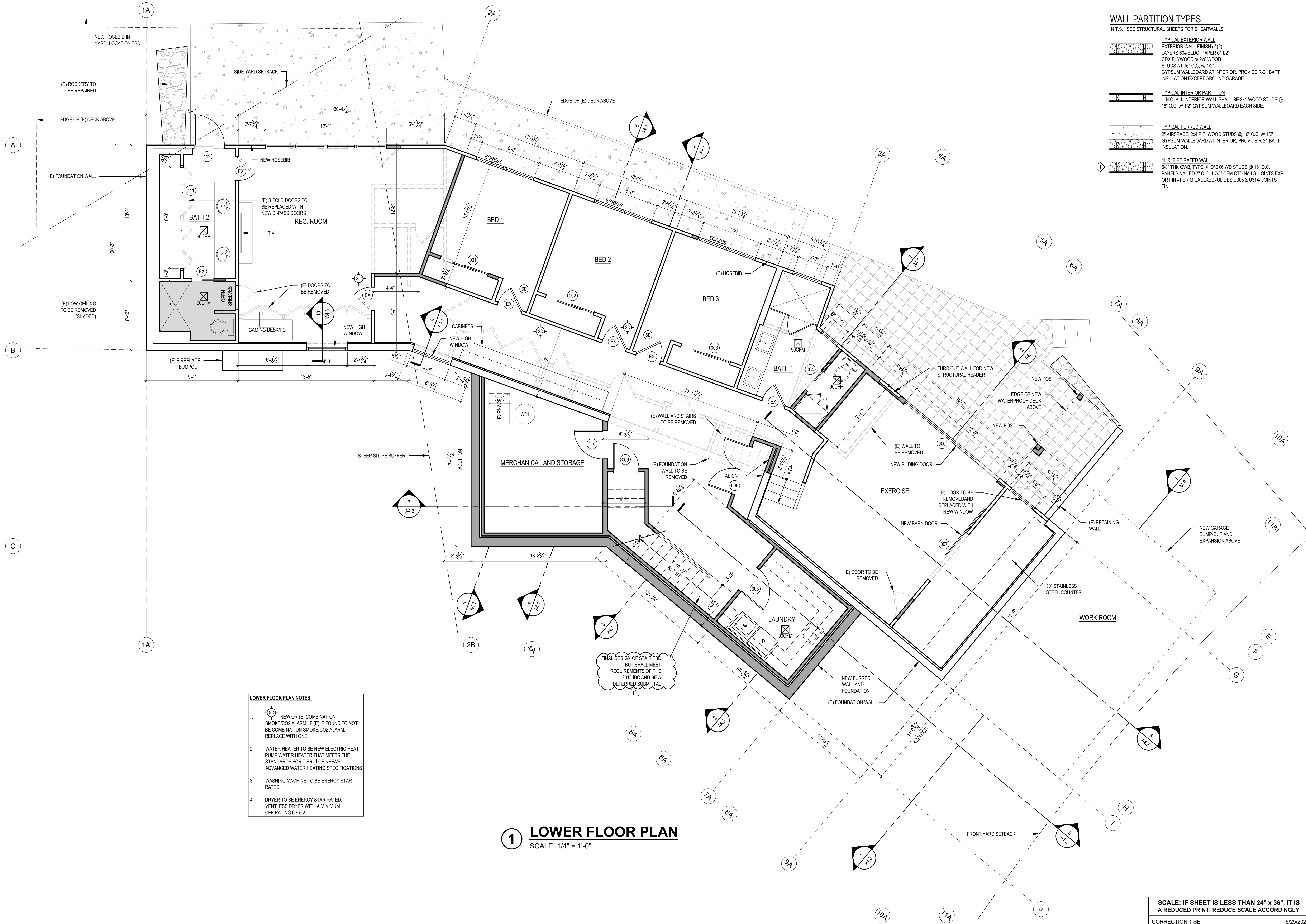
**EROSION CONTROL DETAILS AND NOTES**

REVISIONS:	CORRECTION 1 - 5/20/2024
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DRAWN BY:	
CHECKED BY:	BJS

**C1.0**

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- WALL PARTITION TYPES:**  
N.T.S. (SEE STRUCTURAL SHEETS FOR SHEARWALLS.)
- TYPICAL EXTERIOR WALL**  
EXTERIOR WALL FINISH OF (2) LAYERS 60# BLDG. PAPER OF 1/2" CDX PLYWOOD OF 2x6 WOOD STUDS AT 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION EXCEPT AROUND GARAGE.
  - TYPICAL INTERIOR PARTITION**  
U.N.O. ALL INTERIOR WALL SHALL BE 2x4 WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD EACH SIDE.
  - TYPICAL FURRED WALL**  
2" AIRSPACE, 2x4 P.T. WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION.
  - 1HR. FIRE RATED WALL**  
5/8" THK GWB, TYPE 'X' OI 2X6 WD STUDS @ 16" O.C. PANELS NAILED 7" O.C.-1 7/8" CEM CTD NAILS- JOINTS EXP OR FIN - PERIM CAULKED- UL DES U305 & U314- JOINTS FIN

- LOWER FLOOR PLAN NOTES:**
- NEW OR (E) COMBINATION SMOKE/CO2 ALARM. IF (E) IF FOUND TO NOT BE COMBINATION SMOKE/CO2 ALARM, REPLACE WITH ONE
  - WATER HEATER TO BE NEW ELECTRIC HEAT PUMP WATER HEATER THAT MEETS THE STANDARDS FOR TIER II OF NEEA'S ADVANCED WATER HEATING SPECIFICATIONS
  - WASHING MACHINE TO BE ENERGY STAR RATED
  - DRYER TO BE ENERGY STAR RATED, VENTLESS DRYER WITH A MINIMUM CEF RATING OF 5.2

FINAL DESIGN OF STAIR TBD BUT SHALL MEET REQUIREMENTS OF THE 2018 IBC AND BE A DEFERRED SUBMITTAL

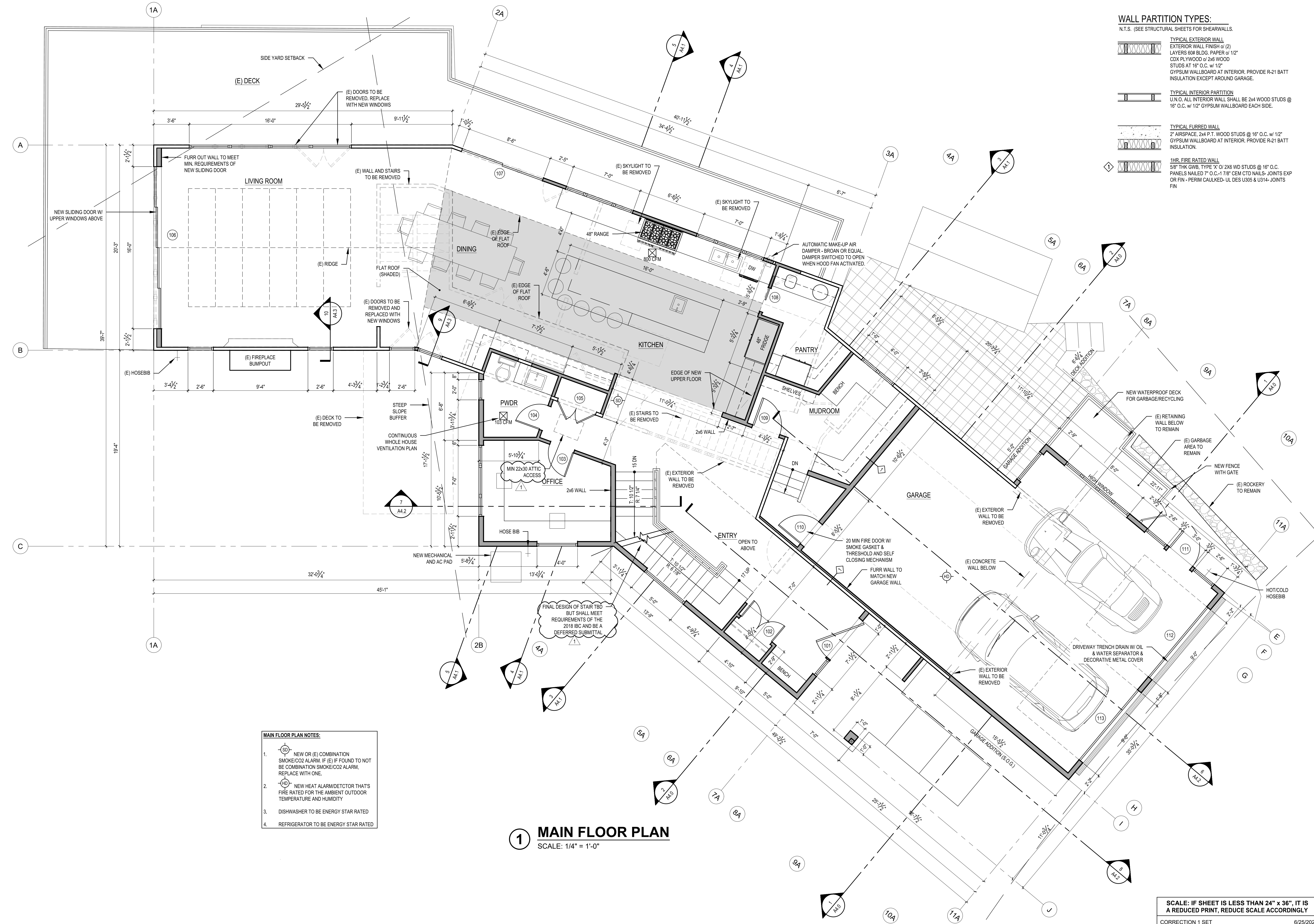
**1 LOWER FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

SCALE: IF SHEET IS LESS THAN 24" x 36", IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY  
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CHECKED BY: BJS



- WALL PARTITION TYPES:**  
 N.T.S. (SEE STRUCTURAL SHEETS FOR SHEARWALLS.)
- TYPICAL EXTERIOR WALL**  
 EXTERIOR WALL FINISH @ (2) LAYERS 5/8" BLDG. PAPER @ 1/2" CDX PLYWOOD @ 2x6 WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION EXCEPT AROUND GARAGE.
  - TYPICAL INTERIOR PARTITION**  
 U.N.O. ALL INTERIOR WALL SHALL BE 2x4 WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD EACH SIDE.
  - TYPICAL FURRED WALL**  
 2" AIRSPACE. 2x4 P.T. WOOD STUDS @ 16" O.C. w/ 1/2" GYPSUM WALLBOARD AT INTERIOR. PROVIDE R-21 BATT INSULATION.
  - 1HR. FIRE RATED WALL**  
 5/8" THK GWB. TYPE X @ 2x6 WID STUDS @ 16" O.C. PANELS NAILED 7" O.C.-1 7/8" CEM CTD NAILS- JOINTS EXP OR FIN - PERM CALKED-UL DES U305 & U314- JOINTS FIN

- MAIN FLOOR PLAN NOTES:**
- NEW OR (E) COMBINATION SMOKE/CO2 ALARM. IF (E) IF FOUND TO NOT BE COMBINATION SMOKE/CO2 ALARM, REPLACE WITH ONE.
  - NEW HEAT ALARM/DETECTOR THAT'S FIRE RATED FOR THE AMBIENT OUTDOOR TEMPERATURE AND HUMIDITY
  - DISHWASHER TO BE ENERGY STAR RATED
  - REFRIGERATOR TO BE ENERGY STAR RATED

**1 MAIN FLOOR PLAN**  
 SCALE: 1/4" = 1'-0"

SCALE: IF SHEET IS LESS THAN 24" x 36" IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY  
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**STURMAN ARCHITECTS**

REGISTERED ARCHITECT  
 BRADLEY J. STURMAN  
 STATE OF WASHINGTON

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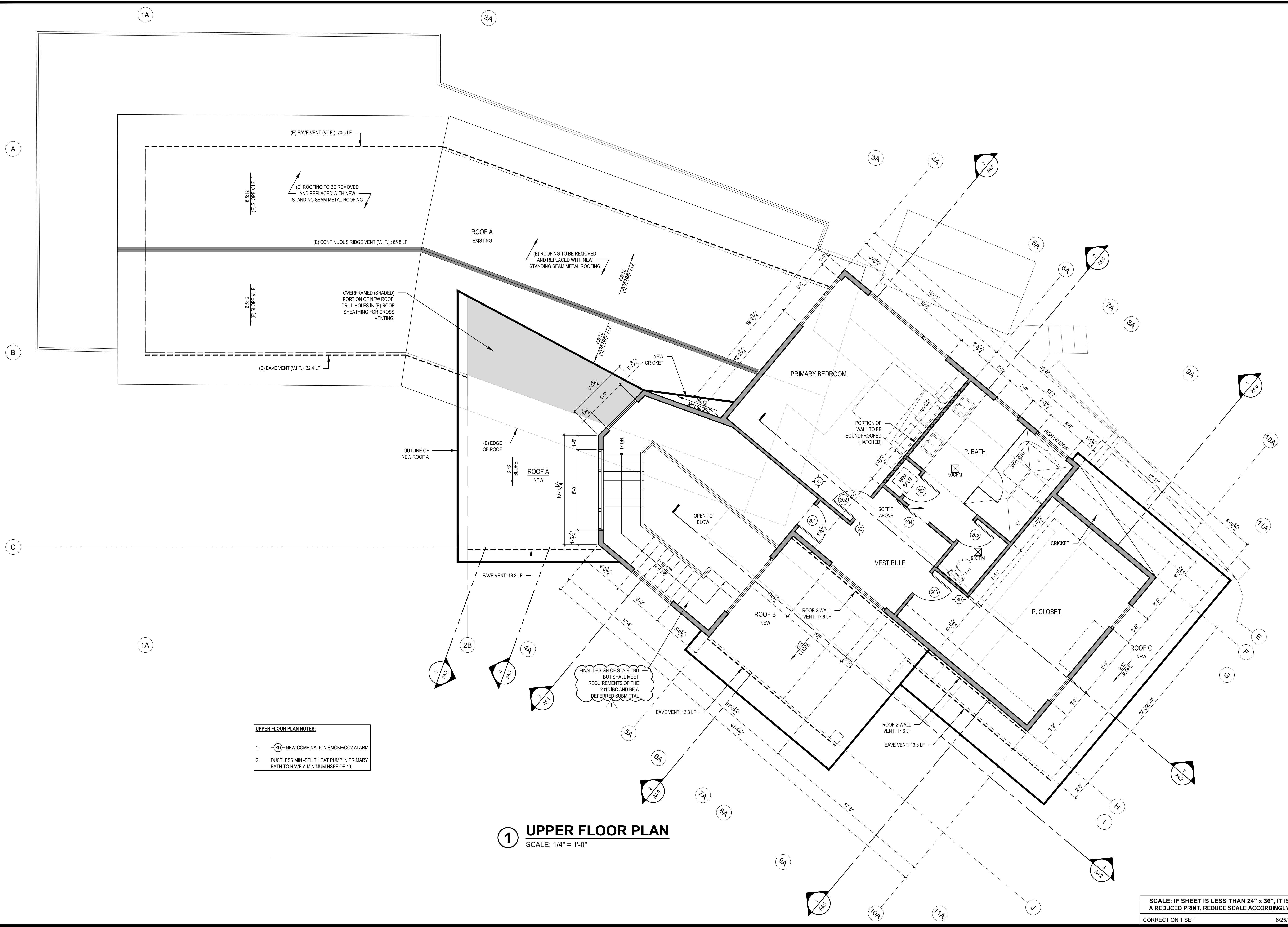
**RAWSON REMODEL**  
 8413 SE 82ND ST  
 MERCER ISLAND, WA 98040

**MAIN FLOOR PLAN**

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 CHECKED BY: BJS  
 SHEET **A2.1**

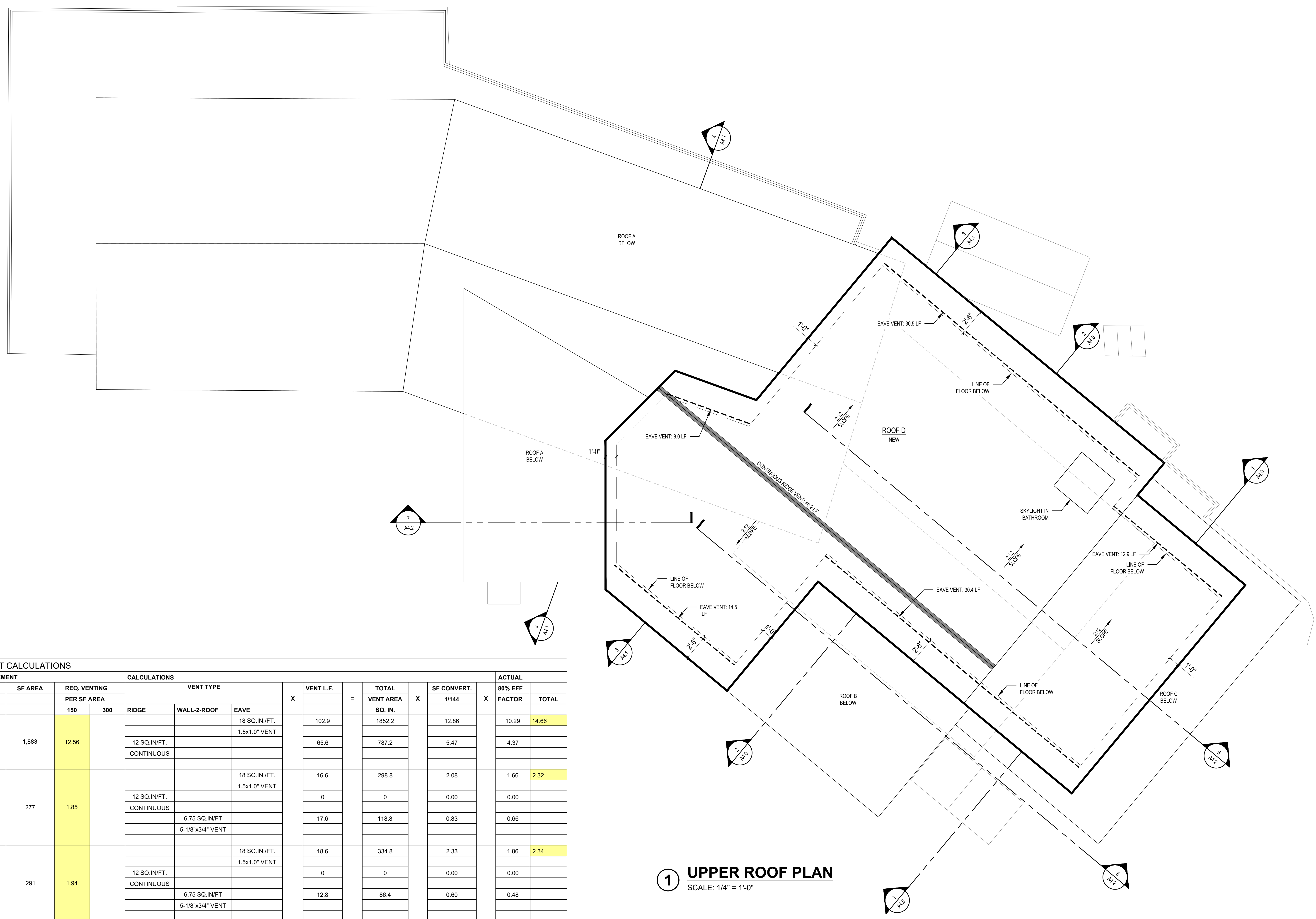
9- 103rd Ave NE Suite 203  
 Bellevue, WA 98004  
 TEL: 425.451.7003



- UPPER FLOOR PLAN NOTES:**
1. (SD) - NEW COMBINATION SMOKE/CO2 ALARM
  2. DUCTLESS MINI-SPLIT HEAT PUMP IN PRIMARY BATH TO HAVE A MINIMUM HSPF OF 10

FINAL DESIGN OF STAIR TBD BUT SHALL MEET REQUIREMENTS OF THE 2018 BC AND BE A DEFERRED SUBMITTAL

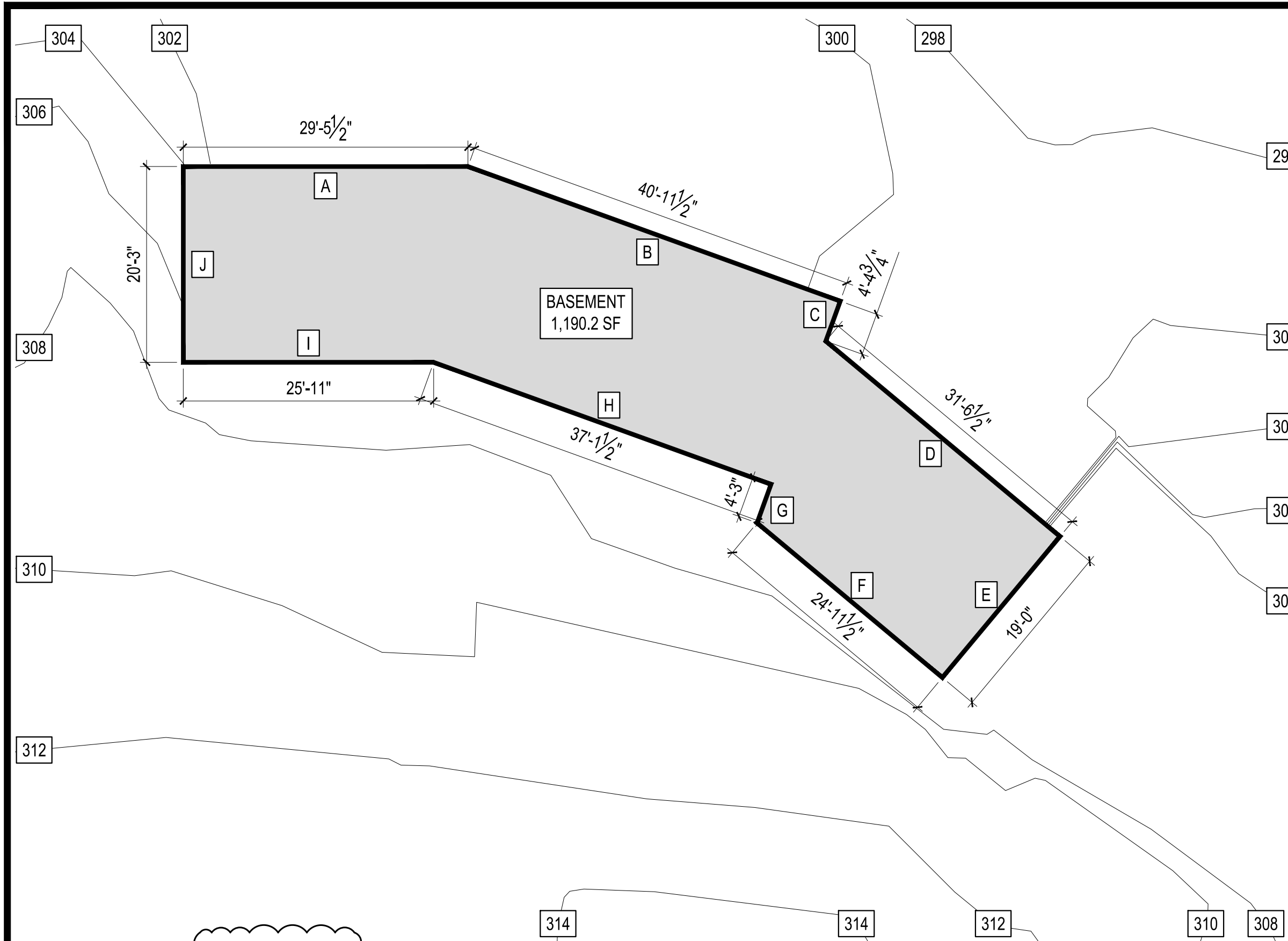
**1 UPPER FLOOR PLAN**  
 SCALE: 1/4" = 1'-0"



**1 UPPER ROOF PLAN**  
 SCALE: 1/4" = 1'-0"

ROOF VENT CALCULATIONS																
CODE REQUIREMENT			CALCULATIONS						ACTUAL							
DESCRIPTION	SF AREA	REQ. VENTING		VENT TYPE			X	VENT L.F.	=	TOTAL VENT AREA SQ. IN.	X	SF CONVERT. 1/144	X	80% EFF FACTOR	TOTAL	
		150	300	RIDGE	WALL-2-ROOF	EAVE										
ROOF A	1,883	12.56				18 SQ.IN./FT.		102.9		1852.2		12.86		10.29	14.66	
						1.5x1.0" VENT										
				12 SQ.IN./FT.				65.6		787.2		5.47			4.37	
ROOF B	277	1.85				18 SQ.IN./FT.		16.6		298.8		2.08		1.66	2.32	
						1.5x1.0" VENT										
				12 SQ.IN./FT.				0		0		0.00		0.00		
				CONTINUOUS												
ROOF C	291	1.94				18 SQ.IN./FT.		18.6		334.8		2.33		1.86	2.34	
						1.5x1.0" VENT										
				12 SQ.IN./FT.				0		0		0.00		0.00		
				CONTINUOUS												
ROOF D	1,573	10.49				18 SQ.IN./FT.		96.3		1733.4		12.04		9.63	12.31	
						1.5x1.0" VENT										
				12 SQ.IN./FT.				40.2		482.4		3.35		2.68		
				CONTINUOUS												

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**AS BUILT BASEMENT FLOOR AREA CALCULATION**

Basement Floor Area = 1889.2 SF  
Basement Ceiling Height = 8 FT

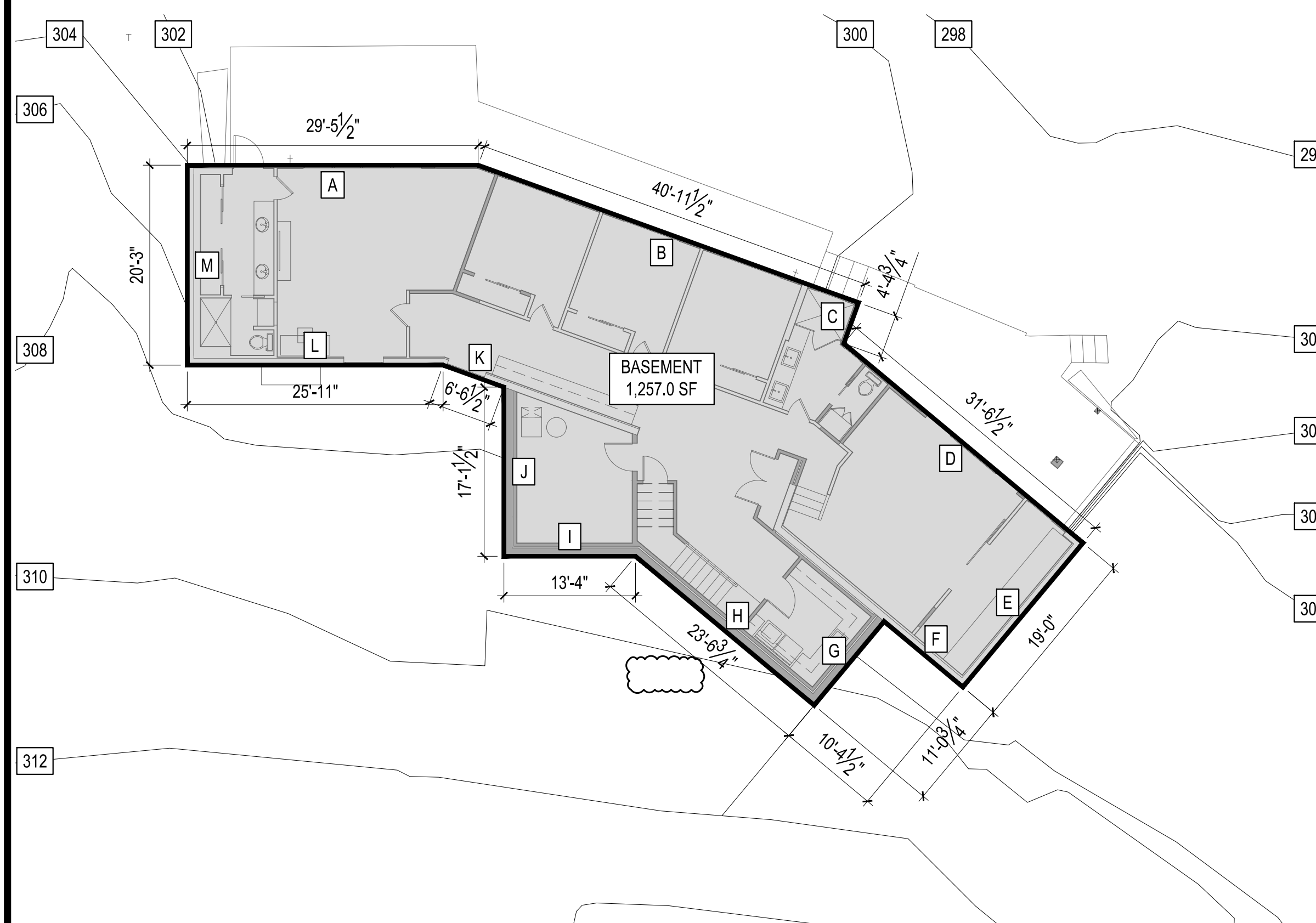
SEGMENT	LENGTH	COVERAGE	RESULT
A	38.9	6%	2.2951
B	40.9	0%	0
C	4.4	0%	0
D	31.5	6%	1.79235
E	19	100%	19
F	24.9	51%	12.5745
G	4.3	99%	4.257
H	37.1	65%	24.0779
I	25.9	67%	17.4307
J	20.3	53%	10.71028
<b>TOTAL</b>	<b>247.2</b>		<b>92.13783</b>

Portion of excluded Basement Floor Area = X SF

1,889.20	X	92.13783
		247.2
1,889.20	X	37%

**699 SF EXCLUDED**  
BASEMENT AREA = 1190.2 SF

**A1 AS BUILT BASEMENT FAR**  
SCALE: 3/32" = 1'-0"



**PROPOSED BASEMENT FLOOR AREA CALCULATION**

Basement Floor Area = 2417.4 SF  
Basement Ceiling Height = 8 FT

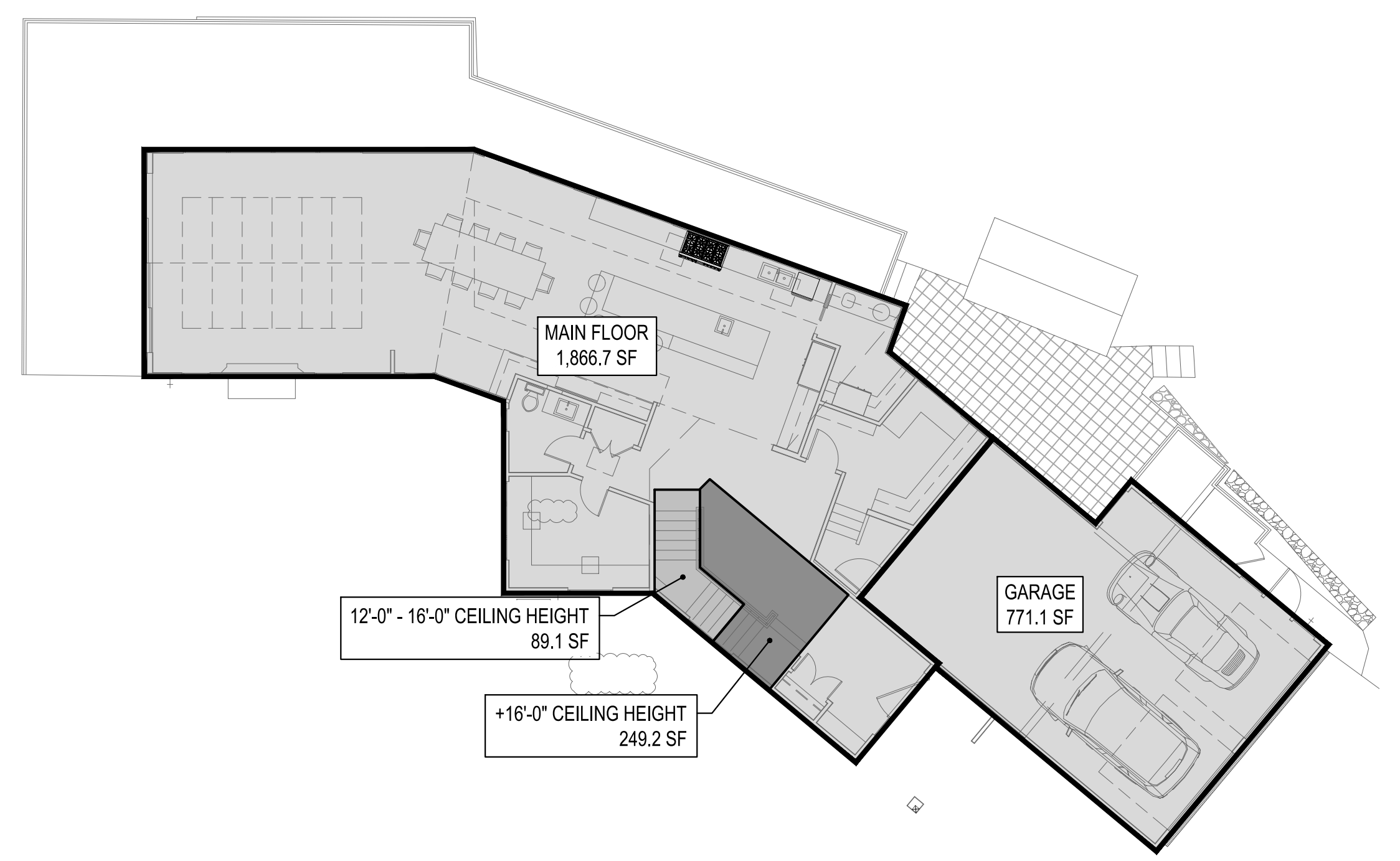
SEGMENT	LENGTH	COVERAGE	RESULT
A	38.9	6%	2.2951
B	40.9	0%	0
C	4.4	0%	0
D	31.5	6%	1.79235
E	19	100%	19
F	10.4	100%	10.4
G	11.1	93%	10.2897
H	23.6	96%	22.6678
I	13.3	92%	12.26127
J	17.1	85%	14.5008
K	6.6	65%	4.28274
L	25.9	67%	17.353
M	20.3	53%	10.759
<b>TOTAL</b>	<b>263</b>		<b>125.60176</b>

Portion of excluded Basement Floor Area = X SF

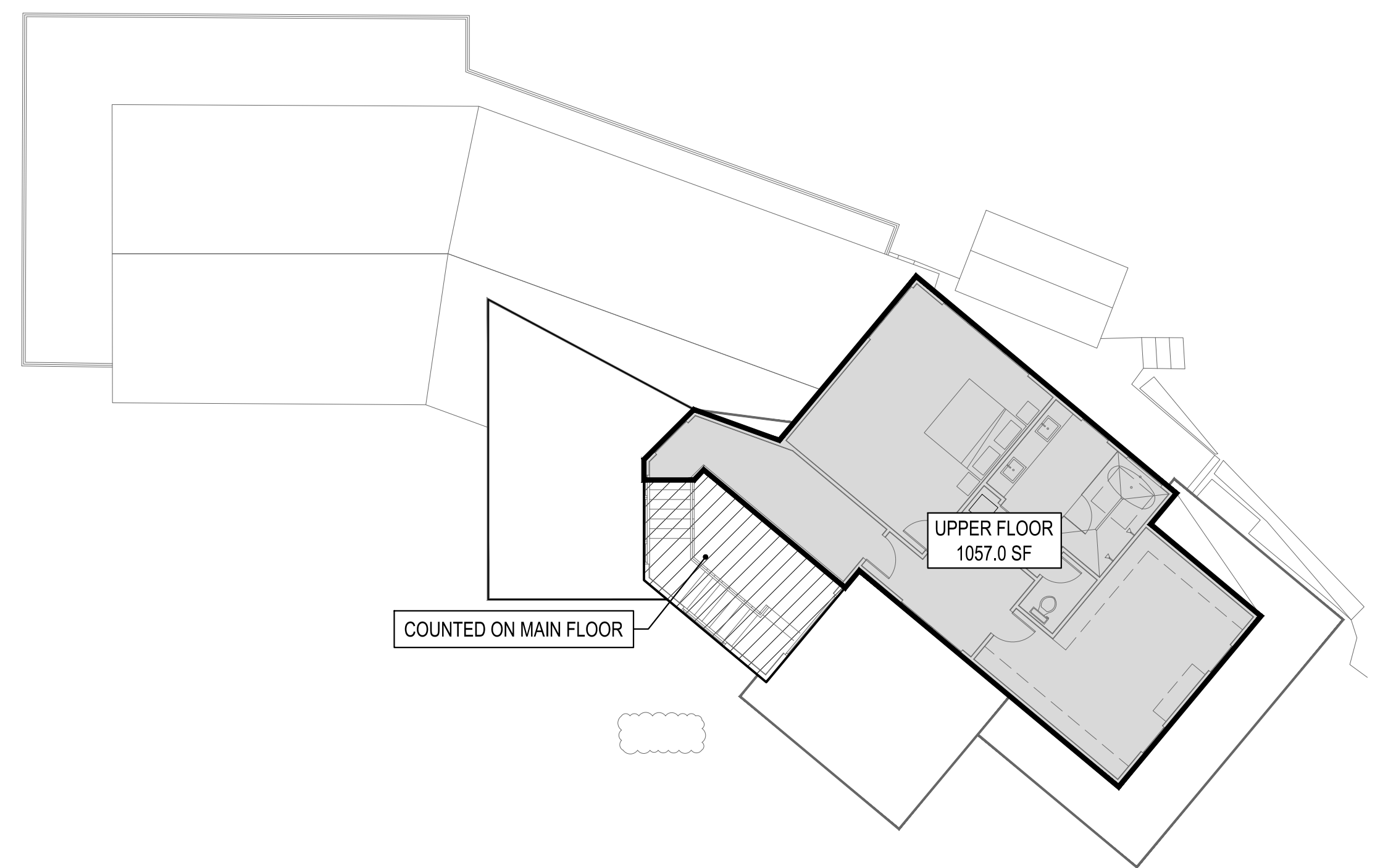
2,417.40	X	125.60176
		263
2,417.40	X	48%

**1,160.4 SF EXCLUDED**  
BASEMENT AREA = 1257.0 SF

**1 BASEMENT FAR**  
SCALE: 3/32" = 1'-0"



**2 MAIN FLOOR FAR**  
SCALE: 3/32" = 1'-0"

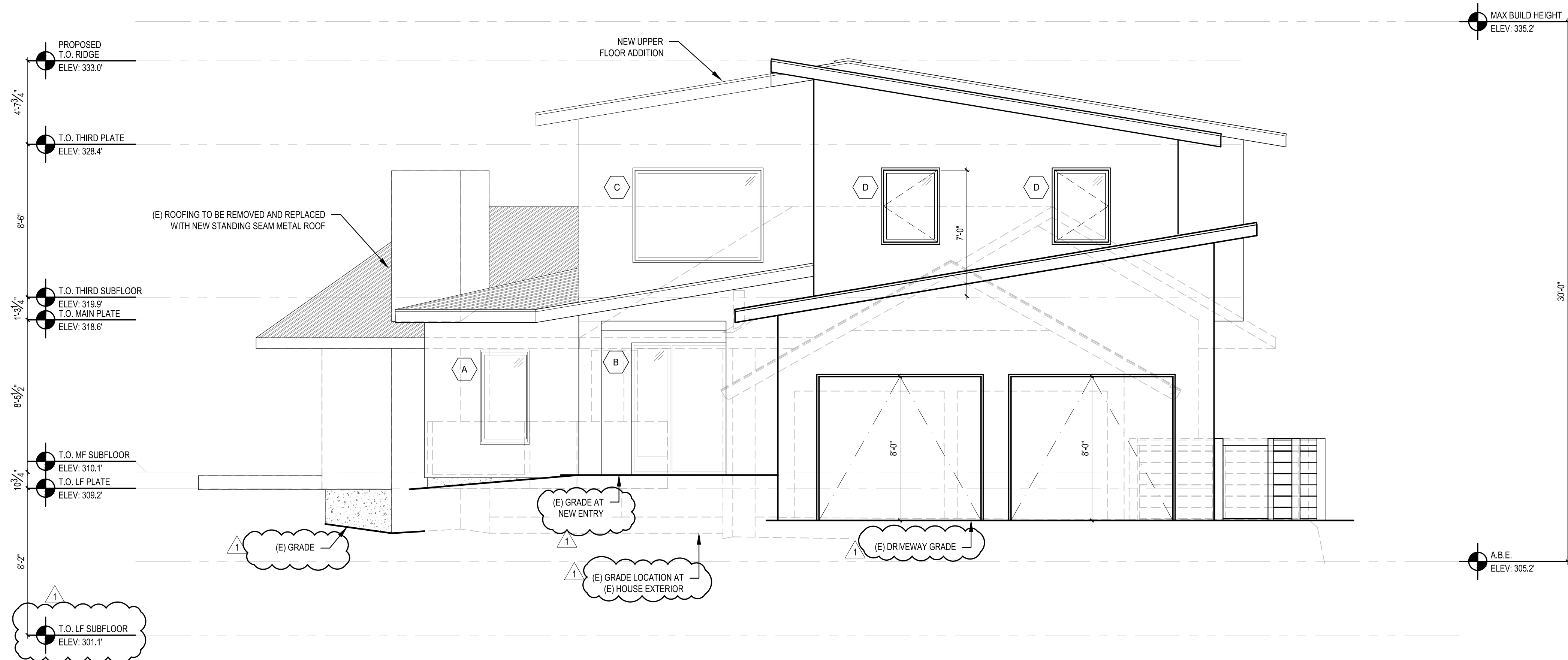


**3 UPPER FLOOR FAR**  
SCALE: 3/32" = 1'-0"

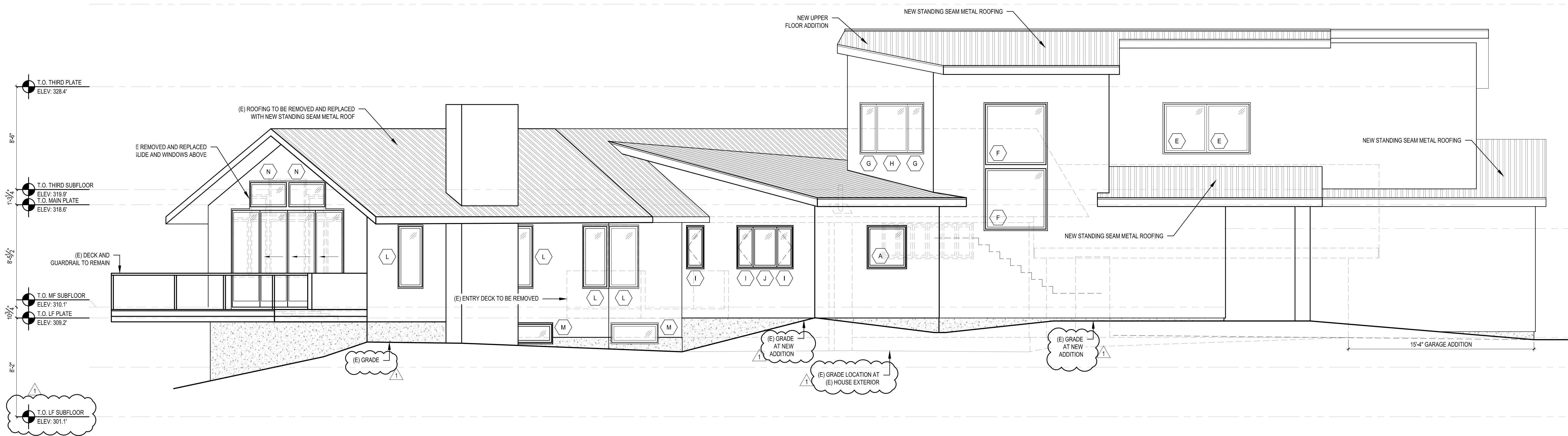
**PROPOSED GROSS FLOOR AREA**

	PROPOSED FLOOR AREA	LOT SIZE	GFA THRESHOLD
BASEMENT	1,257.0 SF	16,550 SF	= 6,620 SF
MAIN FLOOR	1,866.7 SF	PROPOSED GFA	= 5,290.1 SF
THIRD FLOOR	1,057.0 SF	PROPOSED %GFA COVERAGE	= 31.9%
GARAGE	771.1 SF	PROPOSED GFA IS 5,080.4 SF OR 31.9%	
12-16'-0" CEILING HEIGHT	89.1 SF		
16'-0" + CEILING HEIGHT	249.2 SF		
<b>TOTAL</b>	<b>5,290.1 SF</b>		

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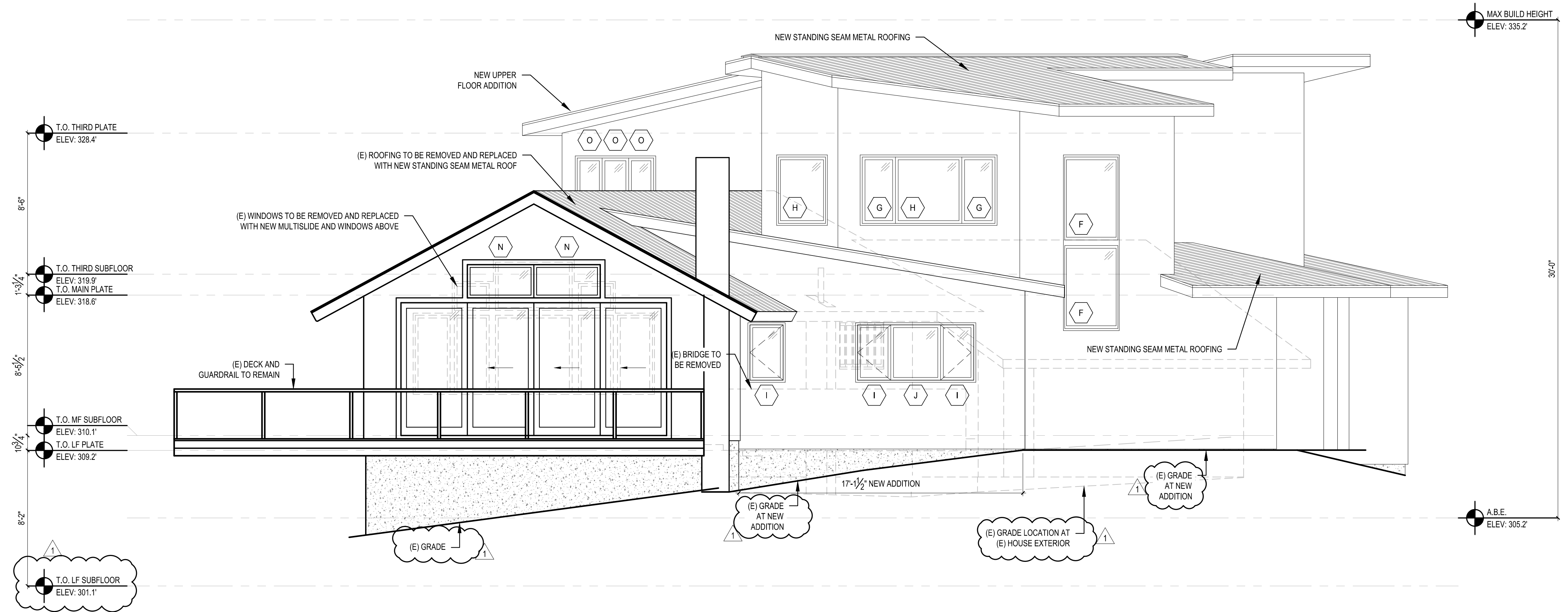
**1 WEST ELEVATION**  
SCALE: 1/4" = 1'-0"



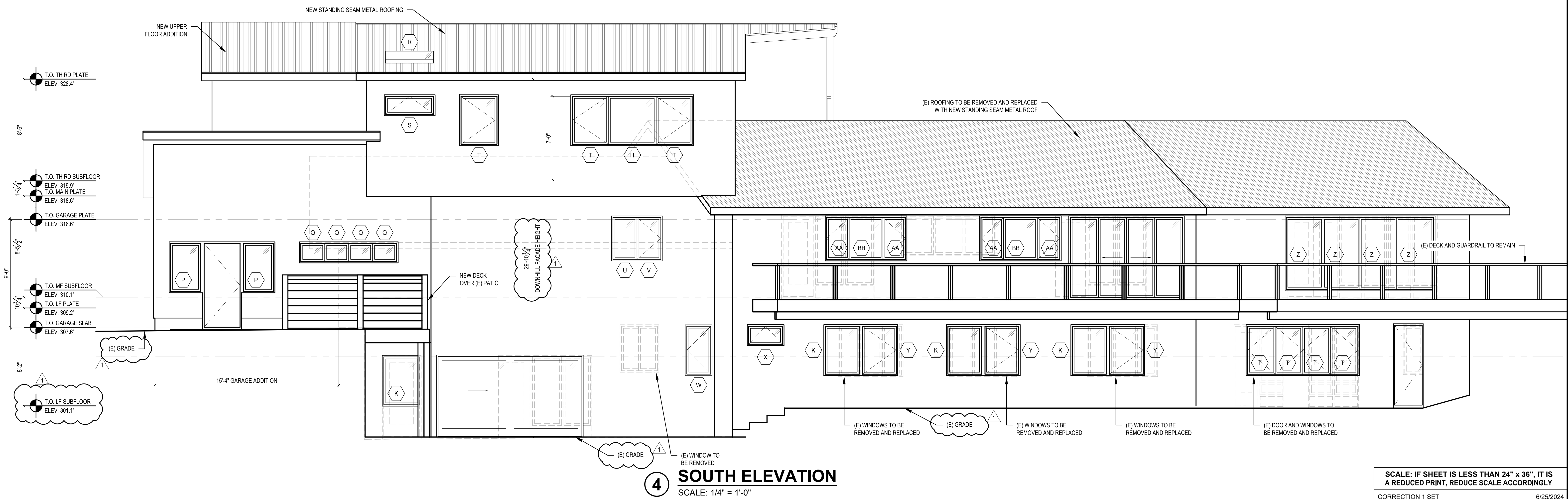
**2 NORTH ELEVATION**  
SCALE: 1/4" = 1'-0"

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SHEET	A3.0

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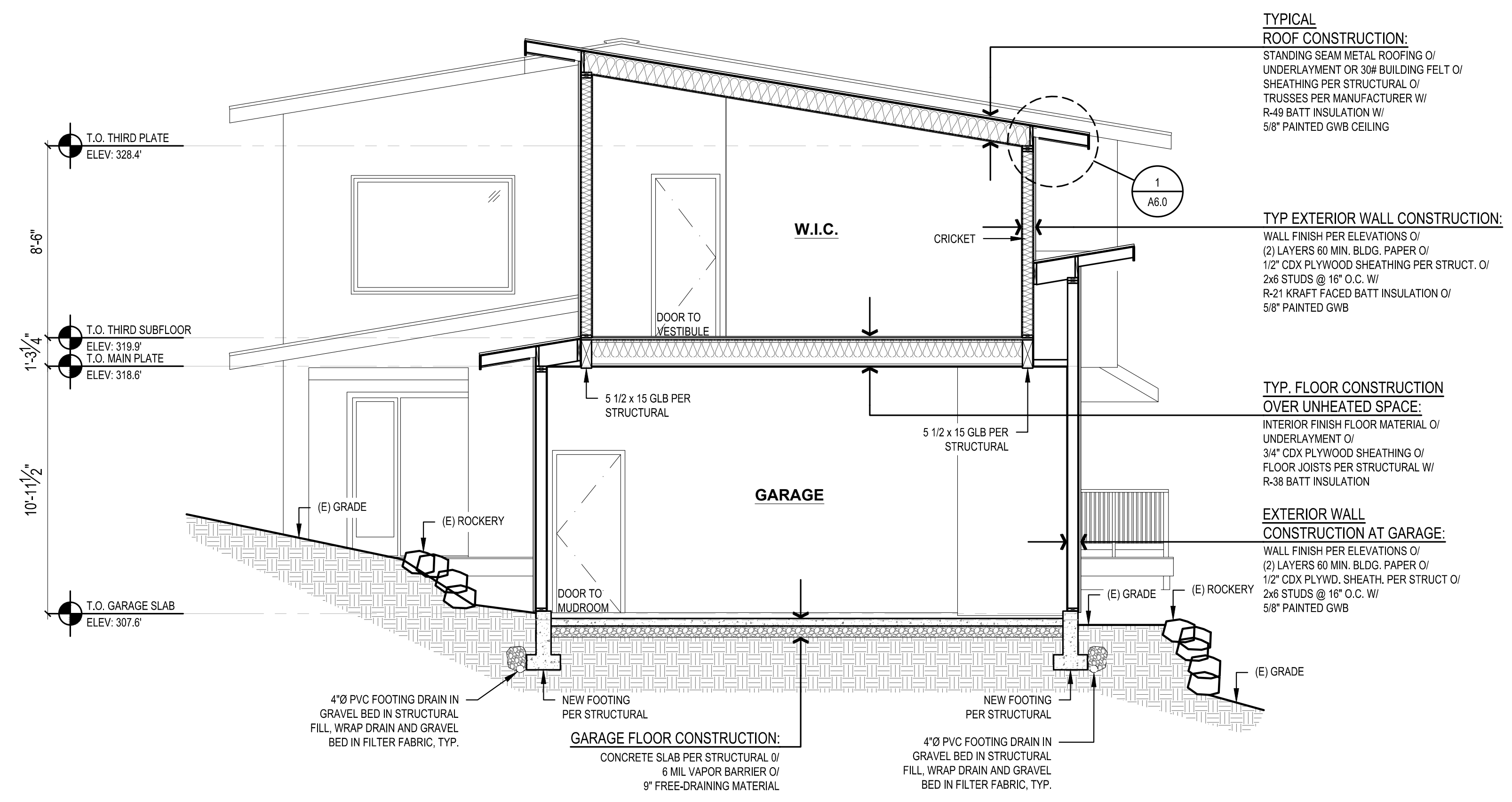
**3 EAST ELEVATION**  
SCALE: 1/4" = 1'-0"



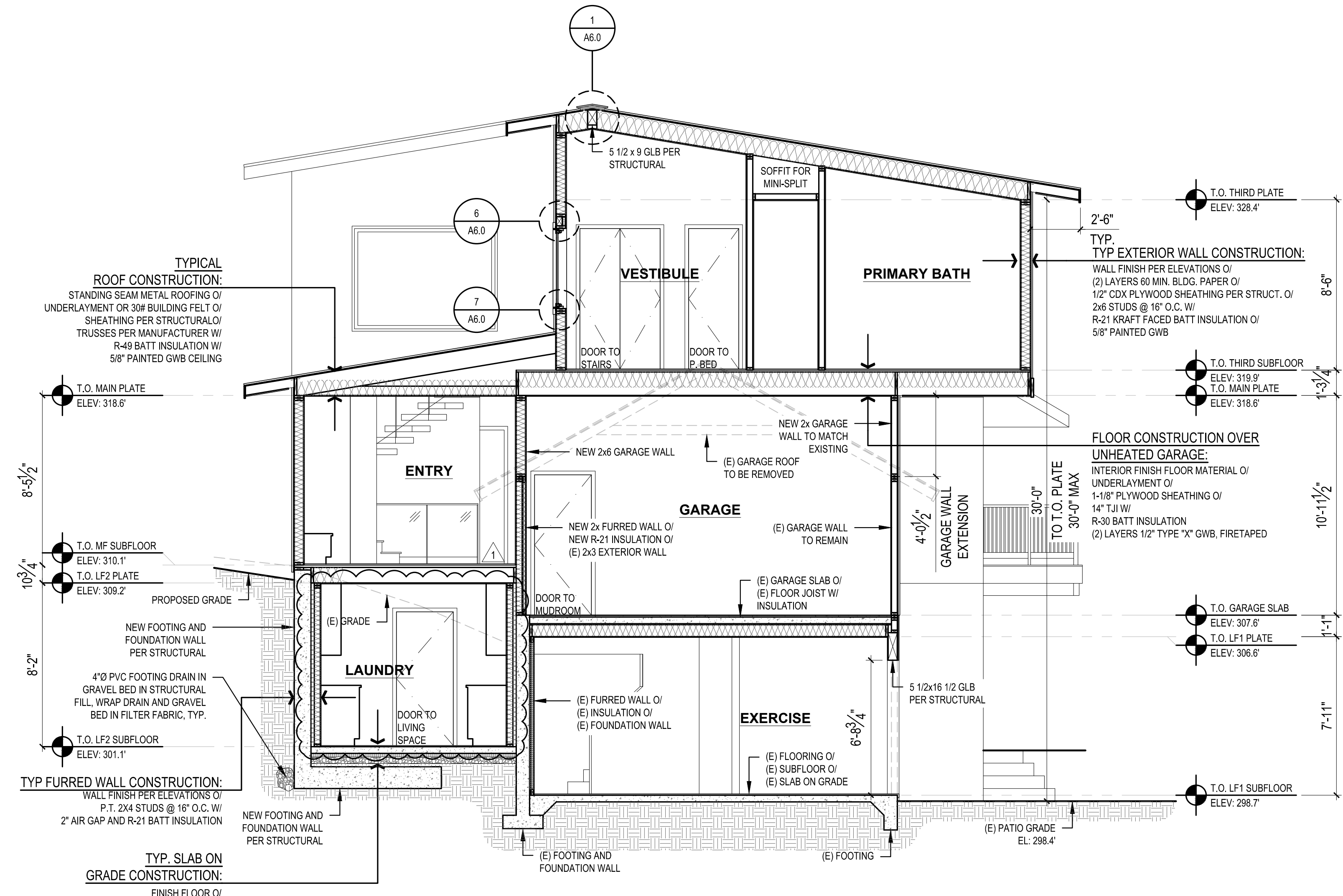
**4 SOUTH ELEVATION**  
SCALE: 1/4" = 1'-0"

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**1 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"



**2 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"

**TYPICAL ROOF CONSTRUCTION:**  
STANDING SEAM METAL ROOFING O/  
UNDERLAYMENT OR 30# BUILDING FELT O/  
SHEATHING PER STRUCTURAL O/  
TRUSSES PER MANUFACTURER W/  
R-49 BATT INSULATION W/  
5/8" PAINTED GWB CEILING

**TYP EXTERIOR WALL CONSTRUCTION:**  
WALL FINISH PER ELEVATIONS O/  
(2) LAYERS 60 MIN. BLDG. PAPER O/  
1/2" CDX PLYWOOD SHEATHING PER STRUCT. O/  
2x6 STUDS @ 16" O.C. W/  
R-21 KRAFT FACED BATT INSULATION O/  
5/8" PAINTED GWB

**TYP. FLOOR CONSTRUCTION OVER UNHEATED SPACE:**  
INTERIOR FINISH FLOOR MATERIAL O/  
UNDERLAYMENT O/  
3/4" CDX PLYWOOD SHEATHING O/  
FLOOR JOISTS PER STRUCTURAL W/  
R-38 BATT INSULATION

**EXTERIOR WALL CONSTRUCTION AT GARAGE:**  
WALL FINISH PER ELEVATIONS O/  
(2) LAYERS 60 MIN. BLDG. PAPER O/  
1/2" CDX PLYWOOD SHEATH. PER STRUCT. O/  
2x6 STUDS @ 16" O.C. W/  
5/8" PAINTED GWB

**GARAGE FLOOR CONSTRUCTION:**  
CONCRETE SLAB PER STRUCTURAL O/  
6 MIL VAPOR BARRIER O/  
9" FREE-DRAINING MATERIAL

**TYPICAL ROOF CONSTRUCTION:**  
STANDING SEAM METAL ROOFING O/  
UNDERLAYMENT OR 30# BUILDING FELT O/  
SHEATHING PER STRUCTURAL O/  
TRUSSES PER MANUFACTURER W/  
R-49 BATT INSULATION W/  
5/8" PAINTED GWB CEILING

**TYP EXTERIOR WALL CONSTRUCTION:**  
WALL FINISH PER ELEVATIONS O/  
(2) LAYERS 60 MIN. BLDG. PAPER O/  
1/2" CDX PLYWOOD SHEATHING PER STRUCT. O/  
2x6 STUDS @ 16" O.C. W/  
R-21 KRAFT FACED BATT INSULATION O/  
5/8" PAINTED GWB

**FLOOR CONSTRUCTION OVER UNHEATED GARAGE:**  
INTERIOR FINISH FLOOR MATERIAL O/  
UNDERLAYMENT O/  
1-1/8" PLYWOOD SHEATHING O/  
14" TJI W/  
R-30 BATT INSULATION  
(2) LAYERS 1/2" TYPE "X" GWB, FIRE TAPED

**TYP FURRED WALL CONSTRUCTION:**  
WALL FINISH PER ELEVATIONS O/  
P.T. 2x4 STUDS @ 16" O.C. W/  
2" AIR GAP AND R-21 BATT INSULATION

**TYP. SLAB ON GRADE CONSTRUCTION:**  
FINISH FLOOR O/  
UNDERLAYMENT O/  
CONCRETE SLAB PER STRUCTURAL O/  
R-10 RIGID INSULATION (ENTIRE SLAB) O/  
6 MIL VAPOR BARRIER O/  
6" FREE-DRAINING MATERIAL

REVISIONS:

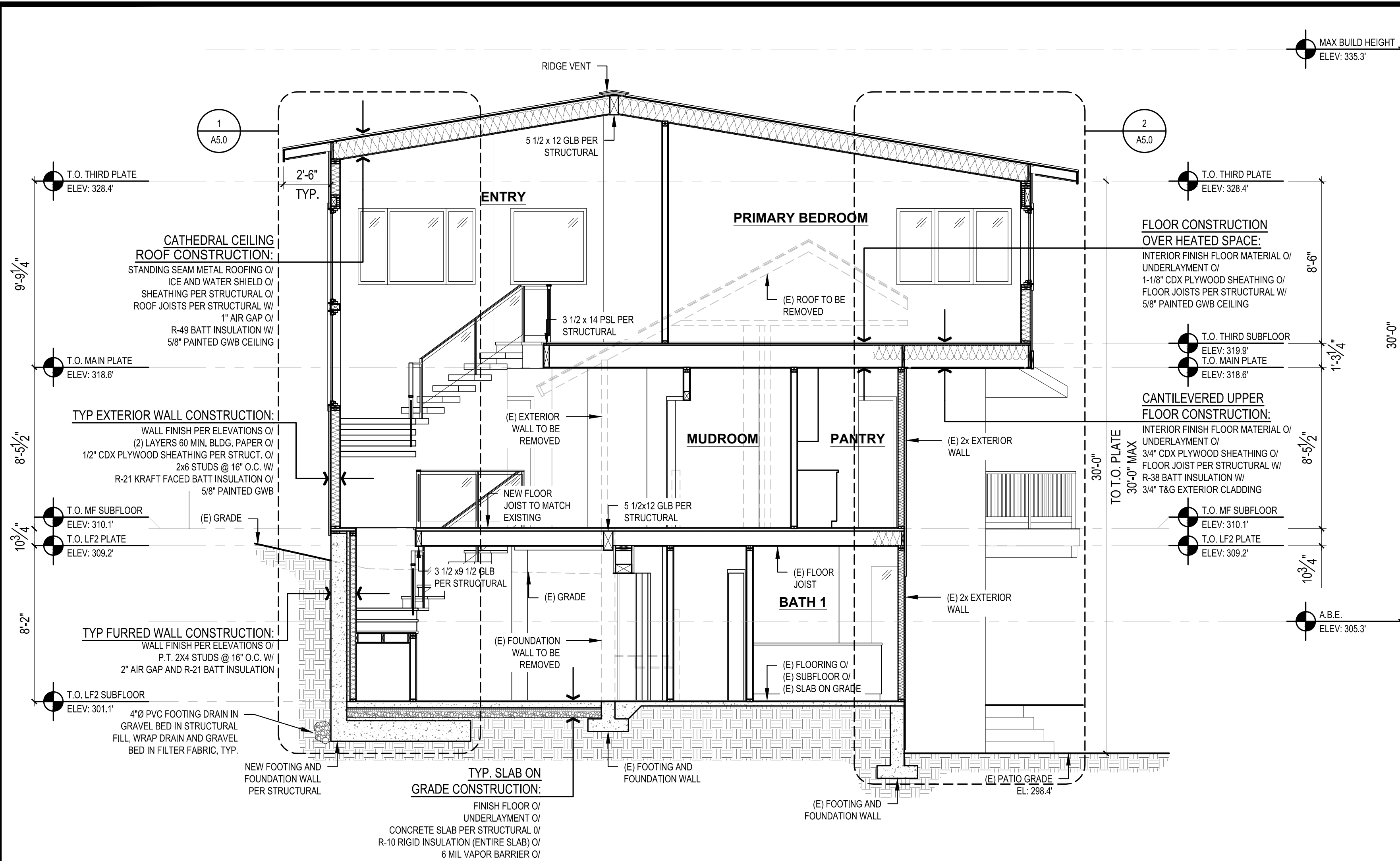
1	CORRECTION 1 - 5/30/2024
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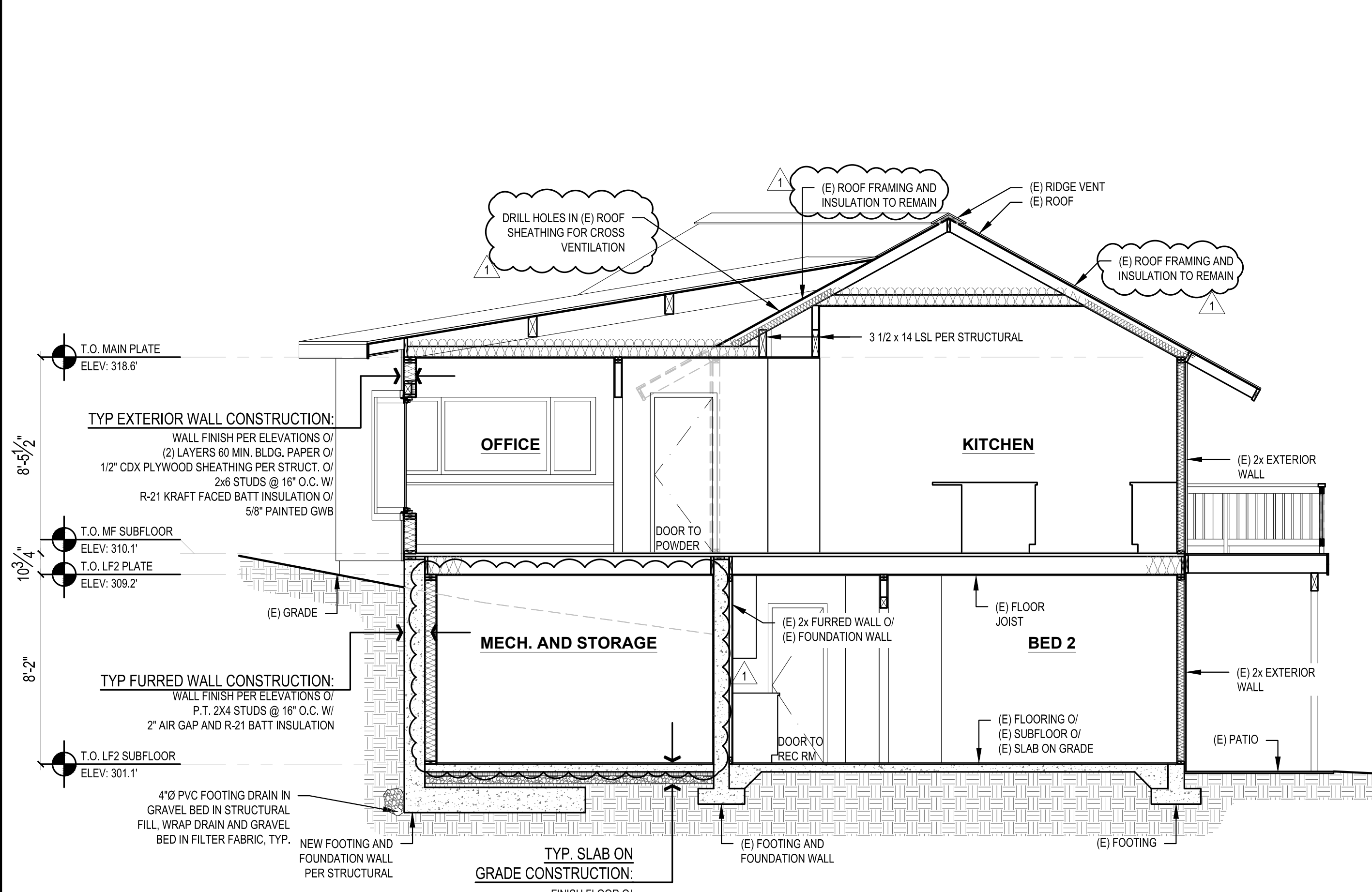
SHEET  
**A4.0**

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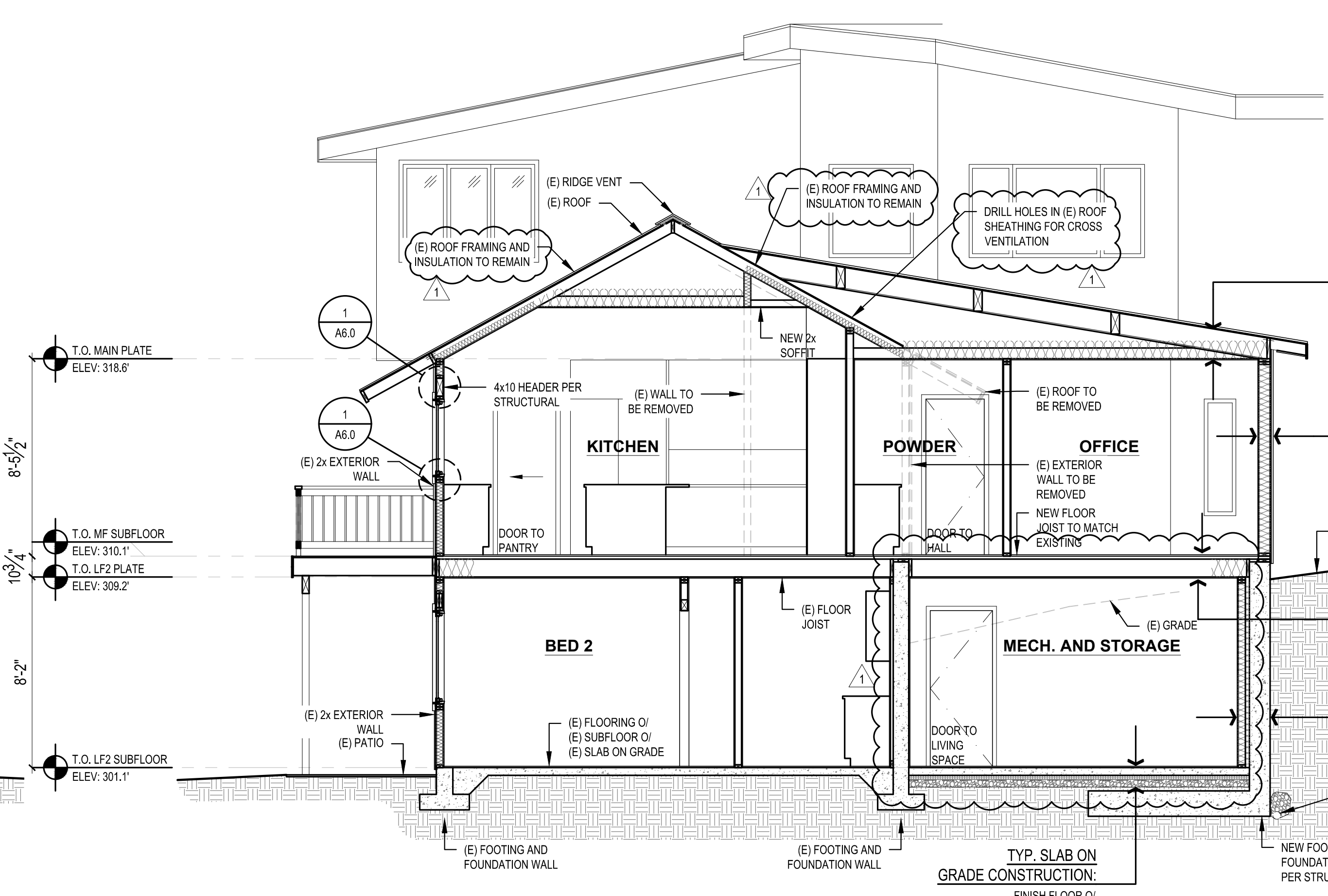




**3 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"



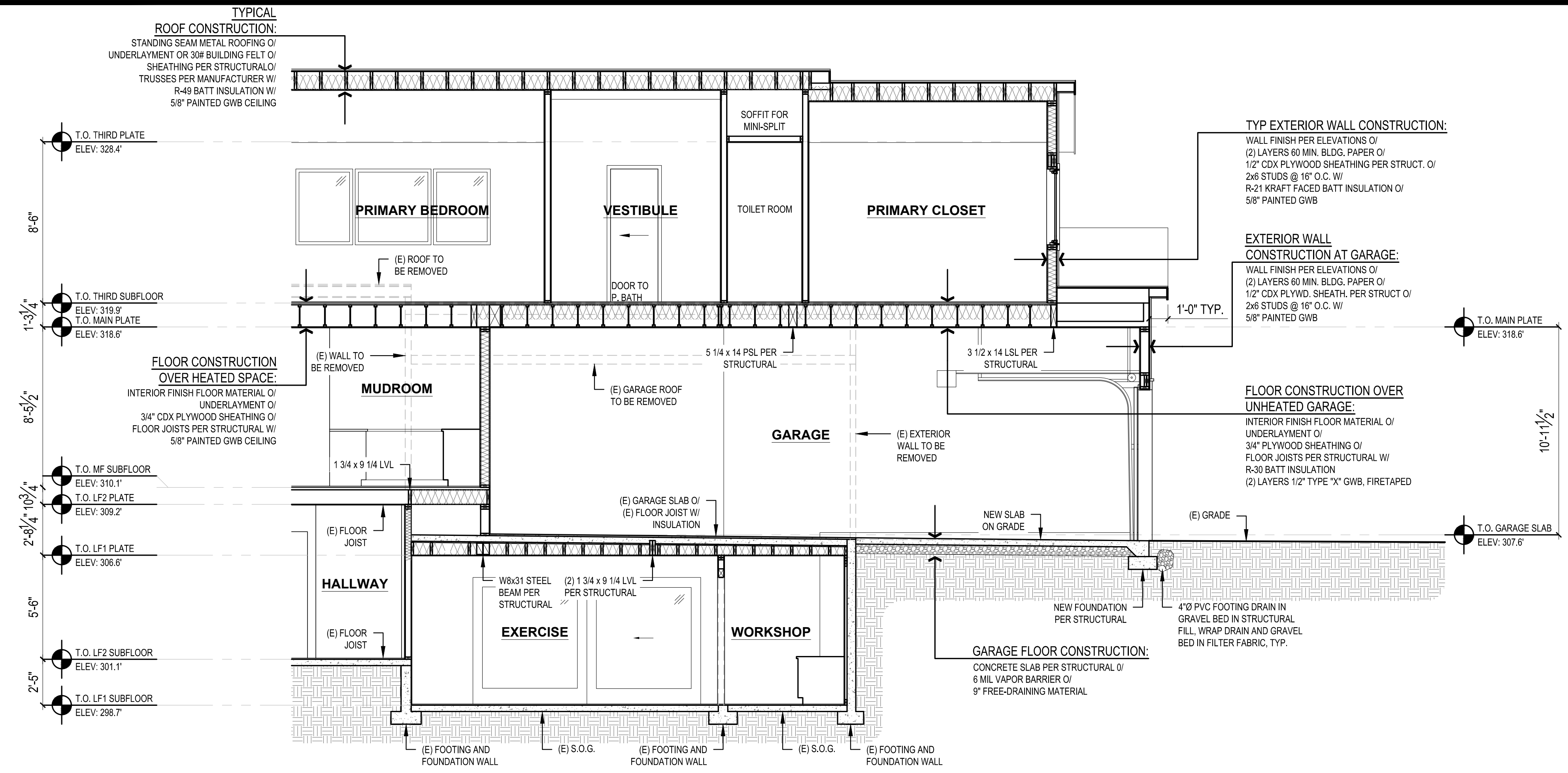
**4 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"



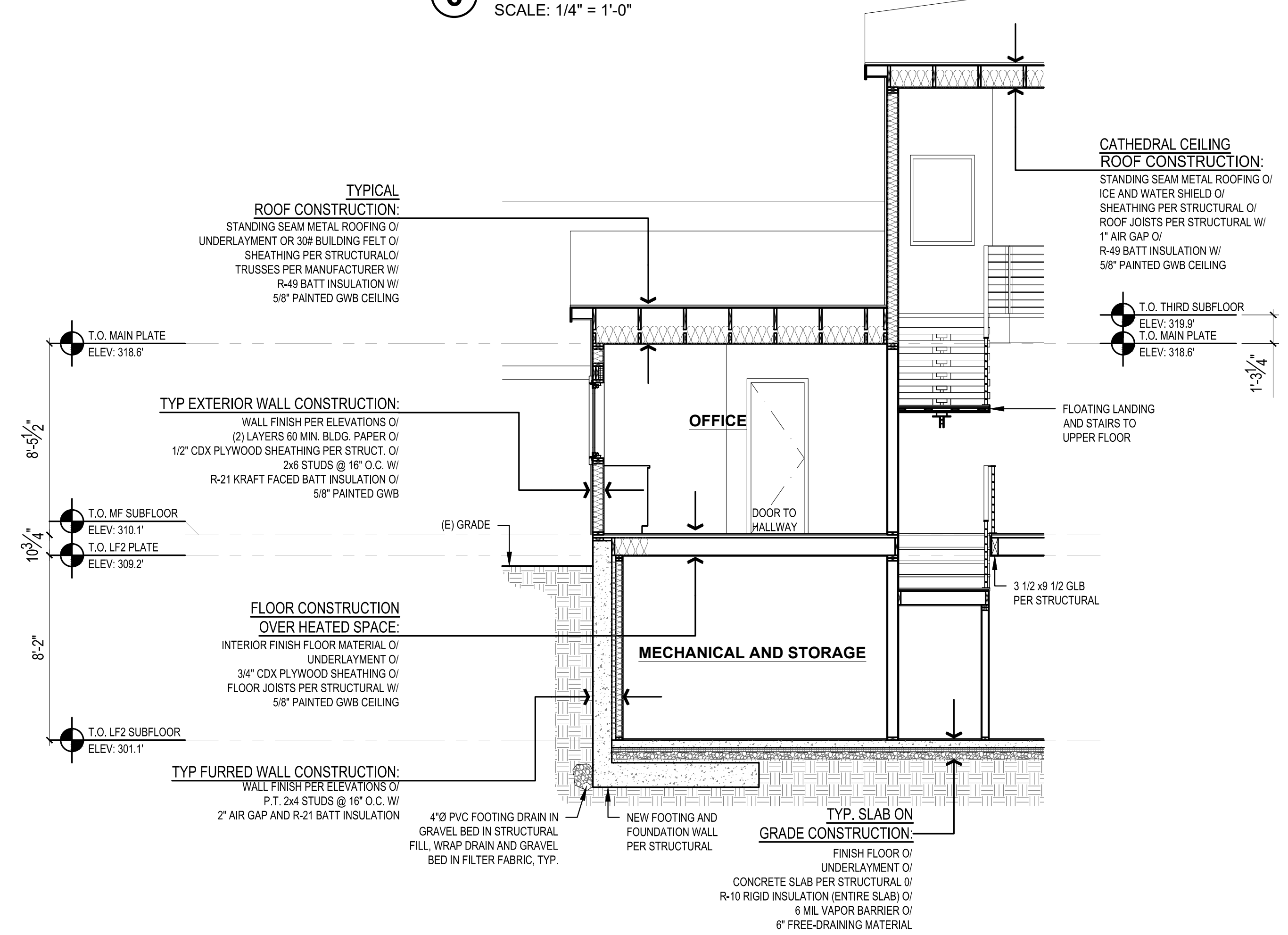
**5 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"

SCALE: IF SHEET IS LESS THAN 24" x 36", IT IS A REDUCED PRINT, REDUCE SCALE ACCORDINGLY  
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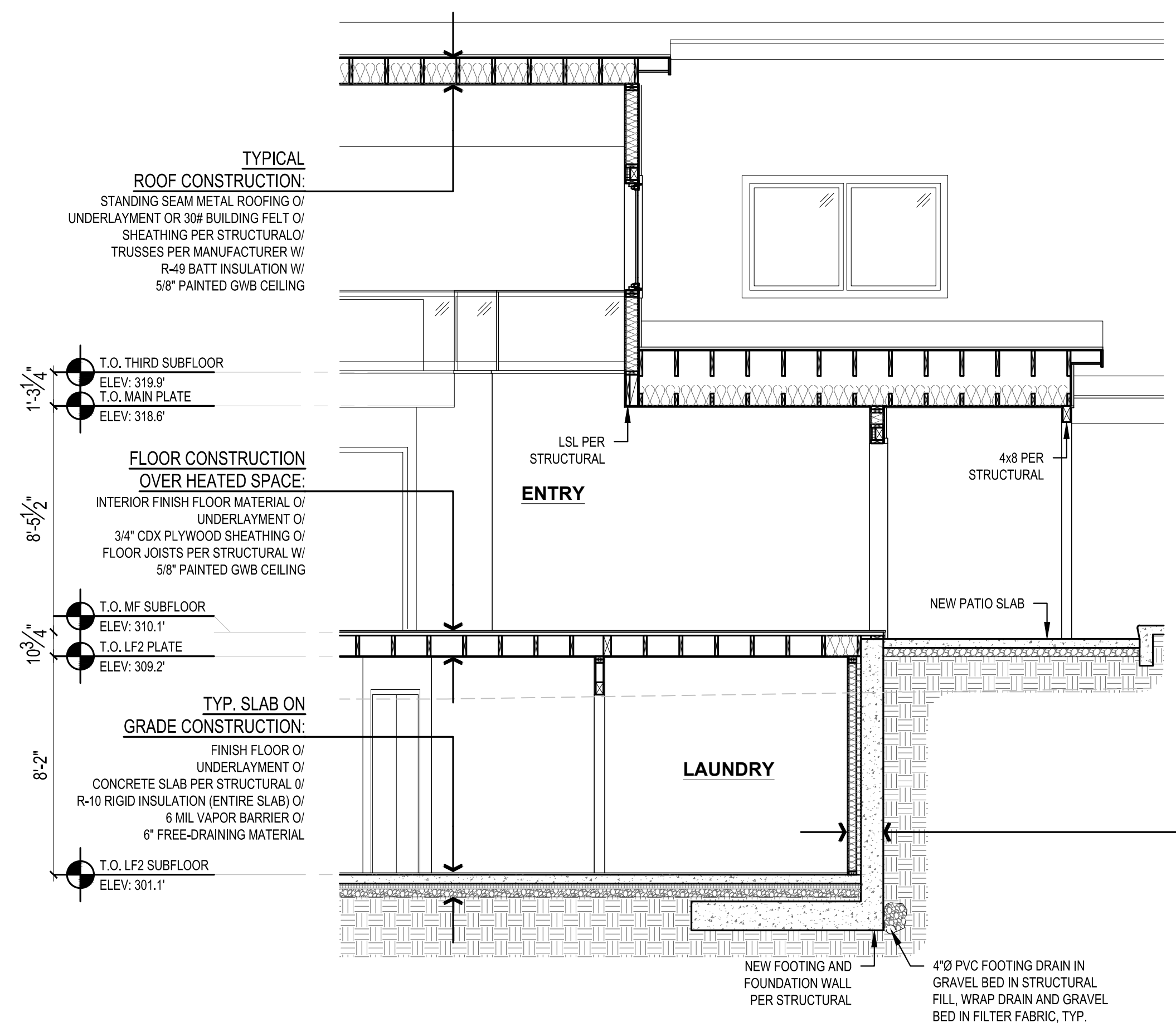
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**6 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"

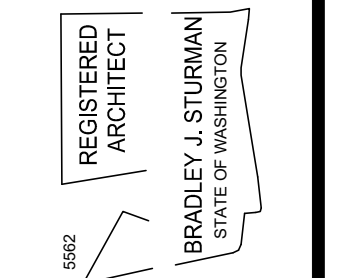


**7 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"



**8 BUILDING SECTION**  
SCALE: 1/4" = 1'-0"

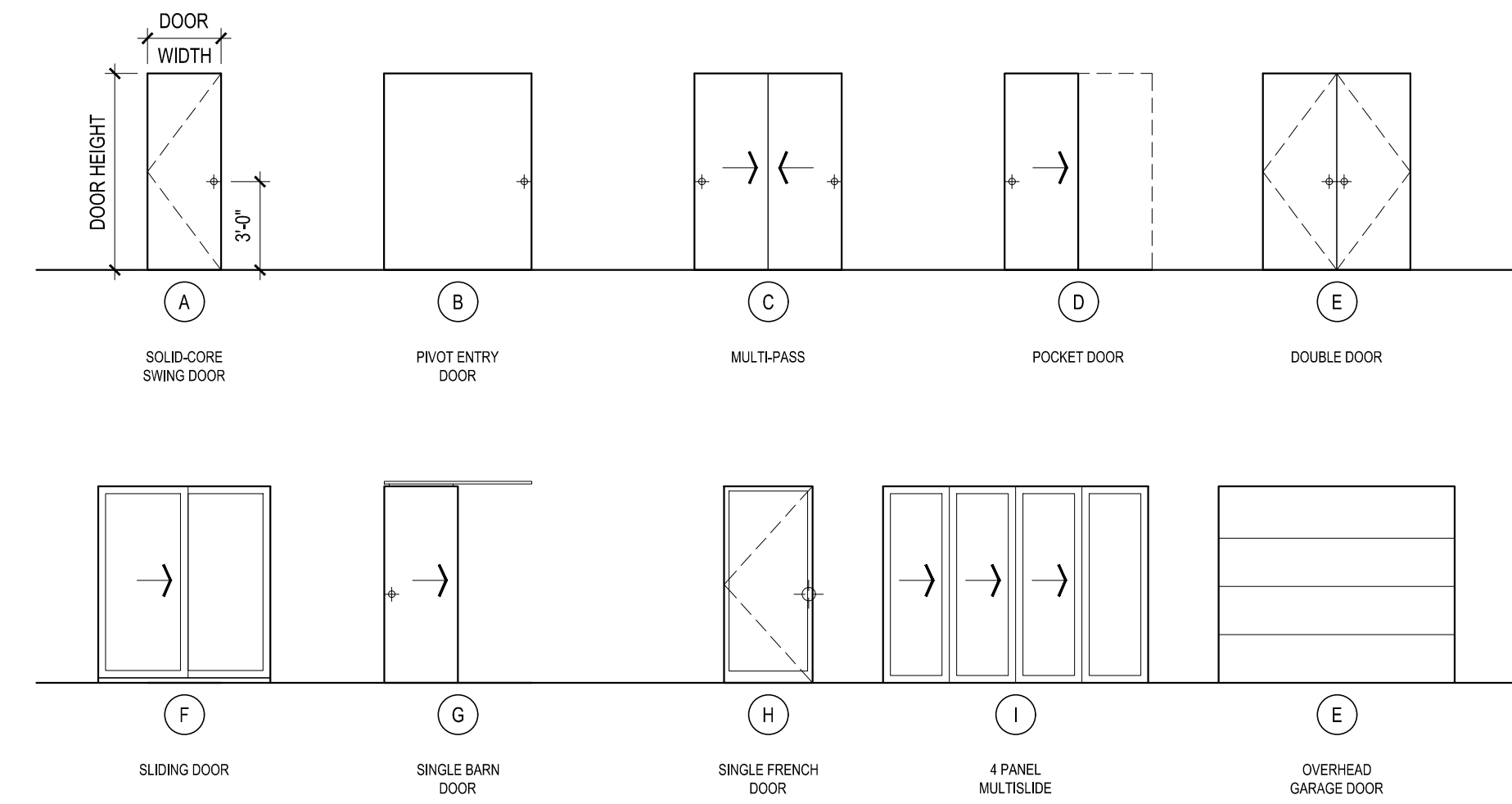
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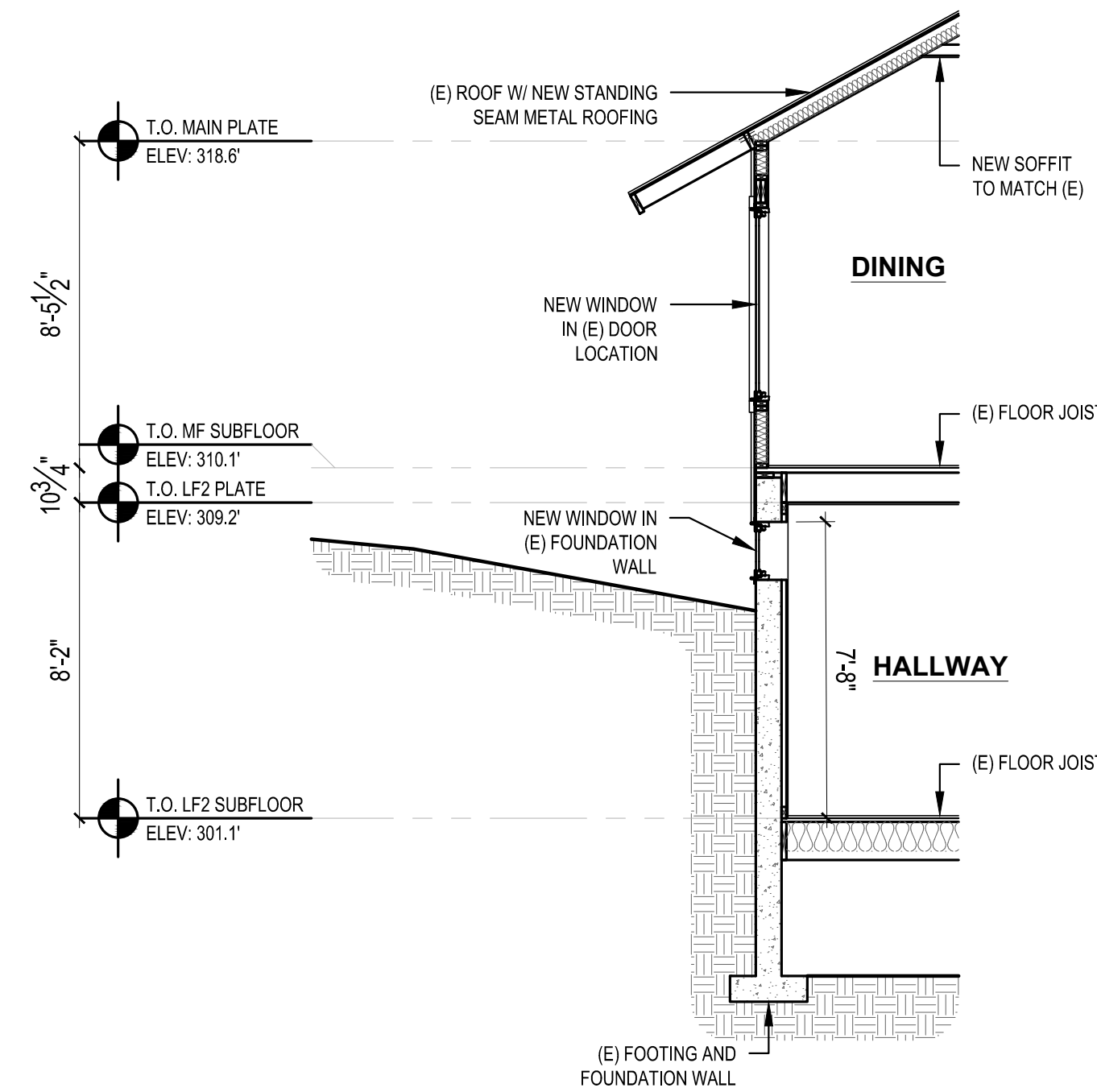
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**DOOR TYPES:**

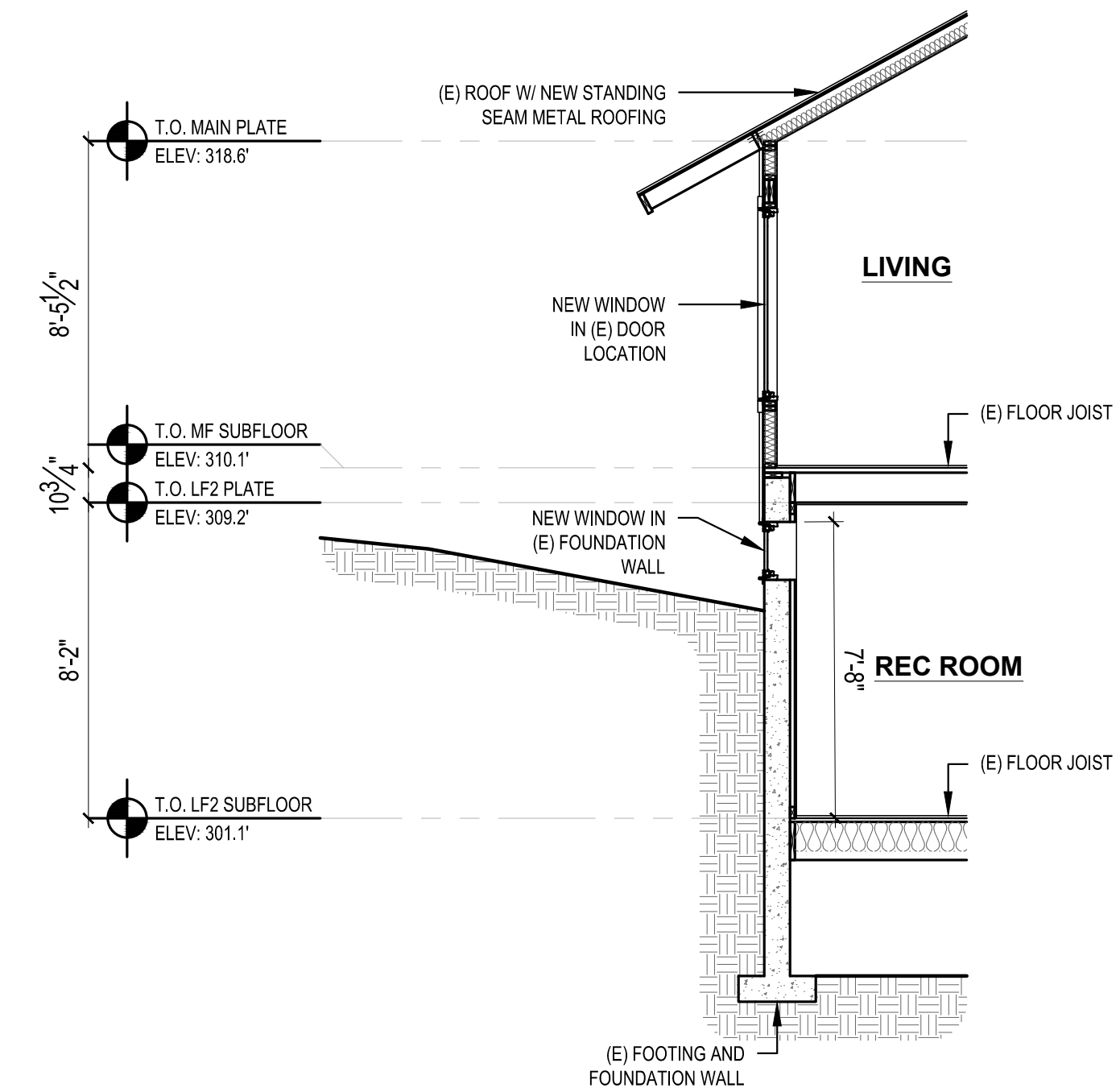


**DOOR SCHEDULE**

DOOR NO.	LOCATION	SIZE WIDTH	SIZE HEIGHT	DOOR TYPE	TEMP. GLASS	DOOR THK.	U-VAL (MIN.)	REMARKS
<b>LOWER FLOOR</b>								
001	BED 1	5' - 0"	6' - 8"	C	-	1-3/4"	-	
002	BED 2	5' - 0"	6' - 8"	C	-	1-3/4"	-	
103	BED 3	5' - 0"	6' - 8"	C	-	1-3/4"	-	
104	BATH 1	2' - 6"	6' - 8"	D	-	1-3/4"	-	
105	HALL CLOSET	2' - 6" PR	6' - 8"	E	-	1-3/4"	-	
106	EXERCISE	12' - 0"	6' - 8"	F	Y	1-3/4"	0.28	
107	EXERCISE	6' - 0"	6' - 8"	G	-	1-3/4"	-	
108	LAUNDRY	2' - 10"	6' - 8"	A	-	1-3/4"	-	
109	STAIR CLOSET	2' - 6"	6' - 8"	A	-	1-3/4"	-	
110	MECH AND STORAGE	2' - 10"	6' - 8"	A	-	1-3/4"	-	
111	BATH 2	10' - 0" PR	6' - 8"	C	-	1-3/4"	-	
112	BATH 2	2' - 10"	6' - 8"	H	Y	1-3/4"	0.28	
<b>MAIN FLOOR</b>								
201	ENTRY DOOR	5' - 0"	7' - 0"	B	-	1-3/4"	-	
202	ENTRY CLOSET	2' - 0" PR	7' - 0"	E	-	1-3/4"	-	
203	OFFICE	2' - 6"	6' - 8"	A	-	1-3/4"	-	
204	POWDER	2' - 6"	6' - 8"	A	-	1-3/4"	-	
205	HALL CLOSET	2' - 0" PR	6' - 8"	E	-	1-3/4"	-	
206	LIVING ROOM	16' - 0"	8' - 0"	I	Y	1-3/4"	0.28	
207	DINING ROOM	8' - 6"	6' - 8"	F	Y	1-3/4"	0.28	
208	PANTRY	2' - 6"	6' - 8"	D	-	1-3/4"	-	
209	MUDROOM	2' - 10"	6' - 8"	A	-	1-3/4"	-	
210	MUDROOM	2' - 10"	6' - 8"	A	-	1-3/4"	-	20 MIN FIRE RATED DOOR
211	GARAGE	3' - 0"	7' - 0"	A	-	1-3/4"	-	
212	GARAGE	9' - 0"	8' - 0"	E	-	1-3/4"	-	
213	GARAGE	9' - 0"	8' - 0"	E	-	1-3/4"	-	
<b>UPPER FLOOR</b>								
201	VESTIBULE	2' - 8"	7' - 0"	A	-	1-3/4"	-	
202	PRIMARY BEDROOM	2' - 8"	7' - 0"	A	-	1-3/4"	-	
203	PRIMARY BATH LINEN	2' - 6"	7' - 0"	A	-	1-3/4"	-	
204	PRIMARY BATH	2' - 6"	7' - 0"	G	-	1-3/4"	-	
205	TOILET ROOM	2' - 6"	7' - 0"	A	-	1-3/4"	-	
206	PRIMARY CLOSET	2' - 6"	7' - 0"	A	-	1-3/4"	-	



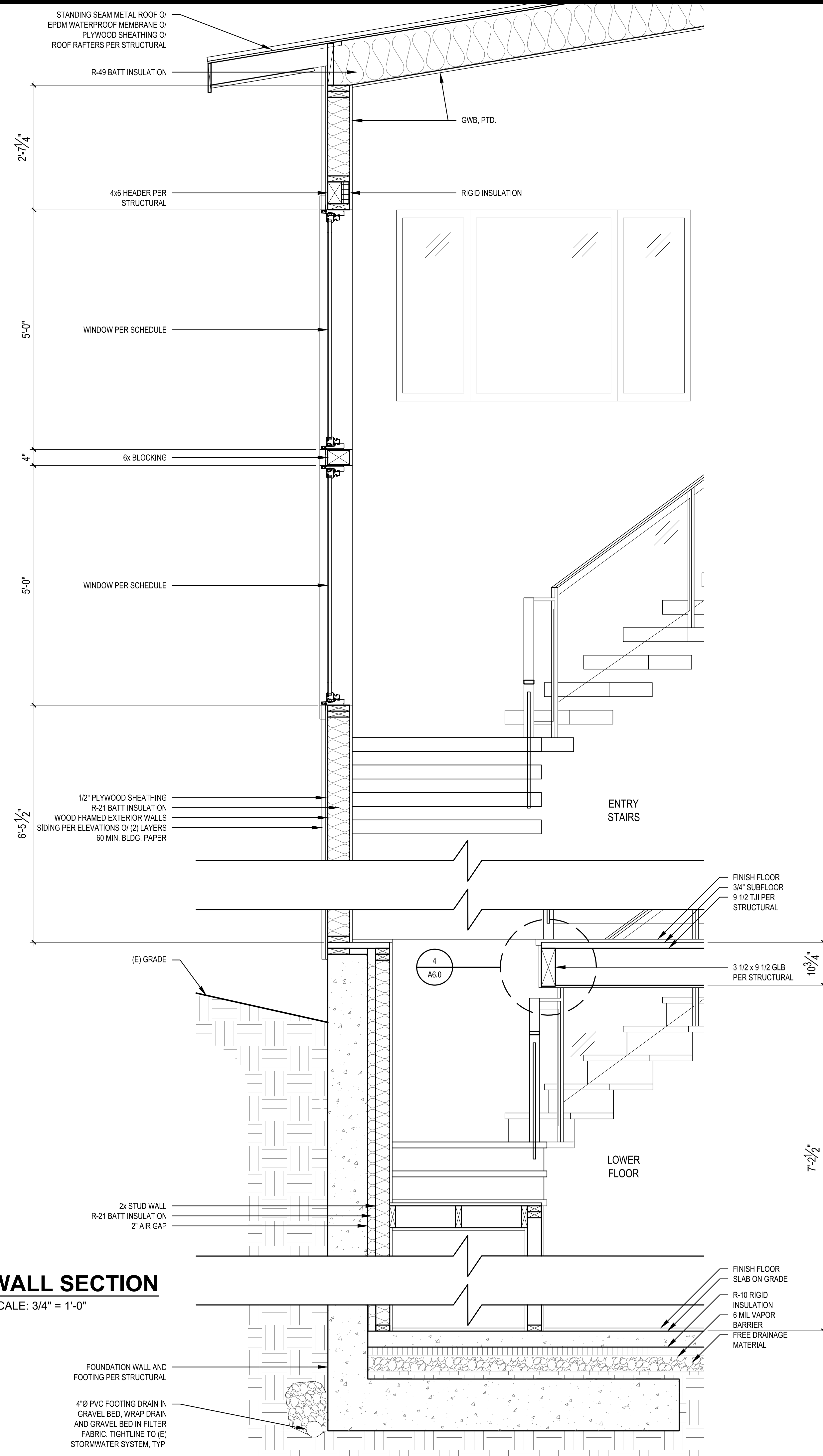
**9 BUILDING SECTION**  
 SCALE: 1/4" = 1'-0"



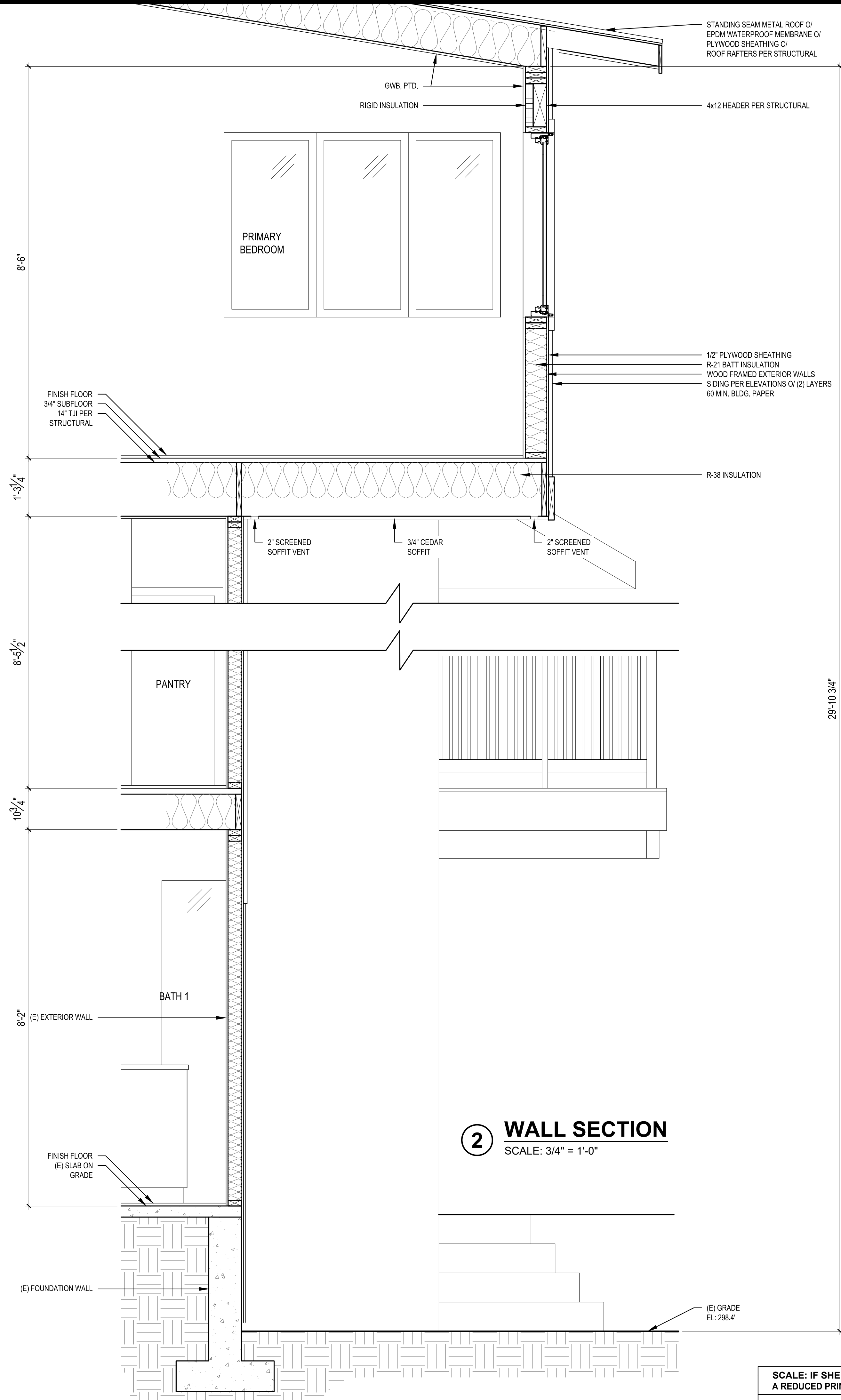
**10 BUILDING SECTION**  
 SCALE: 1/4" = 1'-0"

**WINDOW SCHEDULE**

TAG.	DESCRIPTION	WINDOW SIZE		TEMP.	QTY.	AREA (SF)	U-VAL (MIN.)	GLAZING	REMARKS & NOTES
		WIDTH	HEIGHT						
A	PICTURE	4' - 0"	5' - 0"	-	1	20	0.28	LOW E / CLEAR	
B	SIDELIGHT	2' - 0"	7' - 0"	Y	1	14	0.28	LOW E / CLEAR	
C	PICTURE	7' - 0"	5' - 0"	-	1	35	0.28	LOW E / CLEAR	
D	CASEMENT	4' - 0"	3' - 0"	-	2	24	0.28	LOW E / CLEAR	
E	PICTURE	3' - 6"	4' - 0"	-	2	28	0.28	LOW E / CLEAR	
F	PICTURE	5' - 0"	5' - 0"	Y	2	50	0.28	LOW E / CLEAR	
G	FIXED	2' - 0"	4' - 0"	-	2	16	0.28	LOW E / CLEAR	
H	PICTURE	4' - 0"	4' - 0"	-	1	16	0.28	LOW E / CLEAR	
I	CASEMENT	2' - 0"	3' - 4"	-	3	20	0.28	LOW E / CLEAR	
J	PICTURE	3' - 0"	3' - 4"	-	1	10	0.28	LOW E / CLEAR	
K	FIXED	3' - 0"	4' - 0"	-	3	36	0.28	LOW E / CLEAR	
L	PICTURE	2' - 6"	5' - 0"	-	4	50	0.28	LOW E / CLEAR	
M	PICTURE	4' - 0"	1' - 6"	-	2	12	0.28	LOW E / CLEAR	
N	TRANSOM	4' - 0"	2' - 0"	-	2	16	0.28	LOW E / CLEAR	
O	PICTURE	2' - 0"	4' - 0"	-	3	24	0.28	LOW E / CLEAR	
P	PICTURE	2' - 6"	4' - 0"	Y	2	20	0.28	LOW E / CLEAR	
Q	PICTURE	2' - 0"	1' - 6"	-	4	12	0.28	LOW E / CLEAR	
R	SKYLIGHT	4' - 0"	4' - 0"	Y	1	16	0.5	LOW E / CLEAR	
S	AWNING	4' - 0"	1' - 6"	Y	1	12	0.28	LOW E / CLEAR	
T	CASEMENT	3' - 0"	4' - 0"	-	7	84	0.28	LOW E / CLEAR	EGRESS IN SOME LOCATIONS
U	FIXED	2' - 0"	3' - 6"	-	1	7	0.28	LOW E / CLEAR	
V	CASEMENT	2' - 0"	3' - 6"	-	1	7	0.28	LOW E / CLEAR	
W	CASEMENT	2' - 0"	4' - 0"	-	1	8	0.28	LOW E / CLEAR	
X	AWNING	3' - 0"	1' - 6"	Y	1	4.5	0.28	LOW E / CLEAR	
Y	CASEMENT	3' - 0"	4' - 0"	-	3	36	0.28	LOW E / CLEAR	
Z	PICTURE	4' - 0"	6' - 0"	Y	4	96	0.28	LOW E / CLEAR	
AA	CASEMENT	2' - 0"	3' - 6"	Y	4	28	0.28	LOW E / CLEAR	
BB	PICTURE	3' - 0"	3' - 6"	Y	2	21	0.28	LOW E / CLEAR	



**1 WALL SECTION**  
SCALE: 3/4" = 1'-0"

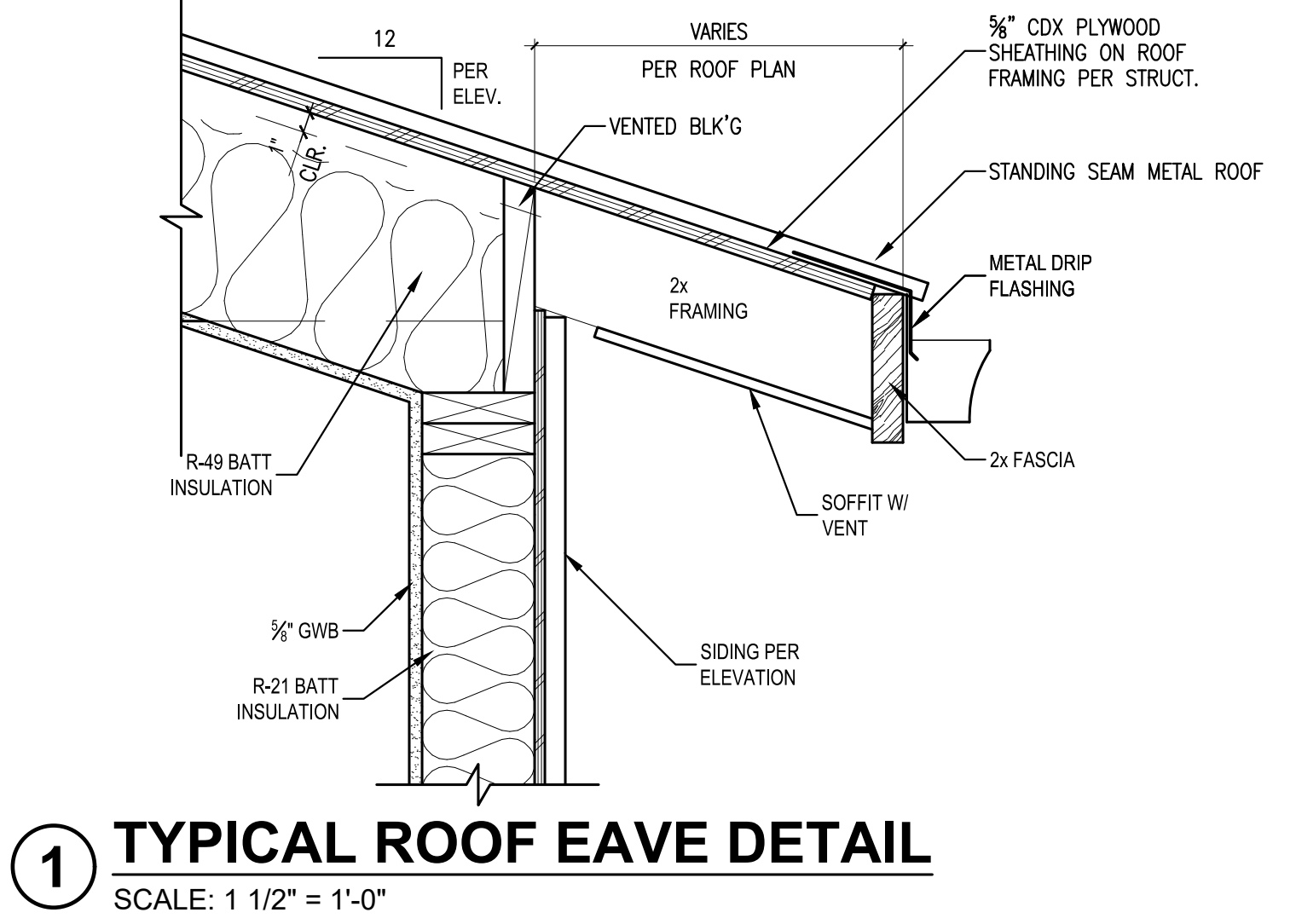


**2 WALL SECTION**  
SCALE: 3/4" = 1'-0"

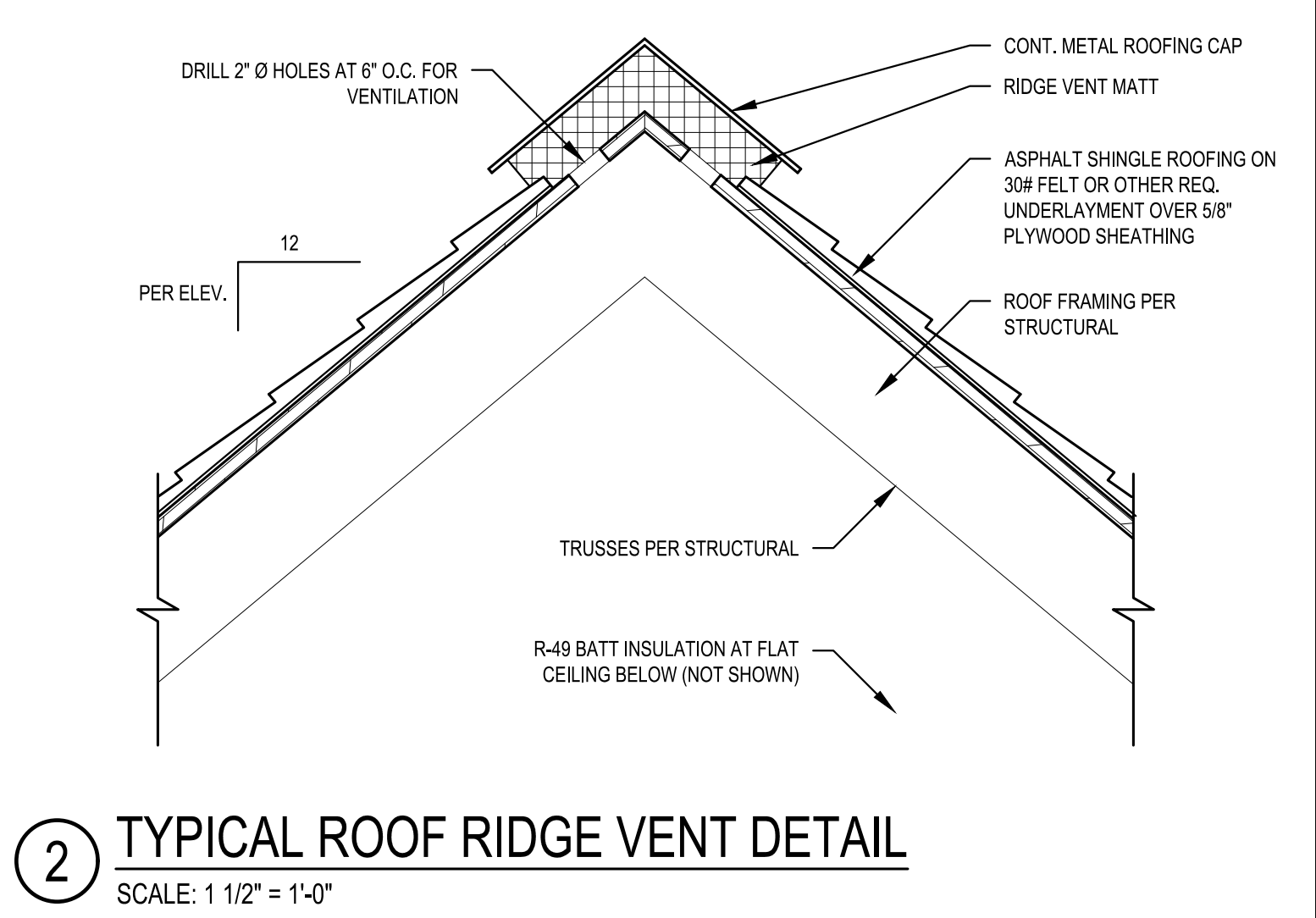
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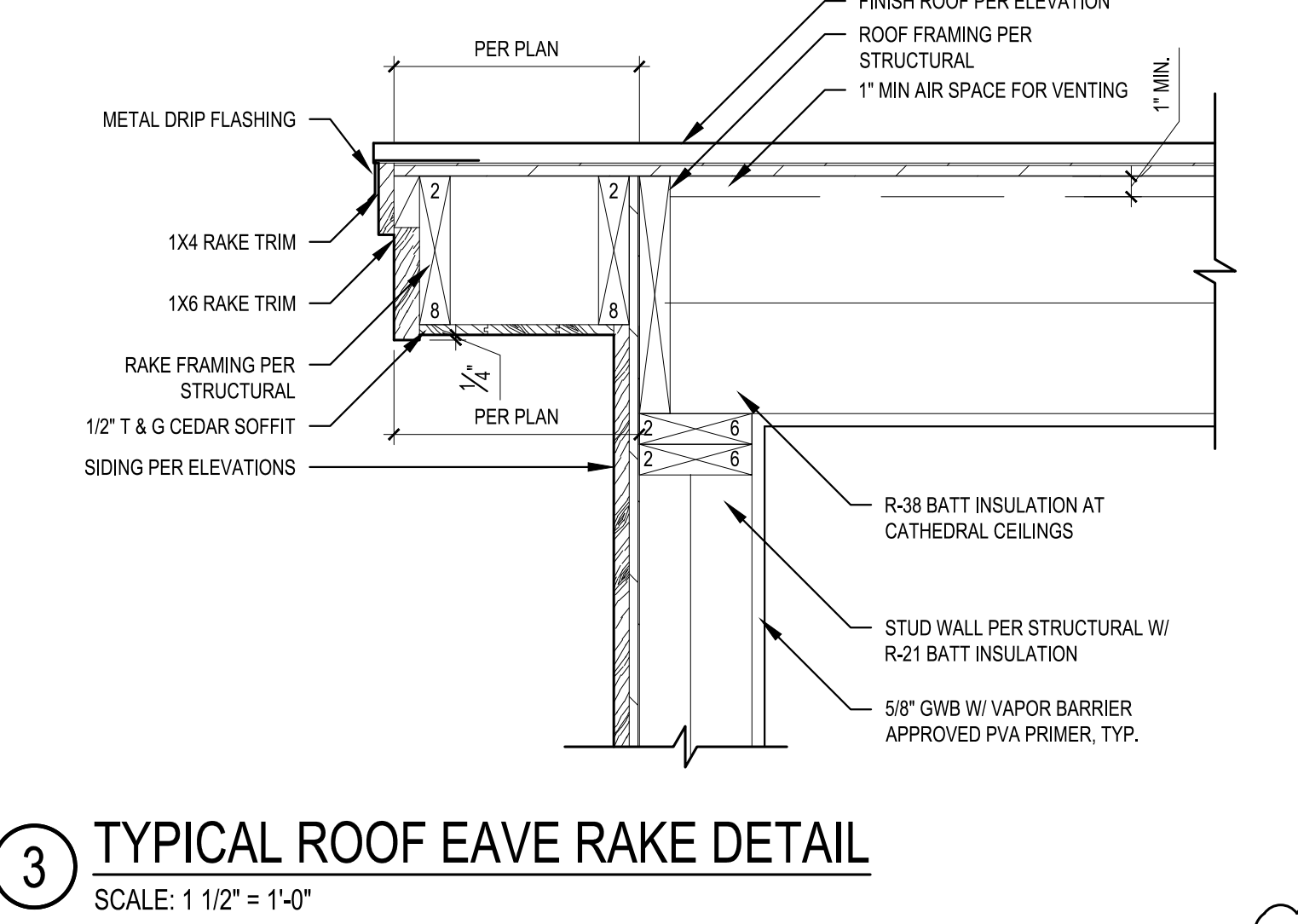
29'-10 3/4"  
MAX DOWNHILL FACADE HEIGHT: 30'-0"



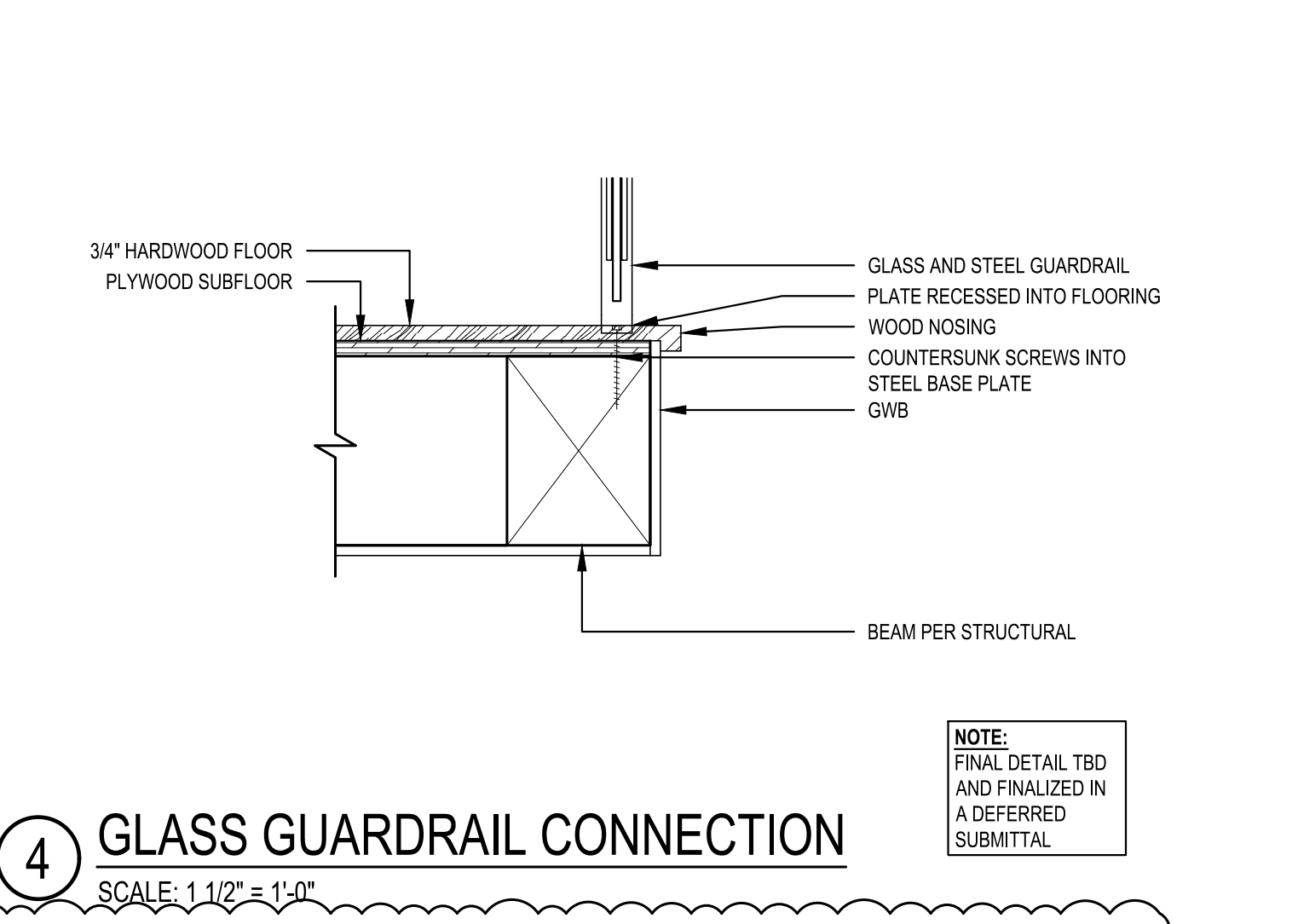
**1 TYPICAL ROOF EAVE DETAIL**  
SCALE: 1 1/2" = 1'-0"



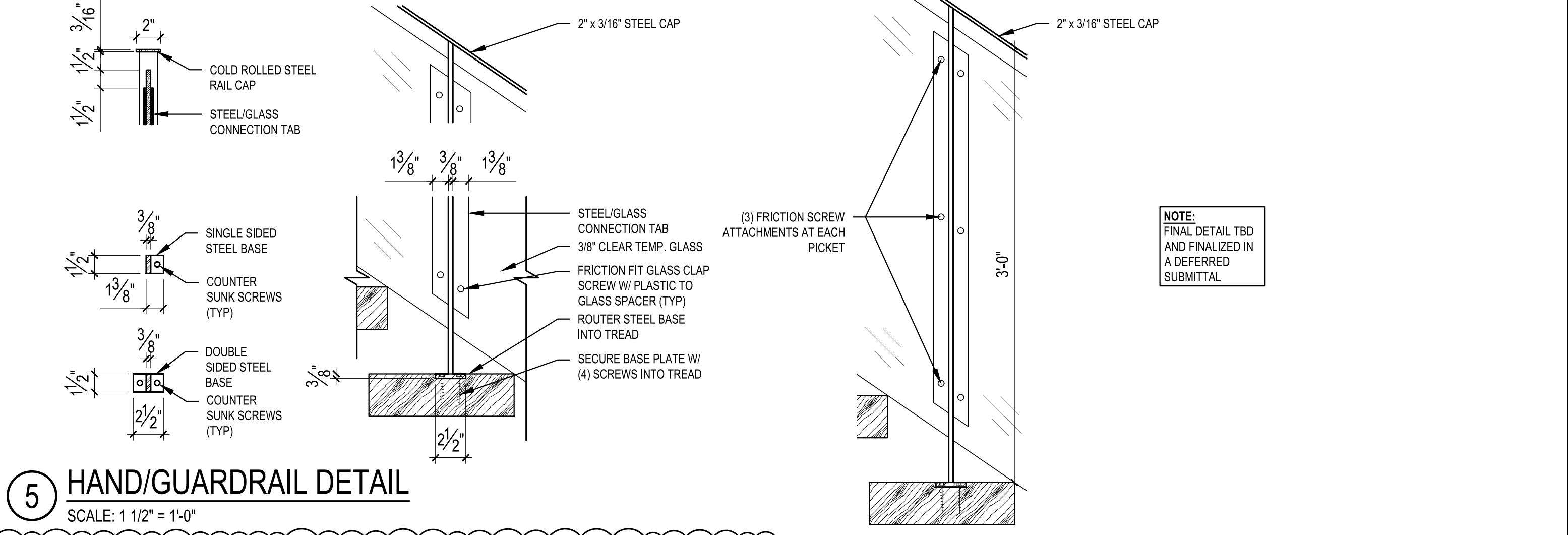
**2 TYPICAL ROOF RIDGE VENT DETAIL**  
SCALE: 1 1/2" = 1'-0"



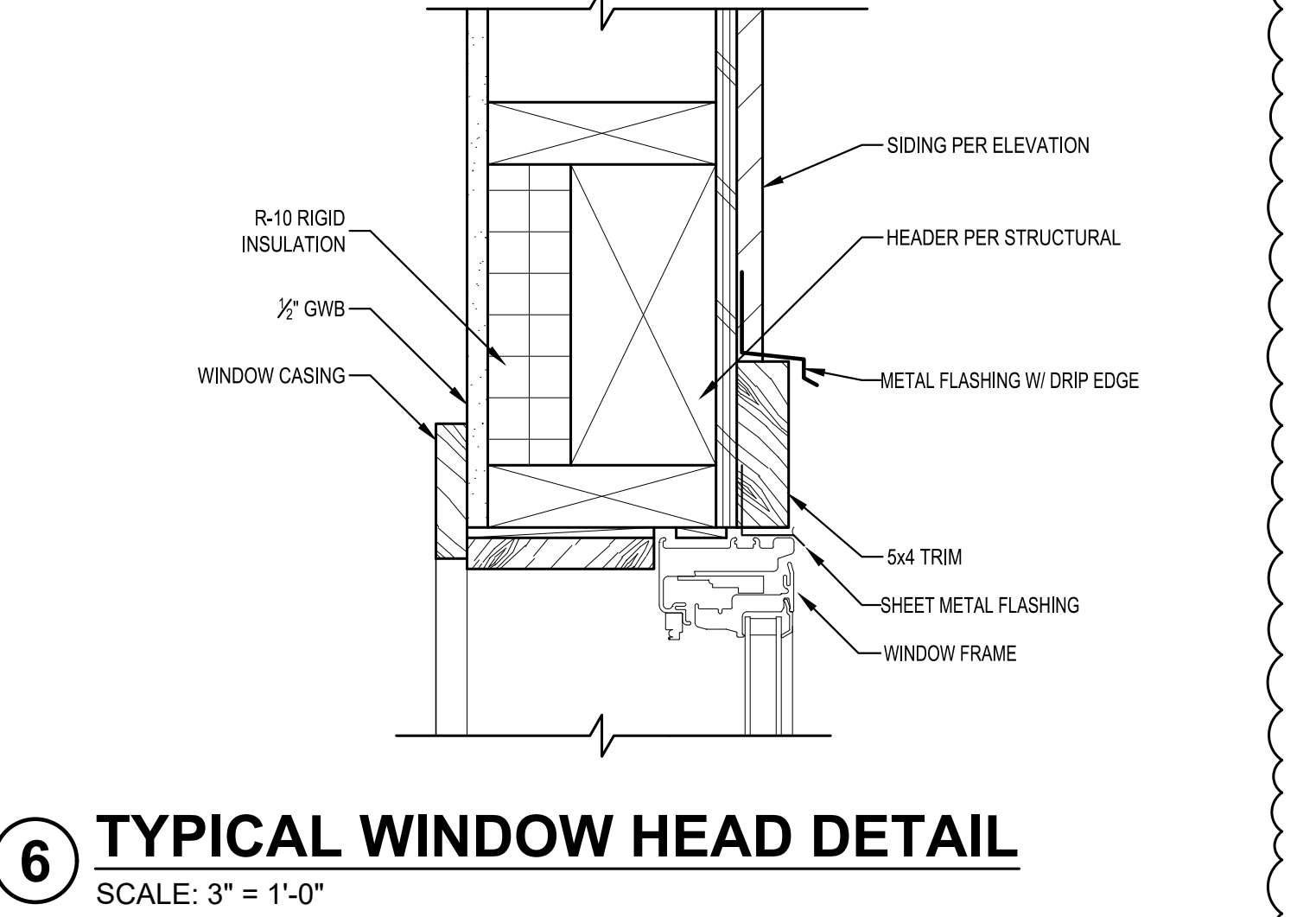
**3 TYPICAL ROOF EAVE RAKE DETAIL**  
SCALE: 1 1/2" = 1'-0"



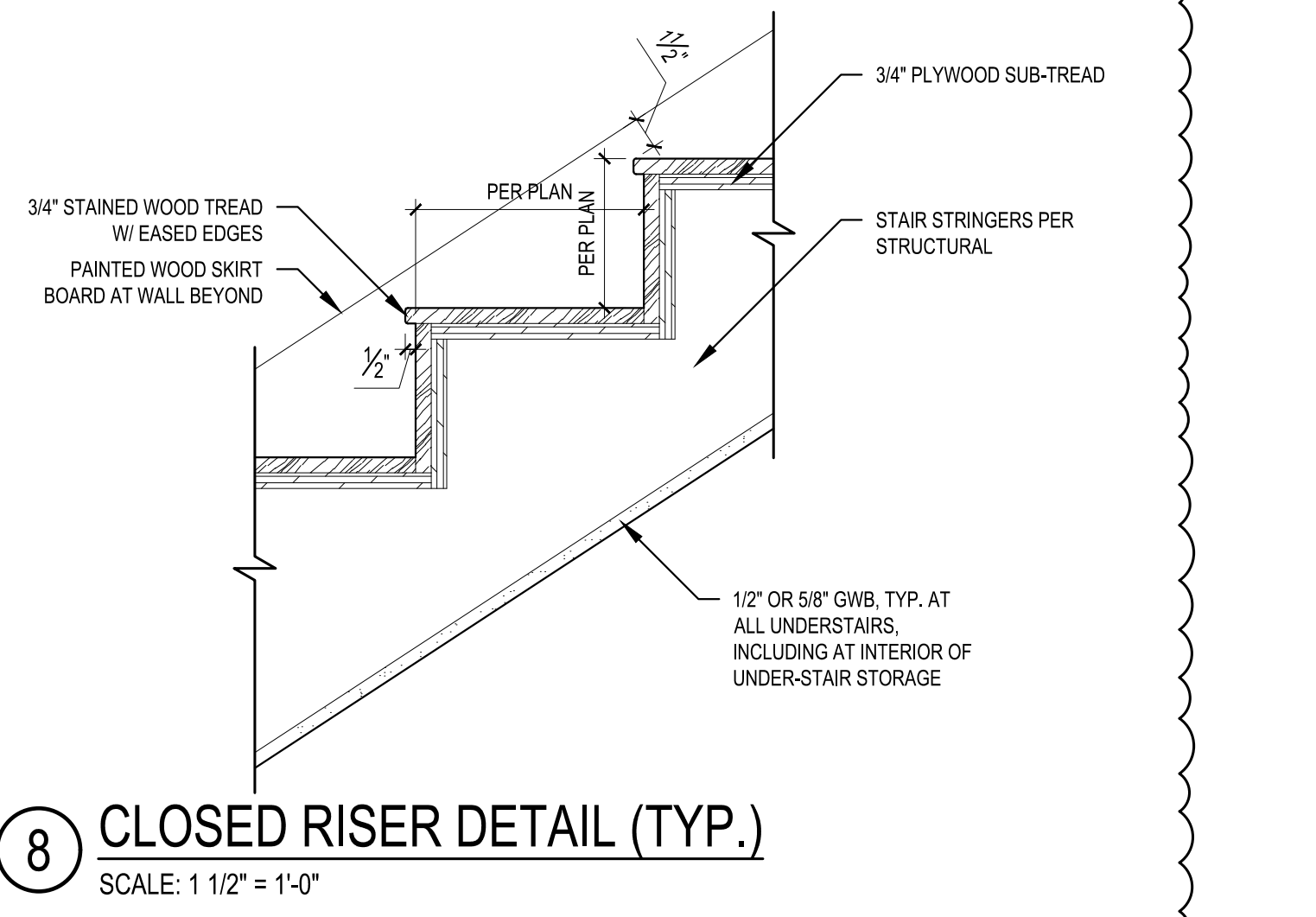
**4 GLASS GUARDRAIL CONNECTION**  
SCALE: 1 1/2" = 1'-0"



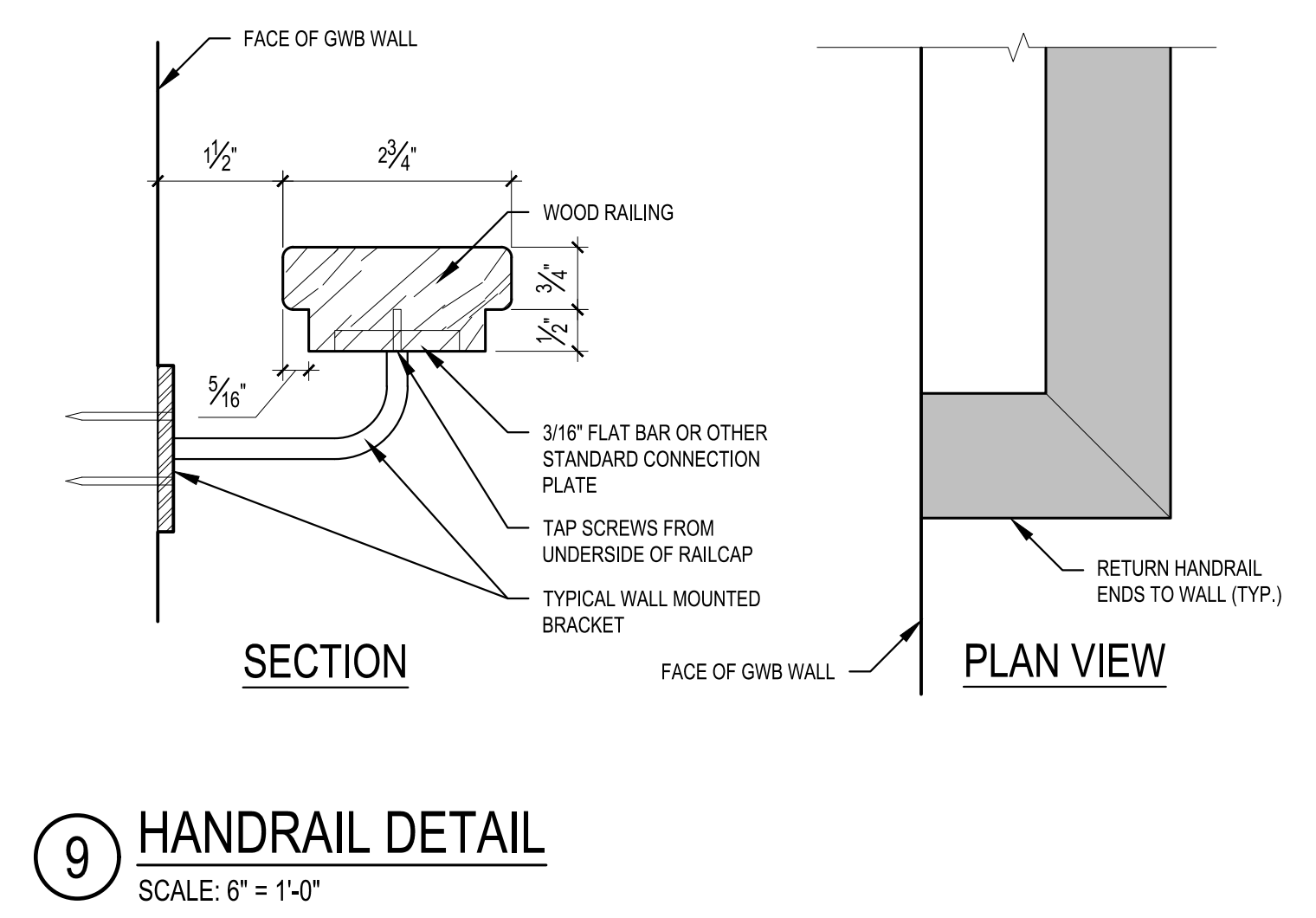
**5 HAND/GUARDRAIL DETAIL**  
SCALE: 1 1/2" = 1'-0"



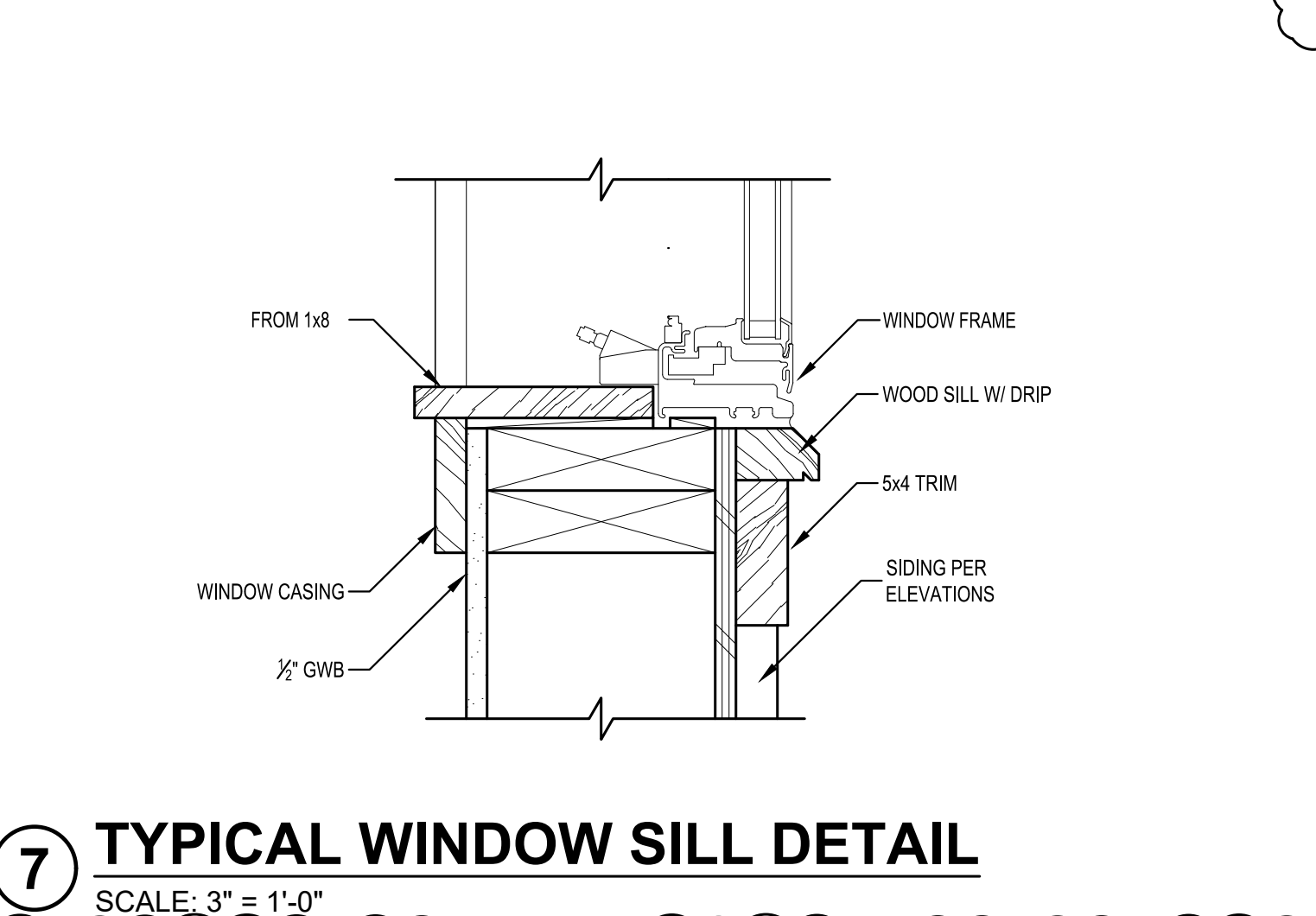
**6 TYPICAL WINDOW HEAD DETAIL**  
SCALE: 3" = 1'-0"



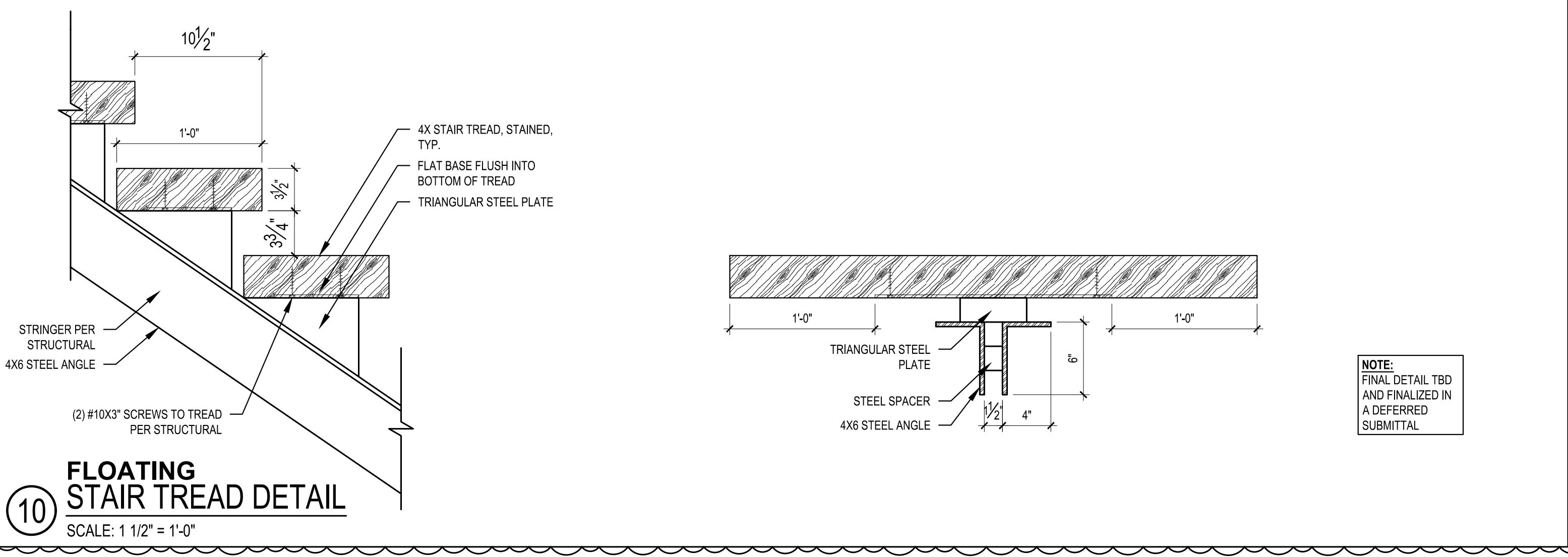
**8 CLOSED RISER DETAIL (TYP.)**  
SCALE: 1 1/2" = 1'-0"



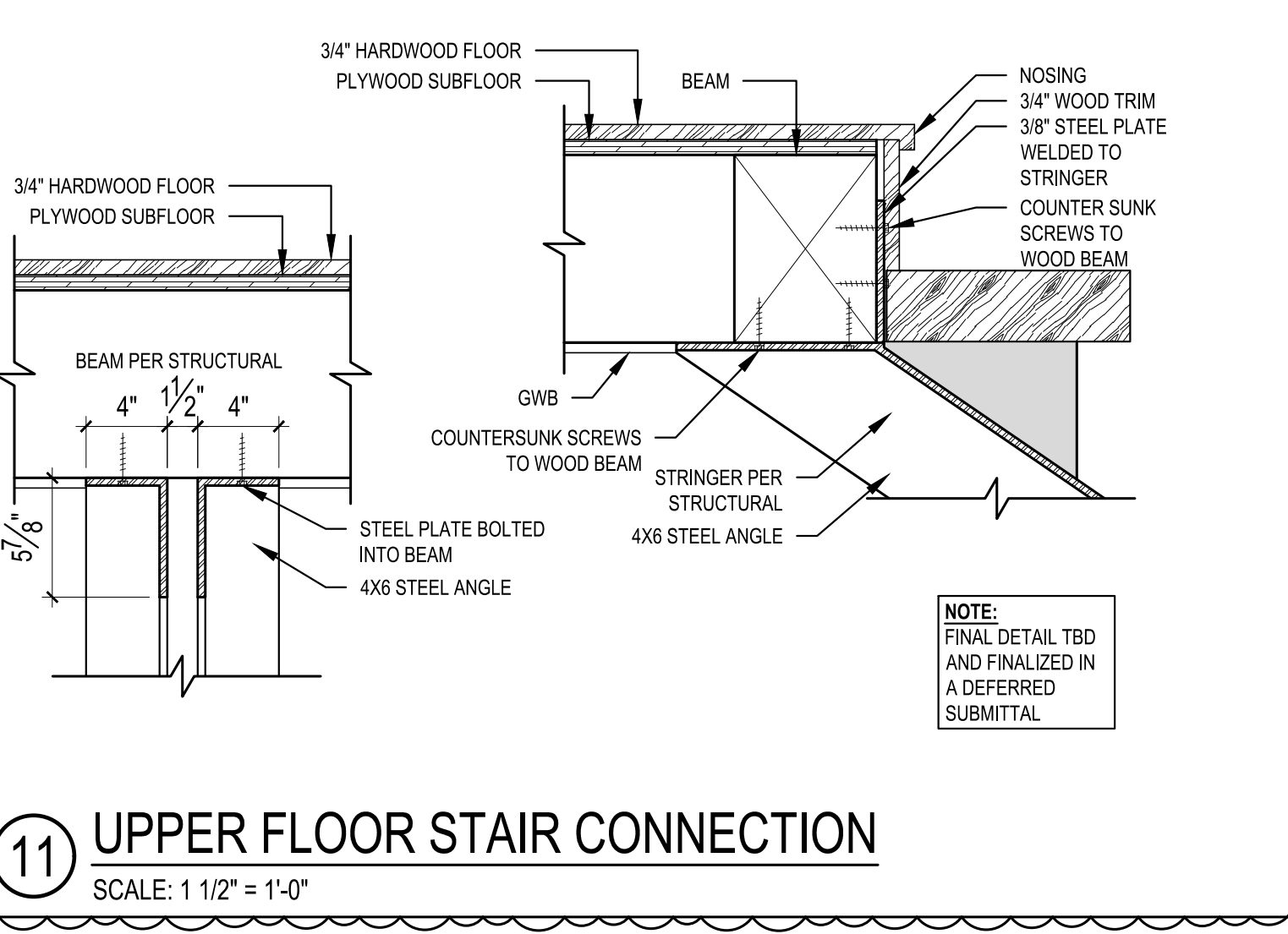
**9 HANDRAIL DETAIL**  
SCALE: 6" = 1'-0"



**7 TYPICAL WINDOW SILL DETAIL**  
SCALE: 3" = 1'-0"

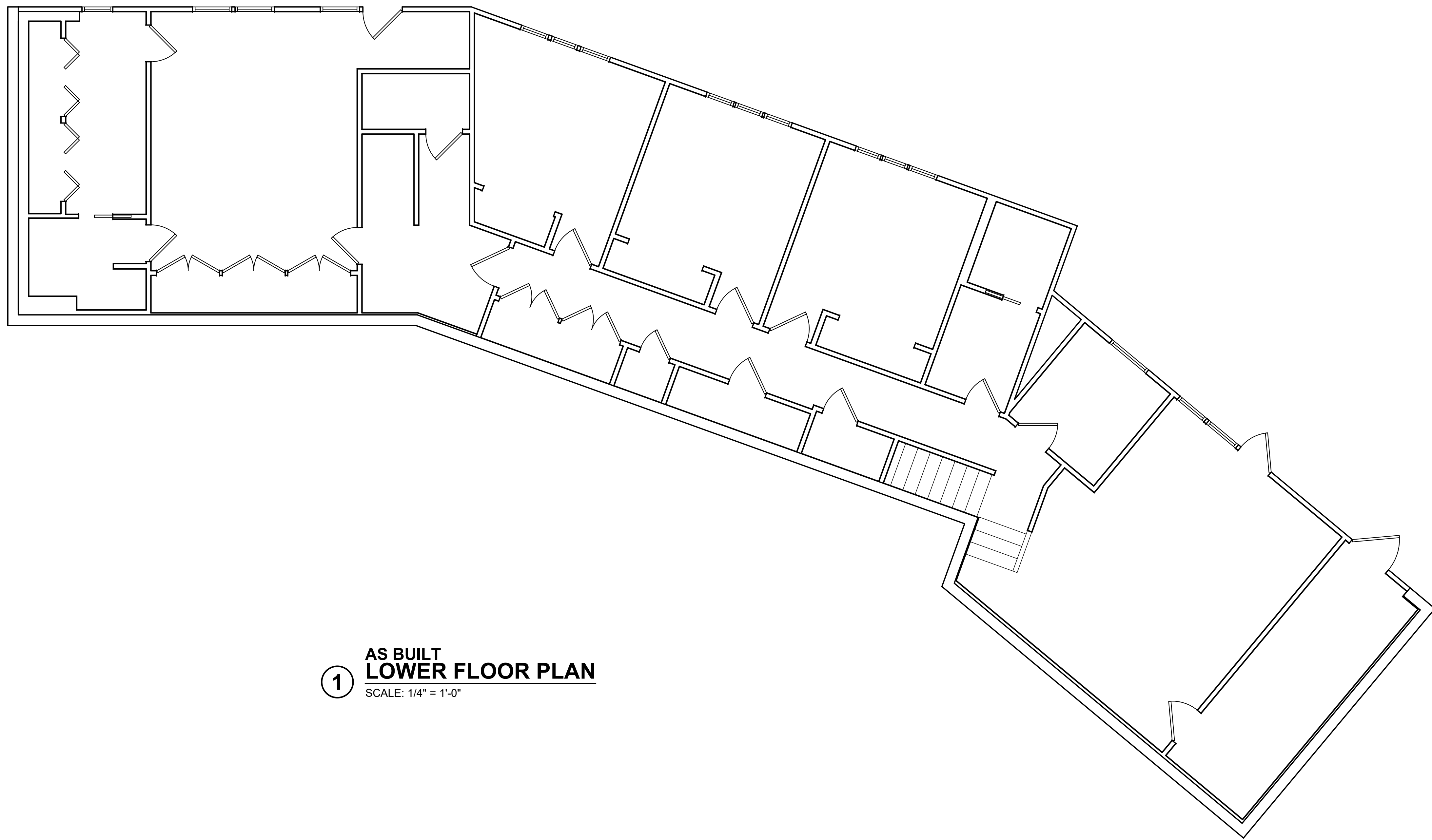


**10 FLOATING STAIR TREAD DETAIL**  
SCALE: 1 1/2" = 1'-0"



**11 UPPER FLOOR STAIR CONNECTION**  
SCALE: 1 1/2" = 1'-0"

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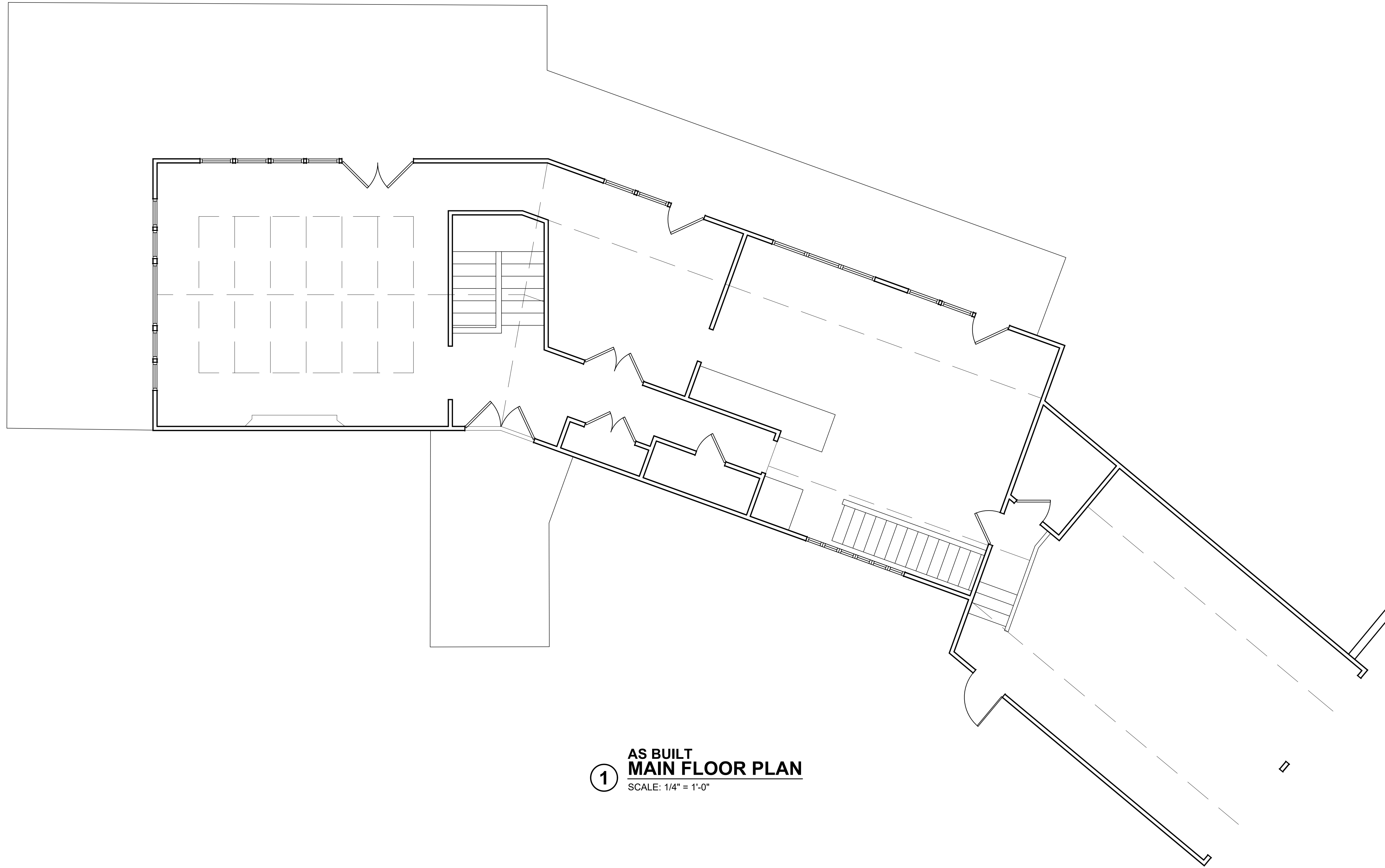


**1 AS BUILT LOWER FLOOR PLAN**  
 SCALE: 1/4" = 1'-0"

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**1 AS BUILT  
MAIN FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

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**General Requirements**

All materials, workmanship, design and construction shall conform to the 2018 International Building Code (IBC) and local jurisdiction amendments.

Definitions: The following definitions are used throughout these structural notes:

- IBC - Governing code including local amendments
SER - Structural Engineer of Record per these Contract Documents
UNO - Unless otherwise noted

Drawings indicate general and typical details of construction. Typical details and general notes shall apply even if not specifically denoted on plans, UNO. Where conditions are not specifically indicated similar details of construction shall be used, subject to review and approval by the Architect and the SER.

Reference to ASTM and other standards shall refer to the latest edition designated by IBC Chapter 35. Refer to the specifications for information in addition to that covered by these structural notes & drawings.

Warranty: The SER has used that degree of care and skill ordinarily exercised under similar circumstances by members of the profession in this locale and no other warranty, either expressed or implied, is made in connection with rendering professional services.

**Design Criteria**

BUILDING CATEGORY: Structural Occupancy Category II (Importance factors listed below)

LIVE LOADS:
Roof snow load, Pf = 25 psf

Table with 2 columns: Residential location and Load (psf). Includes Uninhabitable attics, Habitable attics, and Residential floor.

LATERAL LOADS-WIND: Per ASCE 7-16, Section 27.5
Iw = 1.0; Kzt = 1.90; V = 78.3 kips

Numbering below is per IBC Section 1603.1.4:

- 1. Basic Wind Speed (3-second gust) = 110 mph
2. Importance Factor = 1.0
3. Exposure = C
4. Internal pressure coefficient = +/- 0.18
5. Components and Cladding: The following working loads may be used in lieu of calculations:

Table with 3 columns: Uplift at roof, Roof overhangs, and Walls. Includes Zone designations and load values.

LATERAL LOADS-EARTHQUAKE: Per ASCE 7-16, Chapter 11 & IBC 1613

Numbering below is per IBC Section 1603.1.5:

- 1. Importance Factor = 1.0
2. Mapped Spectral Response Accelerations, Ss = 1.466 g; S1 = 0.505 g
3. Site Class = D; Fa = 1.200, Fv = 1.795
4. Spectral Response Coefficients, Sds = 1.173 g, Sd1 = 0.6040 g
5. Seismic Design Category = D
6. Basic Seismic Force Resisting System is:
Vertical Elements = Wood Structural Panel Shear Walls
Diaphragms = Wood Structural Panel Diaphragms
7. Design Base Shear = 21.6 kips
8. Seismic Response Coefficient Cs = 0.180
9. Response Modification Factor R = 6.5
10. Analysis Procedure = Equivalent Lateral Force Procedure

Table with 2 columns: Building Location, Building Height, and Redundancy Factors. Includes values for Building Location (47,529 N, 122,227 W) and Redundancy Factors (North/South Direction = 1.0, East/West Direction = 1.0).

**Contractor Execution Requirements**

Contractor shall verify all dimensions and all conditions at the job site, including building and site conditions before commencing work, and be responsible for same. All discrepancies shall be reported to the Architect/SER before proceeding with work.

Contractor shall coordinate all dimensioned openings and slab edges shown on the contract documents. Some dimensions, openings and embedded items are shown on the structural drawings, others may be required. Refer to architectural drawings for all dimensions, wall and floor openings, architectural treatment, embeds required for architectural items, etc. Refer to mechanical, plumbing, electrical, fire protection and civil drawings for size and location of all openings for ducts, piping, conduits, etc.

Do not scale drawings. Use only field verified dimensions. When electronic plan files are provided for the contractor's detailing convenience, it shall be noted that the electronic files are not guaranteed to be dimensionally accurate; the contractor uses them at their own risk. The published paper documents are the controlling Contract Documents. Electronic files of detail sheets and notes will not be provided.

Contract Documents and any materials used in preparation of them, including calculations, are the exclusive property of the SER and can be reproduced only with the permission of the SER.

Contractor initiated changes shall be submitted in writing to the Architect/SER for review and acceptance prior to fabrication/construction. Changes shown on shop drawings only will not satisfy this requirement.

The contractor shall provide temporary bracing as required until all permanent connections have been installed. The contractor is responsible for the strength and stability of all partially completed structures including but not limited to concrete or masonry walls, steel framing and erection aids.

**Shop Drawing & Submittal Review**

The contractor shall review and stamp the shop drawings & submittals for review. SER will only review submittals for items shown on SER documents. Submittals for Deferred Structural Components will receive cursory review by SER for loads imposed on primary structure. SER will review shop drawings for general conformance with design concept of the project and general compliance with the information given in the Structural Contract Documents.

**GENERAL STRUCTURAL NOTES**  
(TYPICAL UNLESS NOTED OTHERWISE ON DRAWINGS)

**Shop Drawing & Submittal Review** (including Deferred Structural Components)

The contractor shall review and stamp the shop drawings & submittals for review. SER will only review submittals for items shown on SER documents. Submittals for Deferred Structural Components will receive cursory review by SER for loads imposed on primary structure. SER will review shop drawings for general conformance with design concept of the project and general compliance with the information given in the Structural Contract Documents.

Corrections or comments made on shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications.

Contractor responsible for:

- \* Reviewing, approving, stamping and signing submittals prior to submittal to Architect and SER
\* Timing submittals to allow 10 days of review time for the SER and time for corrections and resubmittal
\* Conformance to requirements of the Contract Documents
\* Dimensions and quantities
\* Verifying information to be confirmed or coordinated
\* Information solely for fabrication, safety, means, methods, techniques and sequences of construction
\* Coordination of all trades

Resubmittals shall be clouded and dated for all changes to the submittal. Only clouded portions of resubmittal will be reviewed and SER's review stamp applies to only these areas.

**Substitutions**

Substitutions shall be submitted in writing prior to submittal of shop drawings. Shop drawings bearing substitutions will be rejected. Submit engineering data to substantiate the equivalence of the proposed items. The SER's basic services contract does not include review of substitutions that require re-engineering of the item or adjacent structure.

**Submittals**

Shop drawings and material submittals shall be submitted to the Architect and SER prior to any fabrication or construction for the following structural items. Submittals shall include one reproducible and one copy; reproducible will be marked and returned. If deviations, discrepancies, or conflicts between shop drawings submittals and the contract documents are discovered either prior to or after shop drawing submittals are processed by the SER, the Contract Documents control and shall be followed.

- \* Structural steel shop and erection drawings
\* 1-joint and engineered wood beam floor framing layout & materials list
\* Glued laminated members (certificates to be on-site and available upon request)
\* Engineered wood beams (certificates to be on-site and available upon request)

**Inspection**

The building official, upon notification, shall make structural inspections as required by local ordinance. The inspection by the building official per IBC Section 109 will be separate from and in addition to the special inspection and structural observation mentioned subsequently.

**Special Inspections**

The owner shall retain a Special Inspector to perform the special inspection requirements required by the building official as outlined in IBC Section 1704. See the specifications for additional requirements for special inspection and testing. The architect, structural engineer, and building department shall be furnished with copies of all inspection reports and test results.

The following inspections are required and shall be performed per the building code:

- \* Steel construction per 1705.2 and AISC 360
\* Epoxy installed anchor bolts and holdowns rods: Continuous per Table 1703.3 - #4

**Structural Observation**

Structural observation is defined as the visual observation of the structural system for general conformance to the Contract Documents at significant construction stages and at completion of the structural system. Structural observation does not include or waive the responsibility for the inspection required by Section 109 or other sections of the IBC.

The owner shall employ a registered design professional to perform structural observation when required by IBC 1709. Observed deficiencies shall be reported in writing to the Architect, special inspector, and contractor. The contractor shall respond to these items in writing indicating how they have been resolved. At the end of the project, the structural observer shall submit to the building official a written statement that the site visits have been made and identify any reported deficiencies that, to the best of the structural observer's knowledge, have not been resolved.

Construction observation by the SER is for general conformance with structural portions of the permit documents only and is not intended in any way to review the Contractor's construction procedures. The SER has no overall supervisory authority or actual/direct responsibility for the specific working conditions at the site and for any hazards resulting from the action of any trade contractor. The SER has no duty to inspect, supervise, note, correct, or report any health or safety deficiencies to the owner, contractors, or other entities or persons at the project site.

The contractor shall provide the SER adequate notice to schedule appropriate site visits for structural observation.

**Geotechnical**

**General Criteria**

Allowable soil pressure and lateral earth pressure are assumed and therefore must be verified by a Geotechnical Inspector or the building official. If soils are found to be other than assumed, notify the structural engineer for possible foundation redesign.

All prepared soil-bearing surfaces shall be inspected by the owners Geotechnical Inspector (or building official) prior to placement of reinforcing steel and concrete. Inspections shall be made per IBC Table 1704.7.

Unless otherwise noted, footings shall be centered below columns or walls.

**Bearing Values**

Allowable soil pressure = 2,000 psf

All footings shall bear on undisturbed soil and shall be lowered to firm bearing if suitable soil is not found at elevations shown. Exterior footings shall bear a minimum of 12" below the finished ground surface. Footing elevations shown on plans (or in details) are minimum depths and for guidance only; the actual elevations of footings must be established by the contractor in the field working with the Geotechnical Inspector.

**Subgrade Preparation**

Prepare subgrade as follows: All footings shall be cast on undisturbed firm natural soils that are free of organic materials. Footing excavation shall be free of loose soils, sloughs, debris and free of water at all times. If organic silt and/or fill material is encountered at subgrade elevations, over-excavate a minimum of 2'-0" below the design foundation subgrade elevation prior to placing footings. The over-excavated areas shall be backfilled with structural fill compacted to 95% proctor per ASTM D-1557 or a lean concrete mix.

**Drainage**

Drainage systems, including foundation, roof and surface drains, shall be installed as directed by the Geotechnical Report and IBC Section 1807. Vapor retarder placed below slab on grade shall conform to ASTM E 1643 and ASTM E 745.

**Retaining Walls**

Grade on either side of concrete walls shall not vary by more than 12", UNO. Slope of backfill shall not exceed 2H to 1V, UNO. Backfill behind all retaining walls with free draining, granular fill. Provide for subsurface drainage. Design pressures used for the design of retaining walls are based on drained conditions.

Active earth pressure (restrained/unrestrained) = 55/35 pcF
Passive earth pressure (factor of safety of 1.5 included) = 300 pcF
Coefficient of friction (factor of safety of 1.5 included) = 0.35

Provide temporary shoring for tops of walls if backfill is placed prior to the supporting structure being constructed. Supporting structure is the floor framing and sheathing completely installed and attached to perpendicular walls.

**Existing Utilities**

The contractor shall determine the location of all adjacent underground utilities prior to any excavation, shoring, pile driving, or pier drilling. Any utility information shown on the plans and details are approximate and not verified by the SER. Contractor is to provide protection of any utilities or underground structures during construction.

**Concrete**

**Cast-in-Place Concrete**

Concrete materials shall conform to the following:
Portland cement: Type 1, ASTM C150
Fly ash (if used): ASTM C618 class F or C, quantity less than (by weight) 25% of cement content, and maximum loss on ignition = 1%
Lightweight aggregates: shall not be used without prior approval of SER and building department
Normal weight aggregates: ASTM C33
Sand equivalent: ASTM C33
Water: Potable per ASTM C94
Air entraining admixtures: ASTM C260
Chemical admixtures: ASTM C494
Flowable concrete admixtures: ASTM C1017

Durability requirements of concrete mixes shall conform to building code. These requirements include water-cementitious material ratios, minimum compressive strengths, air entrainment, type of cement, and maximum chloride ion content.

Concrete strength requirements: Strength at 28 days and normal weight concrete, UNO.

Table with 4 columns: Location, Strength f'c (psi), Max. Aggr. size (inch), and Max. W/C ratio or min cement \*. Includes entries for Lean mix soil replacement, Foundations, and Slab on grade.

\*\* Design strength shown is for weathering purposes only; 2,500 psi strength was used for purposes of structural design. Mixes shall be proportioned to accommodate placement. Slump, W/C ratio, admixtures and aggregate size will be determined by the contractor in accordance with ACI. Mixes will be approved by one of the following criteria.

Mix design is submitted in accordance with ACI 318 Section 5.3.
Mix design is submitted in accordance with ACI 318 Section 5.4.

Admixtures: all concrete, including slab on ground, shall contain an acceptable water-reducing admixture conforming to ASTM C494 and be used in strict accordance with the manufacturer's recommendations.

All concrete which is exposed to freezing/thawing in a moist condition or exposed to deicing chemicals shall contain an air entraining agent, conforming to ASTM C260. The amount of entrained air shall be 5% +/- 1% by volume. Air % is based on 3/4" coarse aggregate; adjust % per ACI 318 for other coarse aggregate sizes. Air-entrainment shall not be used at slabs that will receive a smooth, dense, troweled finish.

Trucks hauling plant-mixed concrete shall arrive on-site with a field ticket indicating the maximum gallons of water that can be added at the site not to exceed the total water content in the approved mix design.

Concrete shall be deposited as nearly as practicable in its final position to avoid segregation due to rehandling or flowing. Concrete shall be thoroughly consolidated by suitable means during placement and shall be thoroughly worked around reinforcement, embedded items, and into corners of forms.

**Formwork and Accessories**

Concrete construction shall conform to ACI 301 "Specifications for Structural Concrete" and the Building Code, including testing procedures. See specifications and/or architectural documents for formwork requirements. Installation shall adhere to ACI 301. Conduits and pipes of aluminum shall not be embedded in concrete construction.

See architectural drawings for exact locations and dimensions of door and window openings in all concrete walls and for all grooves, notches, chamfers, feature strips, color, texture, and other finish details at all exposed concrete surfaces. Concrete accessories and embedded items shall be coordinated with Architectural and all other Contract Documents and suppliers' drawings before placing concrete. Anchor rods, reinforcing, hardware, etc. shall be firmly tied in place prior to concrete placement; wet-setting of these items are not permitted in concrete.

Refer to Architectural documents for waterstops, dampproofing & soil retaining wall drainage requirements at concrete and concrete joints (construction joints, slab to wall joints, curb to slab joints, etc).

**Curing and Finishes**

Protect and cure freshly placed concrete per ACI 305 in hot conditions, ACI 306 in cold conditions, and ACI 308 "standard specification for curing concrete". All exposed edges and corners shall have 3/4" chamfer, UNO. Concrete flatwork shall be sloped to provide positive drainage. Coordinate finish with architectural contract documents.

At the time of application of finish materials or special treatment to concrete, moisture content of concrete shall conform to requirements in finish material specifications. Where vapor sensitive coverings are to be placed on slabs on grade, conform strictly to slab covering manufacturer's recommendations regarding vapor retarder and granular fill requirements below the slab.

**Reinforcing in Cast-in-Place Walls**

See Reinforcement General Notes for more information. Uppermost and lowermost horizontal reinforcing in walls shall be placed within 1/2 of specified spacing from the top and bottom of the wall.

**Concrete wall reinforcing** - typical UNO:

Table with 4 columns: Wall thickness, horizontal bars, vertical bars, and location. Includes values for 8" or less wall thickness.

Concrete protection; provide edge cover as follows. When a thickness of cover required for fire protection is greater than that specified in this section, such greater thickness shall be used:

- Uniformed surfaces cast against and permanently exposed to earth = 3"
• Formed surfaces exposed to earth or weather: #6 bars or larger = 2"; #5 bars or smaller = 1-1/2"
• Clear spacing between 2 or more parallel layers = 1"

**Concrete Crack Maintenance**

Cracking occurs in concrete structures due to inherent shrinkage, creep, and the restraining effects of walls and other structural elements. Most cracking due to shrinkage and creep will likely occur over the first two years of the life of the structure; further concrete movement due to variations in temperature may persist. Cracks that result in water penetration will need to be repaired to protect reinforcing. Other cracking may be repaired at the owner's discretion for aesthetic reasons or performance of applied finishes. Prior to repairing cracks, a structural engineer should be consulted to provide direction on which cracks to repair and on whether observed cracks may affect the strength of the structure.

**Reinforcement in Concrete**

**Materials**

Reinforcing steel shall conform to ASTM A615 (including supplement S1), Grade 60, Fy = 60,000 psi, except any bars specifically so noted on the drawings shall be Grade 40, Fy = 40,000 psi.

Welded Wire Reinforcing (WWR) shall conform to ASTM A185. Lap splice adjacent mats of welded wire shall be a minimum of 8" at sides and ends. In equipment pads, use minimum WWR 6x6-W2.1xW2.1, UNO.

**Procedures**

Reinforcing steel shall be detailed (including hooks and bends) in accordance with ACI 315 "Details and Detailing of Concrete Reinforcement". Lap all reinforcement in accordance with "The Reinforcing Splice and Development Length Schedule" on these documents. If table is not provided, lap all reinforcing by 40 bar diameters. Provide corner bars at all wall and footing intersections.

Reinforcing steel shall be adequately supported to prevent displacement during concrete and grout placement. Bars shall be bent cold. Bars partially embedded in concrete shall not be field bent, unless specifically so detailed or approved by the SER.

Welding or tack welding of reinforcing bars to other bars or to plates, angles, etc, is prohibited, except where specifically approved by the SER.

**Anchorage**

Post installed anchors shall not be installed without prior approval of engineer of record unless otherwise noted on the plans.

**Epoxy-Grouted Items**

Epoxy-Grouted Items (threaded rods or reinforcing bar) specified on the drawings shall be installed using "SET-XP" high strength epoxy as manufactured by the Simpson Strong Tie Company. Install in strict accordance with I.C.C. Report No. ESR 2508. Special inspection of installation is required. Rods shall be ASTM A-307 unless otherwise noted.

**Expansion Bolts**

Expansion bolts into concrete and concrete masonry units shall be "Strong Bolt" as manufactured by the Simpson Strong Tie Company, installed in strict accordance with I.C.C. Report No. ESR-1771, including minimum embedment requirements. Bolts into concrete masonry or brick masonry units shall be into fully grouted cells. Substitutes proposed by contractor shall be submitted for review with ICC reports indicating equivalent or greater load capacities. Special inspection is required for all expansion bolt installation.

**Structural Steel**

**Reference Standards**

Steel construction shall conform to the latest editions of the AISC Specifications and Codes. "Specification for Structural Steel Buildings" ANS/AISC 360 (latest edition), "Specification for Structural Joints Using ASTM A-325 or A-490 Bolts" AISC 348 (latest edition) and "Code of Standard Practice for Steel Buildings and Bridges" AISC 303 (latest edition) amended by the deletion of paragraph 4.4.1.

**Fabricators**

Fabricators for structural steel must have a quality assurance program in place. The quality assurance program must meet the requirements of one of the following methods:

- A. Participation in the AISC quality certification program.
B. Meeting the requirements of AISC seismic provisions for structural steel buildings, appendix Q and submitting plan documentation to the authority having jurisdiction, the engineer of record, and the owner or owner's designee.

**Structural Steel Members**

Structural Steel shall conform to the following requirements (unless otherwise shown on plans):

Table with 3 columns: Type of Member, ASTM Specification, and Fy. Includes Rolled Wide Flange Shapes, Plates, Channels, Angles, Square & Rectangular HSS Section, Anchor Rods, Threaded Rods, Washers, Hex Nuts, and Common Bolts.

**Steel Framing**

The contractor shall be responsible for all erection aids and joint preparations that include, but are not limited to: erection angles, lift holes, and other aids; welding procedures; required root openings; root face dimensions; groove angles; backing bars; copes; surface roughness values; tapers of unequal parts.

**Bolts**

All ASTM A-307 bolts shall be provided with lock washers under nuts or self-locking nuts.

**Welding**

All welding shall be in conformance with AISC and AWS standard and shall be performed by AWS/WABO certified welders in accordance with AWS D1.1. Only Prequalified welders, defined by AWS, shall be used.

Shop drawings shall show all welding with AWS D1.4 symbols. Welds shown on the drawings are the minimum sizes. Increase weld size to AWS minimum sizes, based on plate thickness. Minimum welding shall be 3/16" UNO. All welds shall be made using low-hydrogen electrodes with minimum tensile strength of 70 ksi and a Charpy V-Notch (CVN) toughness of at least 20 foot-pounds at -20 degrees Fahrenheit.

Welding procedures shall be submitted to the owner's testing agency for review prior to commencement of fabrication or erection. Field welds shown are engineer's recommendation. Contractor is responsible for actual welds used to support specific means and methods.

- S1.0 General Structural Notes
S1.1 General Structural Notes and Schedules
S2.0 Foundation Plan
S2.1 Main Floor Framing Plan
S2.2 Upper Floor Framing Plan
S2.3 Roof Framing Plan
S3.0 Structural Details
S3.1 Structural Details
S3.2 Structural Details
S3.3 Structural Details
S3.4 Structural Details

**SHEET INDEX**



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Table with 3 columns: Revision, Issue Date, Drawing Set

1/8/2024 Permit Set

**General Structural Notes**

**S1.0**



### Shop Painting

All steel to be shop primed. All other steel shall be given one coat of shop paint, in accordance with Section 1.24 of the AISC "Specification" and Section 6.5 of the AISC "Code", unless noted otherwise. Structural joints and faying surfaces which are to be connected by means of welds or bolts shall not be painted until all welds and bolts are installed, inspected and approved.

### Finishing

The terms finish, finish column, finishing, milled, milled surface or milling are intended to include surfaces which have been accurately sawed or finished to a true plane as defined by AISC. Grind surface value equal to or less than 1,000 as defined by ANSI B46.2 (4-inch and thinner).

### Wood

#### Material Criteria

Framing lumber shall be kiln dried or mc-19 (unless more stringent criteria are required in these notes or on the drawings) and graded and marked in conformance with the latest WCLIB standard grading rules for west coast lumber no. 17. Furnish to the following minimum standards:

4x beams & posts	DF #2
6x beams & posts	DF #1
4x treated beams & posts, 6x treated posts	HF kdat #2
2x joists, rafters, built-up beams, headers	HF #2
2x flatwise & edgewise blocking	HF standard
2x studs	HF kd stud
2x plates	HF kd15 standard
2x treated plates/ledgers	HF kdat #2

#### Moisture Content and Care of Material During Construction

All 2x studs and plates shall be kiln dried. The Contractor shall take measures to minimize exposure of sawn lumber and engineered wood products to moisture during construction. Excessive changes in moisture content during construction may result in swelling and shrinkage of a single story level in the magnitude of 1/2".

#### Wood Structural Panels

Wood structural panels shall be APA rated sheathing. Plywood shall be grade C-D or Structural II, exterior glue, exposure 1 durability classification, in conformance with USDOC PS 1 or PS 2, ASTM D 5457 and IBC 2304.7 and table 2304.7(2). Oriented strand board (OSB), shall be in accordance with USDOC PS 2, and of equivalent thickness, exposure rating and span rating and may be used in lieu of plywood pending OSB substitution approval by Architect. See plans for thickness, panel identification index and nailing requirements. Unless otherwise noted on plans:

Roof sheathing shall be 15/32" with span rating 32/16
Floor sheathing shall be 23/32" with span rating 48/24
Wall sheathing shall be 15/32" with span rating 24/0

#### Plywood Web Joists

Prefabricated plywood web joist design shown on plans is based on ILevel/Trus-Joist products manufactured by the Weyerhaeuser Corporation. Alternate plywood web joists must have equivalent section properties & allowable stresses to those specified to be considered and are subject to review and approval by the Architect and SER. Alternate joist hangers and other hardware may be substituted for items shown provided they have ICC approval for equal or greater load capacities. All joist hangers, hardware shall be compatible in size with plywood web joist provided. All necessary bridging, blocking, blocking panels, stiffeners, etc., shall be detailed and furnished by the manufacturer. The following deflection criteria shall be maintained:

Floor live load deflections shall be limited to span/480 (span/360 at 100 psf live load).  
Roof total load deflections shall be limited to span/240.

Specified plywood web joists at floors have been designed for a minimum T3-Pro rating of 40 in addition to the maximum allowable deflections noted above.

#### Structural Composite Lumber

Manufactured lumber, PSL, LVL, and LSL, shall be manufactured under a process approved by the national research board. Each piece shall bear a stamp or stamps noting the name and plant number of the manufacturer, the grade, the national research board number, and the quality control agency. All PSL, LVL and LSL lumber shall be manufactured in accordance ICC Report ESR-1387. LVL lumber shall be manufactured using veneer glued with a waterproof glue with the requirements of ASTM D2559 with all grain parallel with the length of the member. The members shall have the following minimum properties:

PSL (2.2E) Beams	Fb = 2,900 psi, E = 2,200 ksi, Fv = 290 psi
LVL (2.0E) Beams	Fb = 2,600 psi, E = 2,000 ksi, Fv = 285 psi
LVL (1.55E) Beams	Fb = 2,325 psi, E = 1,550 ksi, Fv = 310 psi

Design shown on plans is based on ILevel/Trus-Joist products manufactured by the Weyerhaeuser Corporation. Alternate manufacturers may be used subject to review and approval by the Architect and Structural Engineer of Record, alternate joist hangers and other hardware may be substituted for items shown provided they have ICC approval for equal or greater load capacities. All joist hangers and other hardware shall be compatible in size with members provided.

#### Glue Laminated Material

Glued laminated members shall be fabricated in conformance with AITC 117 and APA-EWS Y117, Stress Class 24F-1.8E. Each member shall bear an AITC identification mark and shall be accompanied by an AITC certificate of conformance. All simple span beams shall be dougлас fir combination 24F-V4, Fb = 2,400 psi, Fv = 265 psi and all cantilevered beams and columns shall be Douglas fir combination 24F-V8, Fb = 2,400 psi, Fv = 265 psi unless otherwise noted. Camber all simple span glulam beams to 3,500" radius or zero camber, unless shown otherwise on the plans.

#### Treated Wood

All wood framing in direct contact with concrete or masonry, exposed to weather, or that rest on exterior foundation walls and are located within 8" of earth, shall be pressure-treated with an approved preservative per IBC section 2303.1.8. Cut or drilled sections of treated material shall be treated with an approved preservative per IBC section 2303.1.8. See IBC section 2304.11 for additional requirements.

#### Metal Products in Contact with Treated Lumber

Simpson hardware in contact with ACQ, CA, or CBA pressure-preservative treated wood shall have a Zmax finish (G185 HDG per ASTM A653) or shall be post hot-dip galvanized (per ASTM A123 for connectors and ASTM A153 for fasteners) unless otherwise noted. Exception: type 304 or 316 stainless steel connectors and fasteners are required for the following applications:

- ACQ, CA, or CBA treatments with ammonia where members are used in exterior applications.
- all ACZA treatments
- retention levels greater than 0.40 pcf for ACQ, 0.41 pcf for CBA-A, or 0.21 pcf for CA-B treatments.

Stainless steel connectors require matching stainless steel fasteners. Zmax and post hot-dip galvanized connectors require fasteners galvanized per ASTM A153. Thru-bolts and anchor rods used in dry conditions shall be permitted to be mechanically deposited zinc coated steel with coating weights in accordance with ASTM B 695, class 55 minimum. See IBC section 2304.9.5 and "Framing connectors" notes on this sheet for additional requirements.

#### Framing Connectors

Timber connectors called out by letters and numbers shall be "strong-tie" by Simpson company, as specified in their catalog number C-C-2021. Equivalent devices which by other manufacturers may be substituted, provided they have ICB0 approval for equal or greater load capacities. Provide number and size of fasteners as specified by manufacturer. Connectors shall be installed in accordance with the manufacturer's recommendations. Where connector straps connect two members, place one-half of the nails or bolts in each member. All bolts in wood members shall conform to ASTM A307. Provide washers under the heads and nuts of all bolts and lag screws bearing on wood. Unless otherwise noted, all nails shall be as called out below. Unless otherwise noted on the drawings use the following hangers:

2x or 2-2x member to flush wood beam/ledger  
2x member to sill plate or steel/flush wood beam  
2-2x member to sill plate or steel/flush wood beam  
TJI member to flush wood beam/ledger  
2-TJI member to flush wood beam/ledger  
TJI member to sill plate or steel/flush wood beam  
2-TJI member to sill plate or steel/flush wood beam  
4x, LSL/LVL/PSL beam to flush wood beam/ledger  
4x, LSL/LVL/PSL beam to sill plate or steel beam  
Interior 4x or 6x post to concrete below  
Treated 4x or 6x post to concrete below  
4x or 6x post to wood beam above  
wood beam to wood beam that bears on post

LUS  
LB  
B  
ITS  
MIU  
MIU  
ITS  
MIT  
MIU max  
HWU  
ABU w/ 5/8" dia. anchor rod w/ 7" embed  
CBSQ-SDS2HDG  
PC/EPC  
HU/CTF

#### Fasteners

Shall conform to the following requirements, UNO. Splitting shall be avoided at all wood fasteners:

Steel to wood or wood to wood connection bolts  
Anchor rods (w/ threaded ends & welded nut)  
Lag screws  
Wood screws  
Nails

Nail sizes are specified as follows. If the contractor proposes the use of alternate nails, they shall submit nail specifications to the Structural Engineer of Record (prior to construction) for review and acceptance.

Simpson hardware	typical UNO	see catalog
MSTC holdown straps direct to studs		0.148 x 1-1/2"
MSTC holdown straps over shear wall sheathing to studs		0.148 x 2-1/4"
CS collector straps		0.131 x 2-1/2"
hangers w/ 16d or 10d options		0.162 x 3-1/2"
floor sheathing	typical	0.131 deformed shank x 2-1/2"
roof sheathing	typical	0.131 x 2-1/2"
stud wall APA sheathing	15/32 sheathing	0.148 x 2-1/4"
member to member face nailing	typical UNO	0.131 x 3"
bottom plate to framing below	typical UNO	0.131 x 3-1/4"
toe nailing	typical UNO	0.131 x 3"

Sheathing fasteners shall be driven so that head or crown is flush with sheathing surface. 3/8" min. edge distance shall be maintained on sheathing fasteners.

Spaced fasteners specified on the drawings shall begin at 1/2 specified spacing from the ends of the members, unless otherwise noted. Provide (2) fasteners minimum each member, typ. Anchor rods from sill plates to concrete shall begin a min. of 6" and a max. of 12" from each end of each piece of sill plate.

Thru-bolt and anchor rod holes shall be at least 1/32" but no more than 1/16" larger than bolt/rod diameter. Clearance holes for lag screw shanks shall have the same diameter as the lag shank and the same penetration depth as the length of the unthreaded shank. Lead holes for threaded portion of lag screws shall have a diameter of 55 to 60% of lag screw shank diameter and shall extend the length of the threaded portion of the lag screw.

#### Stair and Stair Landing Framing Requirements

4'-0" maximum width UNO

Landings: span 2x6 joists @ 16"oc in short direction of landing. At full height wood studs, provide 2x6 continuous ledger w/ (3) 0.131 x 3-1/4" nails to each stud. At concrete walls, provide treated 2x6 continuous ledger w/ 5/8" diameter anchor rods @ 16"oc. Embed 5". Where landing edge is not supported by beam, full height stud wall, or full height concrete wall, provide 2x4 @ 16" cripple wall from landing edge to slab on grade below.

Stringers 9'-0" in length or less: provide 2x12 stringers at center and sides of stair. Notch to 5-1/2" minimum depth and provide HUS26 hangers to supporting beams. At center stringer, sister 2x6 ea. side of stringer and at side stringers, sister 2x6 one side of stringer. End sistered 2x6's short of hangers. Stringers 11'-6" to 14'-0" in length: provide 1-3/4" x 14" LVL 1.9E stringers at center and sides of stair. Notch to 8" minimum depth and provide H47 hangers to supporting beams. At center stringer, sister 2x8 ea. side of stringer and at side stringers, sister 2x8 one side of stringer. End sistered 2x8's short of hangers.

Where stringers bear on top of wood floor framing below, provide (2) LS70 clip at bottom of stringer. Where stringers bear on concrete slab, provide 2x treated sill plate w/ 5/8" exp. bolt at each stringer (embed 3-1/8").

#### General Wood Framing Criteria (UNO in previous sections)

All wood framing details not shown otherwise shall be constructed to the minimum standards of section 2308 of the IBC. Minimum nailing, unless otherwise noted, shall conform to table 2304.9.1 of the IBC. Unless otherwise noted, all nails shall be common. Coordinate the size and location of all openings with Mechanical and Architectural drawings. Provide washers under the heads and nuts of all bolts, anchor rods, and lag screws bearing on wood, unless otherwise noted. Installation of lag screws shall conform to NDS section 11.1.3. Bolts, anchor rods, and lag screws shall be centered in members, uno.

All structural stud walls (bearing or shear walls) shown and not otherwise noted shall be 2x4 studs @ 16"oc at interior walls and 2x6 @ 16"oc at exterior walls. See Architectural drawings for differing wall widths and for framing at nonstructural walls. Two studs minimum shall be provided at the end of all walls and at each side of all openings, and below beam bearing points. Solid blocking for 4x/6x wood posts and multi-stud posts shall be provided through intermediate levels to supports below. Provide continuous solid blocking at mid-height of all stud walls over 10'-0" in height and at mid-height of walls with sheathing on one side only (i.e. Each side of party walls).

All stud walls shall have their lower wood plates attached to wood framing below with 0.131 x 3-1/4" nails @ 8"oc or bolted to concrete with 5/8" diameter anchor rods @ 6'-0"oc for structures not exceeding 2 stories and @ 4'-0" for all other structures unless otherwise noted. Embed anchor rods 7" unless otherwise noted. Individual members of built-up posts shall be nailed to each other with 0.131 x 3" nails @ 8"oc staggered.

Refer to the plans and shear wall schedule for required sheathing and nailing. When not otherwise noted, provide gypsum wallboard on interior surfaces nailed to all studs, top and bottom plates and blocking with nails at 7"oc. Use #6 x 1-5/8" screws for 1/2" GWB and #6 x 1-7/8" screws for 5/8" GWB. Provide 15/32" APA rated sheathing on exterior surfaces nailed at all panel edges (block unsupported edges), top and bottom plates with 0.148 x 2-1/4" nails @ 6"oc and to all intermediate studs and blocking @ 12"oc. Allow 1/8" gap at all APA sheathing panel edges and ends. (see details where larger gap is required).

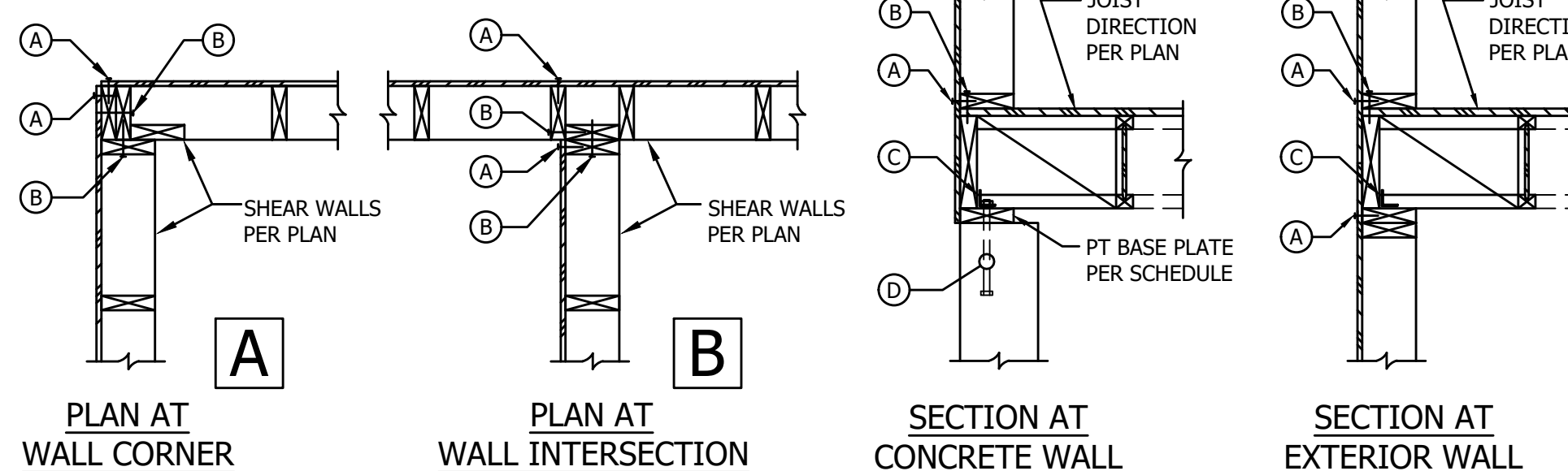
At exterior walls, provide flat wise 2x6 at all door heads and window sills and heads, unless otherwise noted. (provide flat wise 2-2x6 where opening width is greater than 6'-0" and less than 9'-6", unless otherwise noted). Provide (3) 0.131 x 3" toenails each end of each 2x6 member.

Provide double joists under all parallel partitions that extend over more than half the joist length and around all openings in floors or roofs unless otherwise noted. Provide solid blocking at all bearing points.

Toenail joists to supports with (3) 0.131 x 3" nails. Attach timber joists to flush headers or beams with Simpson metal joist hangers in accordance with notes above. Individual members of multi-joist beams shall be nailed to each other with (2) rows of 0.131 x 3" nails @ 12"oc.

Unless otherwise noted on the plans, APA sub-flooring and roof sheathing shall be laid up with grain (strength axis) perpendicular to supports (joists, trusses, etc.) and in a staggered pattern. Nails shall be @ 6"oc to framed panel edges, @ 4"oc over shear walls and @ 12"oc to intermediate supports. See notes above for nail sizes. All sub-flooring edges shall have approved tongue-and-groove joints or shall be supported with solid blocking/framing. Plywood clips are recommended at all roof sheathing edges (solid blocking/framing is not required at panel edges unless specifically noted in the structural drawings or required by the roofing manufacturer). Glue sub-flooring to all supports with adhesive conforming to APA spec. AFG-01 in accordance with the manufacturer's recommendations. Allow 1/8" gap at all panel edges and ends of floor and roof sheathing. Where blocked floor and roof diaphragms are indicated, provide flat 2x blocking at all unframed panel edges and nail with edge nailing specified.

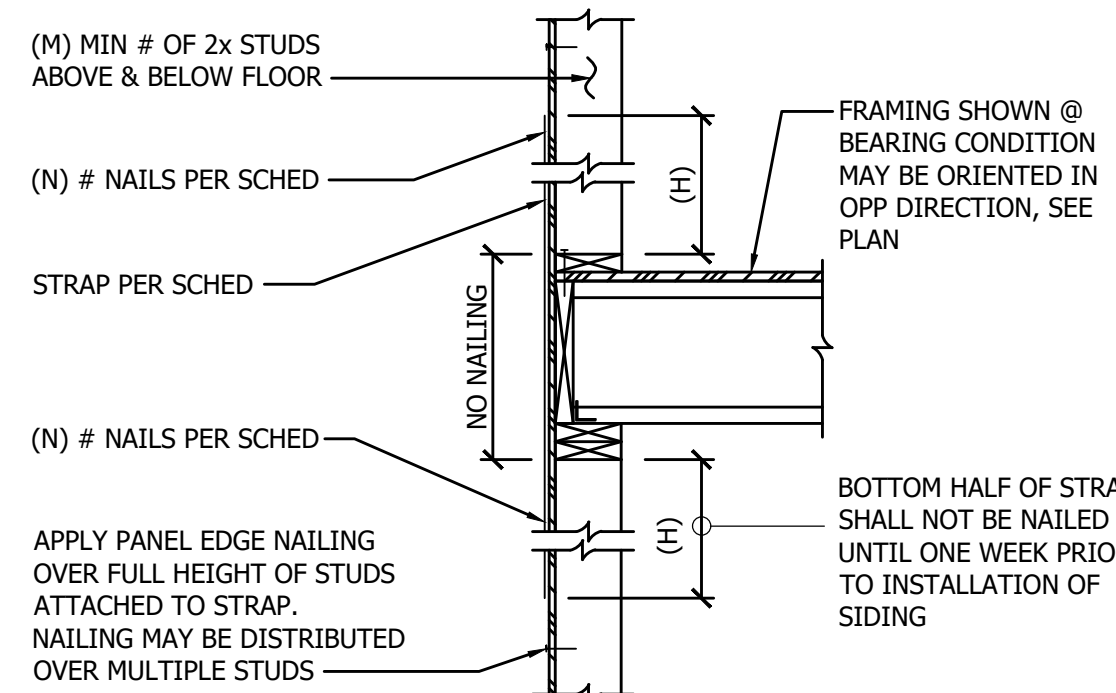
SHEAR WALL SCHEDULE								
MARK	SHEATHING	PANEL EDGE NAILING (A)	TOP PLATE NAILING (B)	A35 CLIPS (C)	MUDSILL TO CONCRETE (D)		CAPACITY (PLF)	
					2x6 P.T.	3x6 P.T.	SEISMIC	WIND
SW6	1/2" PLYWOOD	0.131" @ 6"oc	0.131" @ 6"oc	A35 @ 24"oc	5/8" AB @ 48"oc	5/8" AB @ 64"oc	223	270
SW4	1/2" PLYWOOD	0.131" @ 4"oc	0.131" @ 4"oc	A35 @ 16"oc	5/8" AB @ 32"oc	5/8" AB @ 48"oc	325	405
SW3 <sup>5</sup>	1/2" PLYWOOD	0.131" @ 3"oc	0.131" @ 3"oc	A35 @ 12"oc	5/8" AB @ 16"oc	5/8" AB @ 32"oc	418	540
SW2 <sup>5</sup>	1/2" PLYWOOD, DOUG-FIR	0.131" @ 2"oc	(2) ROWS 0.131" @ 3"oc	A35 @ 7"oc	5/8" AB @ 16"oc	5/8" AB @ 16"oc	600	895
SW2* <sup>5</sup>	1/2" OSB/STRUCT-1, DOUG-FIR	0.148" @ 2"oc	(2) ROWS 0.131" @ 3"oc	A35 @ 6"oc	5/8" AB @ 12"oc	5/8" AB @ 16"oc	600	1080



### 1 Shear Wall Schedule

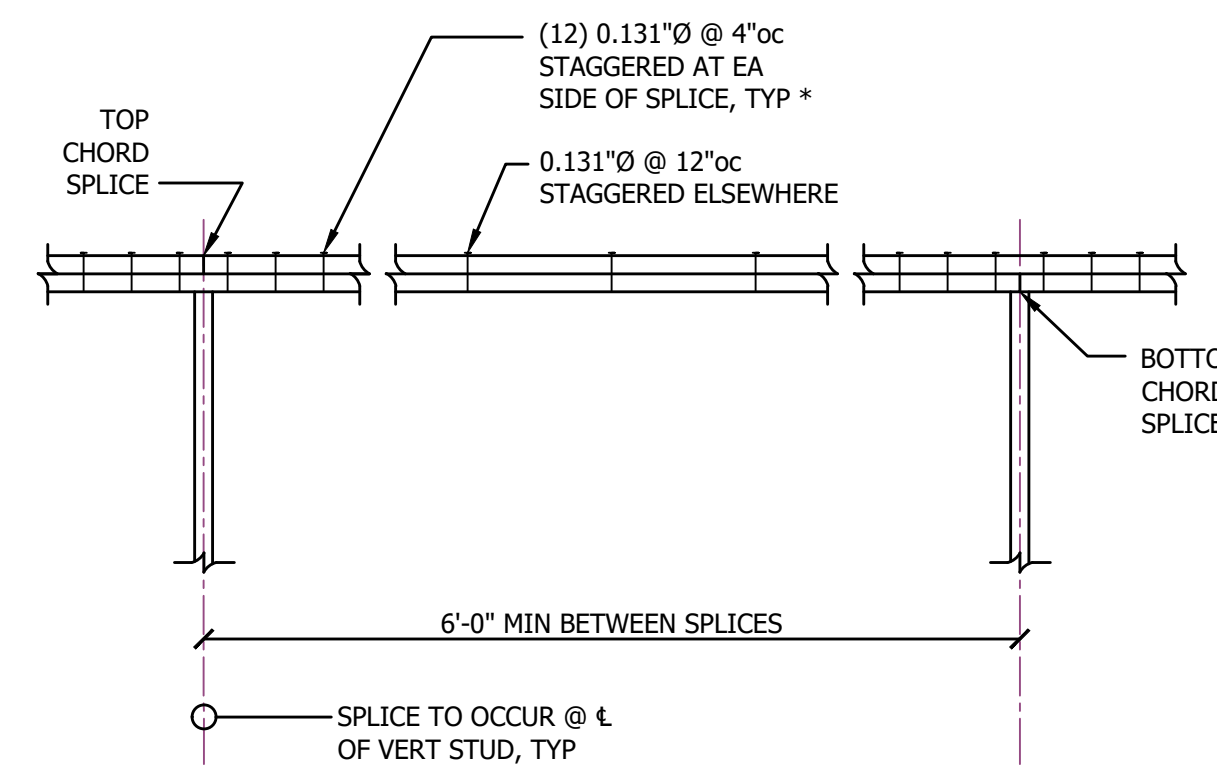
3/4" = 1'-0"

STRAP SCHEDULE				
MARK	H	N	M	HF CAPACITY
CS16	14"	(13) 0.131"	1	1,705#
MSTC40	12"	(14) 0.148"	2	2,325#
MSTC52	17"	(22) 0.148"	2	3,650#
MSTC66	24"	(32) 0.148"	2	5,505#



### 2 Strap Schedule

3/4" = 1'-0"

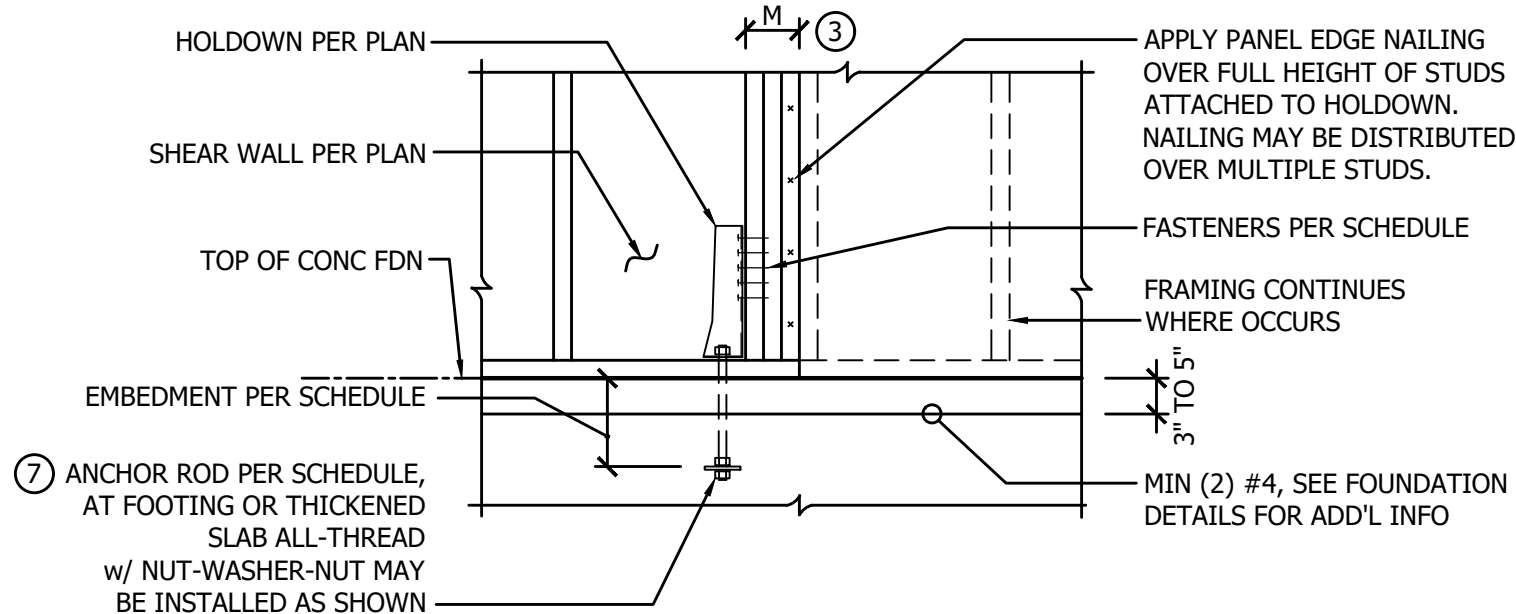


### 3 Top Plate Splice, Typ.

3/4" = 1'-0"

HOLDOWN SCHEDULE (1) (2)										
MARK	FASTENERS	M (3)	FOOTING / STRUCTURAL SLAB				TOP OF STEM WALL (4)			
			ANCHOR ROD	EMBEDMENT	EDGE DISTANCE	CAPACITY	ANCHOR ROD	EMBEDMENT	CAPACITY (SEISMIC / WIND)	
									CONTINUOUS (5)	CORNER (5)
HDU2	(6) SDS<sup>1/4</sup>x2<sup>1/2</sup>	3"	5/8"<sup>Ø</sup>	6"	8"	2,645#	S8<sup>1/2</sup>x24	18"		2,645#
HDU8	(20) SDS<sup>1/4</sup>x2<sup>1/2</sup>	4x6 DF	5/8"<sup>Ø</sup>	9"	14"	7,870#	S8<sup>1/2</sup>x24	18"	7,870#	7,855# / 7,870# 5,730# / 6,820#

- PLACEMENT OF ANCHOR ROD IS BASED ON CAST-IN-PLACE INSTALLATION.
- INSTALL ALL HOLDOWNS PER MANUFACTURER'S INSTRUCTIONS.
- DEPTH OF WOOD FRAMING MEMBER ATTACHED TO HOLDOWN. MEMBERS SHALL BE HEM-FIR UNLESS NOTED OTHERWISE NOTED.
- MIN 6" CONCRETE WALL THICKNESS REQ'D, MIN EDGE DISTANCE OF 1 1/2".
- BASED ON MIN 27" DISTANCE FROM END/CORNER OF WALL.
- BASED ON MIN 4 1/4" DISTANCE FROM END OF WALL.
- AT RETROFIT CONDITIONS USE 5/8" THREADED ROD w/ EPOXY PER GENERAL STRUCTURAL NOTES, MIN. 12" EMBED. 1"Ø EPOXY RODS REQUIRE 20" EMBEDMENT.



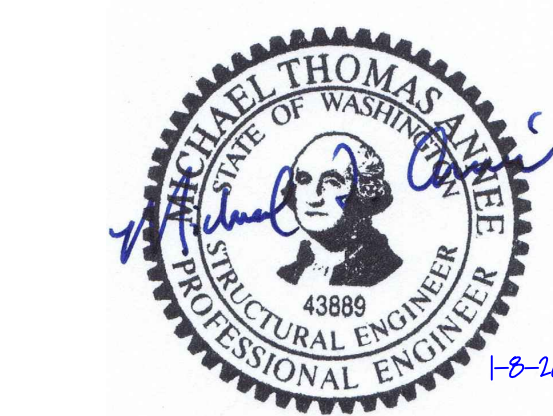
### 4 Holddown Schedule

3/4" = 1'-0"

- NOTES:
- ALL EXTERIOR WALLS SHALL BE SW6 (TYP, UNO). WALL FRAMING SHALL BE 2x HF (UNO) STUDS @ 16"oc BLOCK ALL PANEL EDGES WITH 2x LAID FLAT. ALL STUDS ATTACHED TO STRAPS OR HOLDOWNS SHALL BE PANEL-EDGE NAILED. NAIL TO ALL INTERMEDIATE SUPPORTS WITH 0.131"Ø @ 12"oc SHEATHING SHALL BE MIN. 1/8" OSB OR PLYWOOD.
  - PANEL EDGE NAILS SHALL BE A MINIMUM OF 2 1/2" IN LENGTH. PLATE NAILS SHALL BE A MINIMUM OF 3" IN LENGTH.
  - LTP4 OR LS50 CLIPS MAY BE SUBSTITUTED FOR A35 CLIPS.
  - EMBED ANCHOR BOLTS 7" MIN. ALL BOLTS SHALL HAVE 3x3x1/4" PLATE WASHERS (EDGE OF WASHER SHALL BE WITHIN 1/2" OF SHEATHING). EACH MUDSILL SHALL HAVE A MINIMUM OF (2) ANCHOR BOLTS WITH (1) BOLT LOCATED NOT MORE THAN 12" OR LESS THAN 4 1/2" TO EACH END. SIMPSON TITEN HD SCREWS, SIMPSON STRONG-BOLT OR HILTI KWIK-BOLT TZ EXPANSION BOLTS MAY BE SUBSTITUTED FOR ANCHOR BOLTS w/ 5" MIN EMBED.
  - FOR SW2, SW3: AT (2) ROWS NAILING/CLIPS: USE DOUBLE RIM, JOIST OR BLOCKING. FRAMING AT BUTTING PANEL EDGES SHALL BE 3x MINIMUM OR (2) 2x STITCHED TOGETHER w/ PLATE NAILING PER APA FORM #TT-076. ALL PANEL EDGE NAILING TO BE STAGGERED. 3x SILL PLATES ARE REQUIRED AT ANCHOR BOLT CONNECTIONS.



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General Structural  
Notes & Schedules

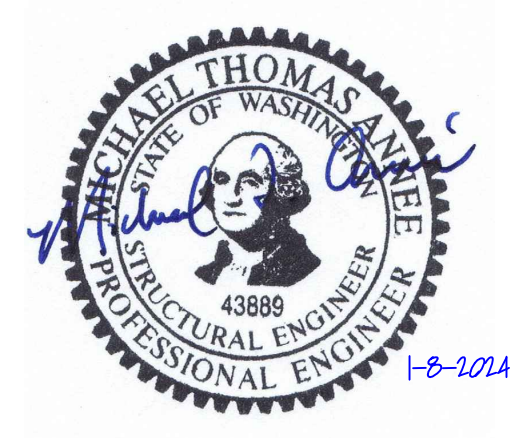
S1.1

GENERAL STRUCTURAL NOTES  
(TYPICAL UNLESS NOTED OTHERWISE ON DRAWINGS)



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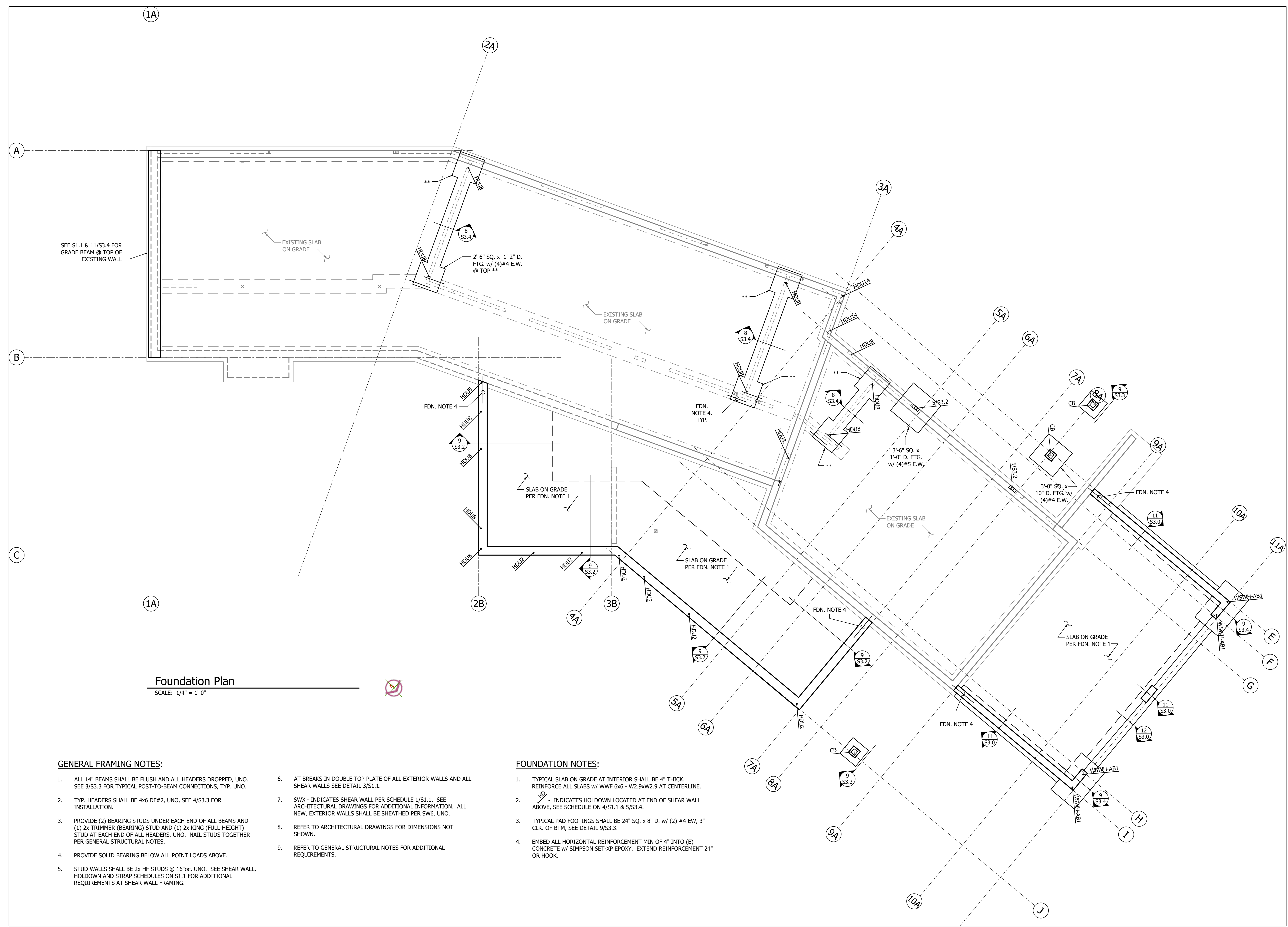
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Revision Issue Date Drawing Set

1/8/2024 Permit Set

Foundation Plan

S2.0



**Foundation Plan**  
SCALE: 1/4" = 1'-0"

**GENERAL FRAMING NOTES:**

- ALL 14" BEAMS SHALL BE FLUSH AND ALL HEADERS DROPPED, UNO. SEE 3/S3.3 FOR TYPICAL POST-TO-BEAM CONNECTIONS, TYP. UNO.
- TYP. HEADERS SHALL BE 4x6 DF#2, UNO, SEE 4/S3.3 FOR INSTALLATION.
- PROVIDE (2) BEARING STUDS UNDER EACH END OF ALL BEAMS AND (1) 2x TRIMMER (BEARING) STUD AND (1) 2x KING (FULL-HEIGHT) STUD AT EACH END OF ALL HEADERS, UNO. NAIL STUDS TOGETHER PER GENERAL STRUCTURAL NOTES.
- PROVIDE SOLID BEARING BELOW ALL POINT LOADS ABOVE.
- STUD WALLS SHALL BE 2x HF STUDS @ 16"oc, UNO. SEE SHEAR WALL, HOLDOWN AND STRAP SCHEDULES ON S1.1 FOR ADDITIONAL REQUIREMENTS AT SHEAR WALL FRAMING.

**FOUNDATION NOTES:**

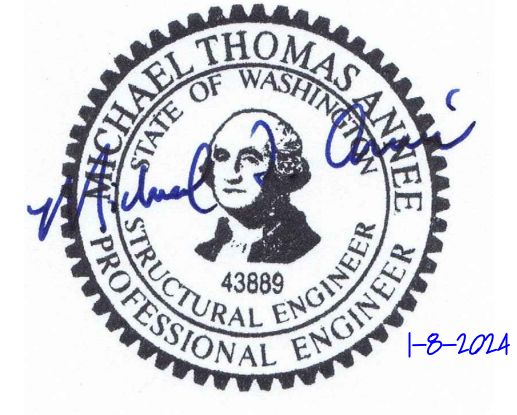
- TYPICAL SLAB ON GRADE AT INTERIOR SHALL BE 4" THICK. REINFORCE ALL SLABS w/ WWF 6x6 - W2.9xW2.9 AT CENTERLINE.
- HD - INDICATES HOLDOWN LOCATED AT END OF SHEAR WALL ABOVE, SEE SCHEDULE ON 4/S1.1 & 5/S3.4.
- TYPICAL PAD FOOTINGS SHALL BE 24" SQ. x 8" D. w/ (2) #4 EW, 3" CLR. OF BTM, SEE DETAIL 9/S3.3.
- EMBED ALL HORIZONTAL REINFORCEMENT MIN OF 4" INTO (E) CONCRETE w/ SIMPSON SET-XP EPOXY. EXTEND REINFORCEMENT 24" OR HOOK.

- AT BREAKS IN DOUBLE TOP PLATE OF ALL EXTERIOR WALLS AND ALL SHEAR WALLS SEE DETAIL 3/S1.1.
- SWX - INDICATES SHEAR WALL PER SCHEDULE 1/S1.1. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION. ALL NEW, EXTERIOR WALLS SHALL BE SHEATHED PER SW6, UNO.
- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



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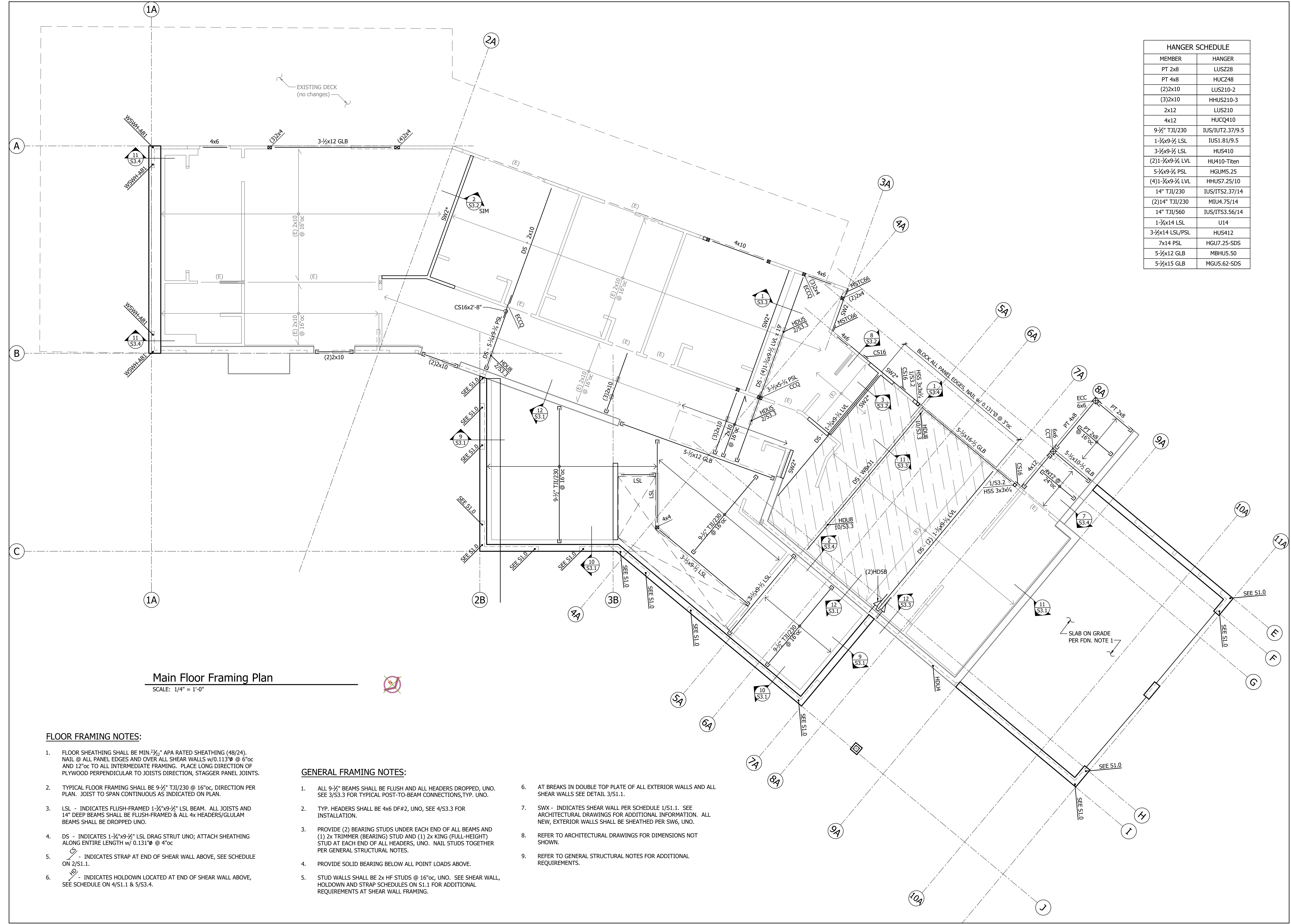
Revision Issue Date Drawing Set

1/8/2024 Permit Set

Main Floor Framing Plan

S2.1

HANGER SCHEDULE	
MEMBER	HANGER
PT 2x8	LUS228
PT 4x8	HUC248
(2)2x10	LUS210-2
(3)2x10	HHUS210-3
2x12	LUS210
4x12	HUCQ410
9-1/2" TJI/230	IUS/IUT2.37/9.5
1-3/4"x9-1/2" LSL	IUS1.81/9.5
3-1/2"x9-1/2" LSL	HUS410
(2)1-3/4"x9-1/2" LVL	HU410-Titen
5-1/4"x9-1/2" PSL	HGUM5.25
(4)1-3/4"x9-1/2" LVL	HHUS7.25/10
14" TJI/230	IUS/ITS2.37/14
(2)14" TJI/230	MIU4.75/14
14" TJI/560	IUS/ITS3.56/14
1-3/4"x14 LSL	U14
3-1/2"x14 LSL/PSL	HUS412
7x14 PSL	HGU7.25-SDS
5-1/2"x12 GLB	MBHUS.50
5-1/2"x15 GLB	MGUS.62-SDS



Main Floor Framing Plan  
SCALE: 1/4" = 1'-0"

FLOOR FRAMING NOTES:

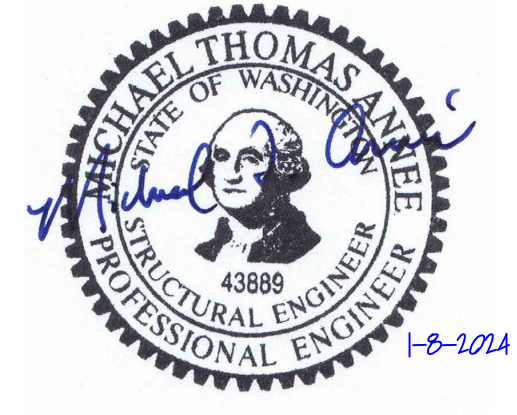
- FLOOR SHEATHING SHALL BE MIN 3/8" APA RATED SHEATHING (48/24). NAIL @ ALL PANEL EDGES AND OVER ALL SHEAR WALLS w/0.113" @ 6" oc AND 12" oc TO ALL INTERMEDIATE FRAMING. PLACE LONG DIRECTION OF PLYWOOD PERPENDICULAR TO JOISTS DIRECTION, STAGGER PANEL JOINTS.
- TYPICAL FLOOR FRAMING SHALL BE 9-1/2" TJI/230 @ 16" oc, DIRECTION PER PLAN. JOIST TO SPAN CONTINUOUS AS INDICATED ON PLAN.
- LSL - INDICATES FLUSH-FRAMED 1-3/4"x9-1/2" LSL BEAM. ALL JOISTS AND 14" DEEP BEAMS SHALL BE FLUSH-FRAMED & ALL 4x4 HEADERS/GLULAM BEAMS SHALL BE DROPPED UNO.
- DS - INDICATES 1-3/4"x9-1/2" LSL DRAG STRUT UNO; ATTACH SHEATHING ALONG ENTIRE LENGTH w/ 0.131" @ 4" oc
- SWX - INDICATES STRAP AT END OF SHEAR WALL ABOVE, SEE SCHEDULE ON 2/S1.1.
- HDUN - INDICATES HOLDDOWN LOCATED AT END OF SHEAR WALL ABOVE, SEE SCHEDULE ON 4/S1.1 & 5/S3.4.

GENERAL FRAMING NOTES:

- ALL 9-1/2" BEAMS SHALL BE FLUSH AND ALL HEADERS DROPPED, UNO. SEE 3/S3.3 FOR TYPICAL POST-TO-BEAM CONNECTIONS, TYP. UNO.
- TYP. HEADERS SHALL BE 4x6 DF#2, UNO, SEE 4/S3.3 FOR INSTALLATION.
- PROVIDE (2) BEARING STUDS UNDER EACH END OF ALL BEAMS AND (1) 2x TRIMMER (BEARING) STUD AND (1) 2x KING (FULL-HEIGHT) STUD AT EACH END OF ALL HEADERS, UNO. NAIL STUDS TOGETHER PER GENERAL STRUCTURAL NOTES.
- PROVIDE SOLID BEARING BELOW ALL POINT LOADS ABOVE.
- STUD WALLS SHALL BE 2x HF STUDS @ 16" oc, UNO. SEE SHEAR WALL, HOLDOWN AND STRAP SCHEDULES ON S1.1 FOR ADDITIONAL REQUIREMENTS AT SHEAR WALL FRAMING.
- AT BREAKS IN DOUBLE TOP PLATE OF ALL EXTERIOR WALLS AND ALL SHEAR WALLS SEE DETAIL 3/S1.1.
- SWX - INDICATES SHEAR WALL PER SCHEDULE 1/S1.1. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION. ALL NEW, EXTERIOR WALLS SHALL BE SHEATHED PER SW6, UNO.
- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



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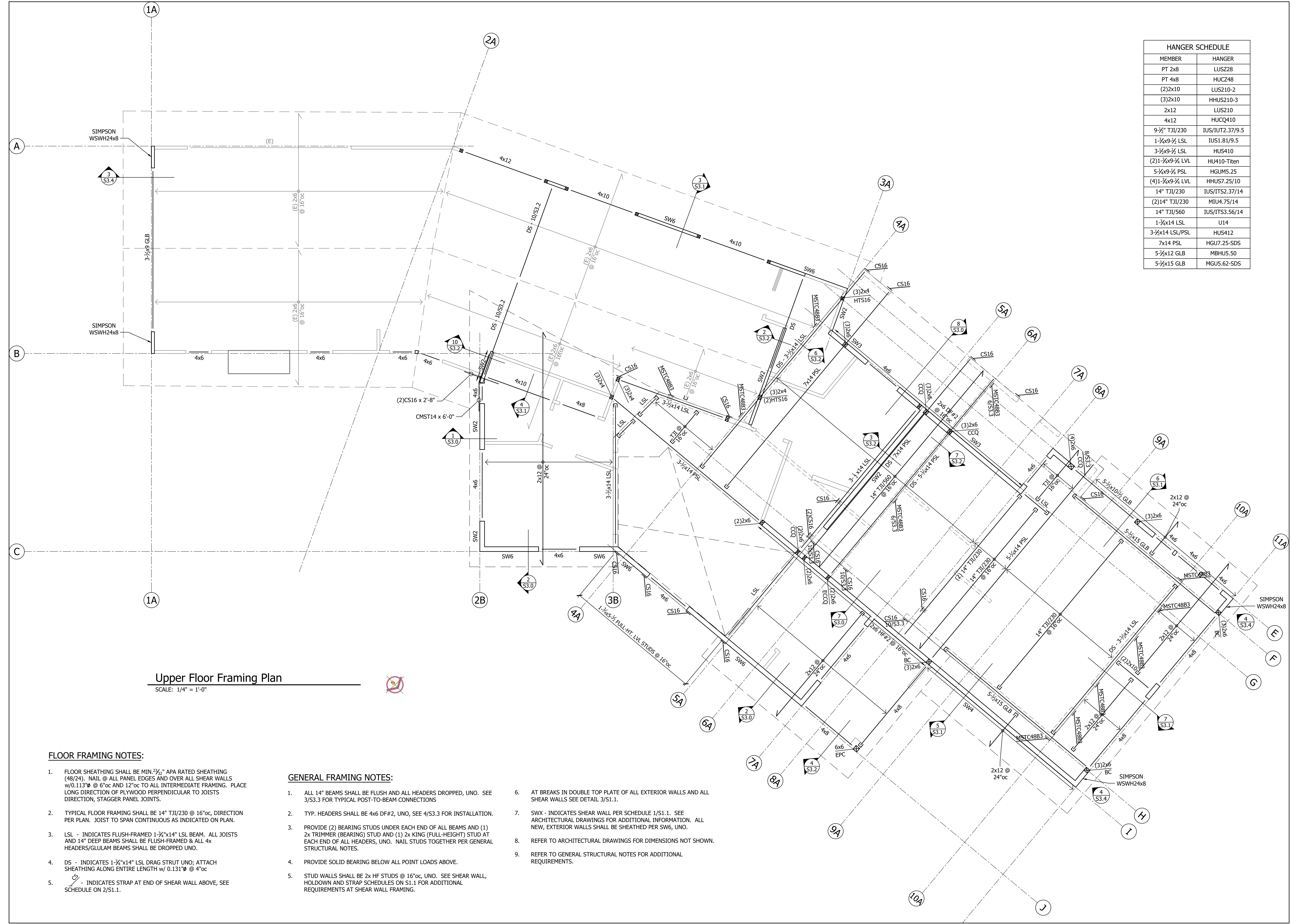
Revision Issue Date Drawing Set

1/8/2024 Permit Set

Upper Floor Framing Plan

S2.2

HANGER SCHEDULE	
MEMBER	HANGER
PT 4x8	LUS228
PT 4x8	HUCZ48
(2)2x10	LUS210-2
(3)2x10	HHUS210-3
2x12	LUS210
4x12	HUCQ410
9-1/2" TJI/230	IUS/IUT2.37/9.5
1-3/4x9-1/2 LSL	IUS1.81/9.5
3-3/4x9-1/2 LSL	HUS410
(2)1-3/4x9-1/2 LVL	HU410-Titen
5-3/4x9-1/2 PSL	HGUM5.25
(4)1-3/4x9-1/2 LVL	HHUS7.25/10
14" TJI/230	IUS/ITS2.37/14
(2)14" TJI/230	MIU4.75/14
14" TJI/560	IUS/ITS3.56/14
1-3/4x14 LSL	U14
3-3/4x14 LSL/PSL	HUS412
7x14 PSL	HGU7.25-SDS
5-3/4x12 GLB	MBHUS.50
5-3/4x15 GLB	MGUS.62-SDS



**Upper Floor Framing Plan**  
 SCALE: 1/4" = 1'-0"

**FLOOR FRAMING NOTES:**

- FLOOR SHEATHING SHALL BE MIN. 3/8" APA RATED SHEATHING (48/24). NAIL @ ALL PANEL EDGES AND OVER ALL SHEAR WALLS w/0.131" @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING. PLACE LONG DIRECTION OF PLYWOOD PERPENDICULAR TO JOISTS DIRECTION, STAGGER PANEL JOINTS.
- TYPICAL FLOOR FRAMING SHALL BE 14" TJI/230 @ 16"oc, DIRECTION PER PLAN. JOIST TO SPAN CONTINUOUS AS INDICATED ON PLAN.
- LSL - INDICATES FLUSH-FRAMED 1-3/4"x14" LSL BEAM. ALL JOISTS AND 14" DEEP BEAMS SHALL BE FLUSH-FRAMED & ALL 4x HEADERS/GLULAM BEAMS SHALL BE DROPPED UNO.
- DS - INDICATES 1-3/4"x14" LSL DRAG STRUT UNO; ATTACH SHEATHING ALONG ENTIRE LENGTH w/ 0.131" @ 4"oc
- ⊖ - INDICATES STRAP AT END OF SHEAR WALL ABOVE, SEE SCHEDULE ON 2/S1.1.

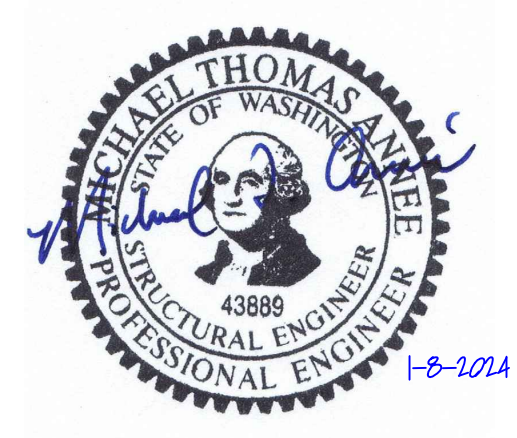
**GENERAL FRAMING NOTES:**

- ALL 14" BEAMS SHALL BE FLUSH AND ALL HEADERS DROPPED, UNO. SEE 3/S3.3 FOR TYPICAL POST-TO-BEAM CONNECTIONS
- TYP. HEADERS SHALL BE 4x6 DF#2, UNO, SEE 4/S3.3 FOR INSTALLATION.
- PROVIDE (2) BEARING STUDS UNDER EACH END OF ALL BEAMS AND (1) 2x TRIMMER (BEARING) STUD AND (1) 2x KING (FULL-HEIGHT) STUD AT EACH END OF ALL HEADERS, UNO. NAIL STUDS TOGETHER PER GENERAL STRUCTURAL NOTES.
- PROVIDE SOLID BEARING BELOW ALL POINT LOADS ABOVE.
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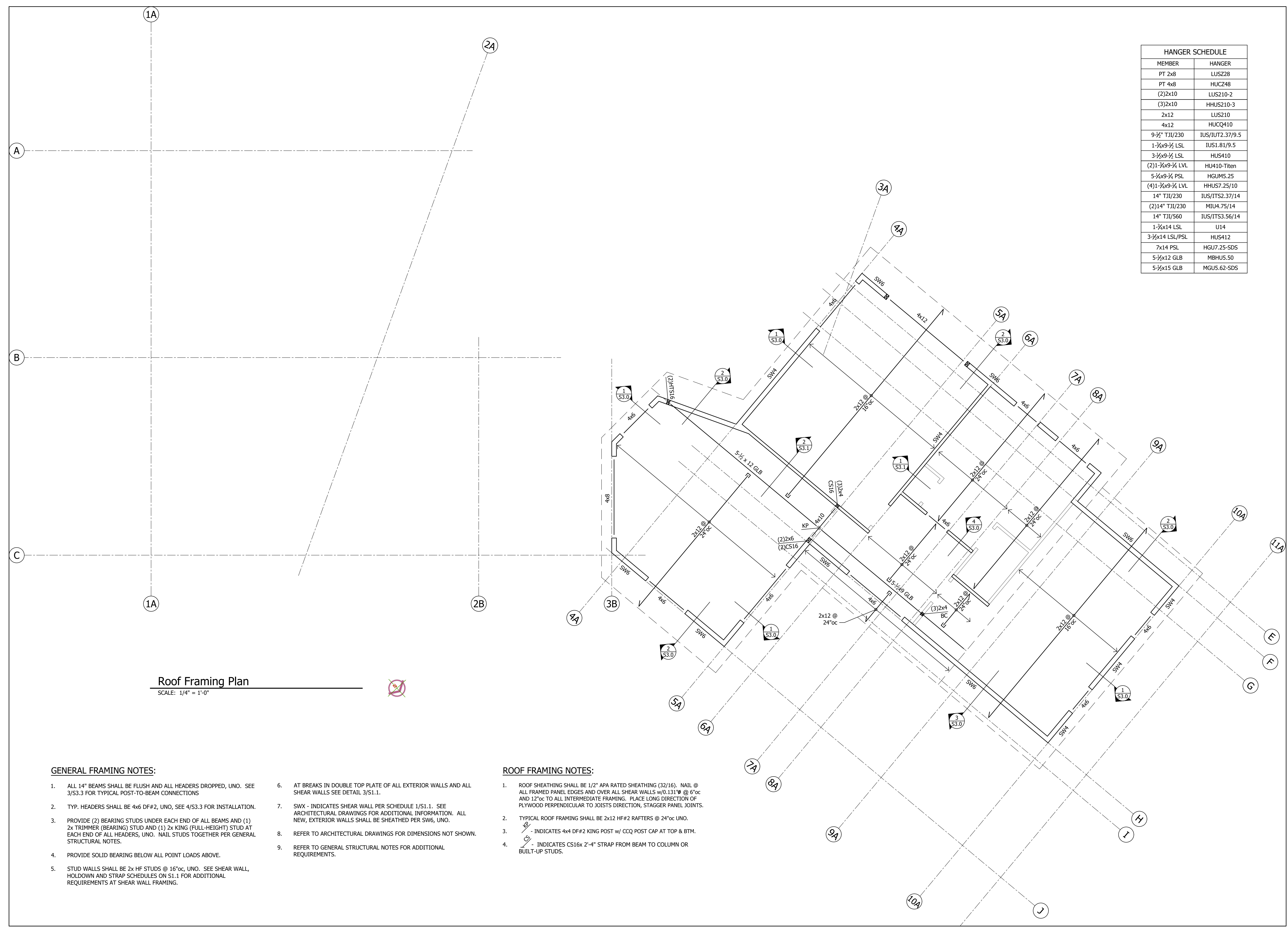


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HANGER SCHEDULE	
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PT 4x8	HUC248
(2)2x10	LUS210-2
(3)2x10	HHUS210-3
2x12	LUS210
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9-1/2" TJI/230	IUS/IUT2.37/9.5
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(4)1-3/4x9-1/2 LVL	HHUS7.25/10
14" TJI/230	IUS/ITS2.37/14
(2)14" TJI/230	MIU4.75/14
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1-3/4x14 LSL	U14
3-3/4x14 LSL/PSL	HUS412
7x14 PSL	HGU7.25-SDS
5-1/2x12 GLB	MBHUS.50
5-1/2x15 GLB	MGUS.62-SDS



**Roof Framing Plan**  
SCALE: 1/4" = 1'-0"

**GENERAL FRAMING NOTES:**

- ALL 14" BEAMS SHALL BE FLUSH AND ALL HEADERS DROPPED, UNO. SEE 3/S3.3 FOR TYPICAL POST-TO-BEAM CONNECTIONS
- TYP. HEADERS SHALL BE 4x6 DF#2, UNO, SEE 4/S3.3 FOR INSTALLATION.
- PROVIDE (2) BEARING STUDS UNDER EACH END OF ALL BEAMS AND (1) 2x TRIMMER (BEARING) STUD AND (1) 2x KING (FULL-HEIGHT) STUD AT EACH END OF ALL HEADERS, UNO. NAIL STUDS TOGETHER PER GENERAL STRUCTURAL NOTES.
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- REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

**ROOF FRAMING NOTES:**

- ROOF SHEATHING SHALL BE 1/2" APA RATED SHEATHING (32/16). NAIL @ ALL FRAMED PANEL EDGES AND OVER ALL SHEAR WALLS w/0.131" @ 6"oc AND 12"oc TO ALL INTERMEDIATE FRAMING. PLACE LONG DIRECTION OF PLYWOOD PERPENDICULAR TO JOISTS DIRECTION, STAGGER PANEL JOINTS.
- TYPICAL ROOF FRAMING SHALL BE 2x12 HF#2 RAFTERS @ 24"oc UNO.
- INDICATES 4x4 DF#2 KING POST w/ CCQ POST CAP AT TOP & BTM.
- INDICATES CS16x 2'-4" STRAP FROM BEAM TO COLUMN OR BUILT-UP STUDS.

Revision Issue Date Drawing Set

1/8/2024 Permit Set

**Roof Framing Plan**

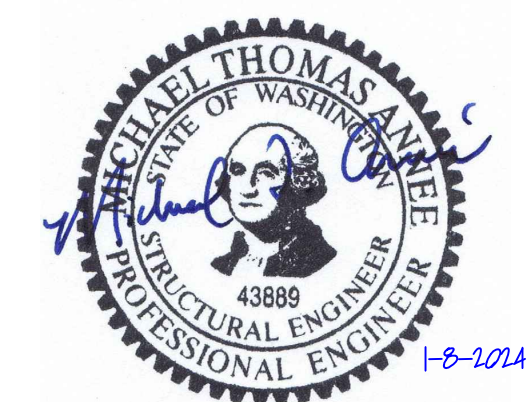
**S2.3**

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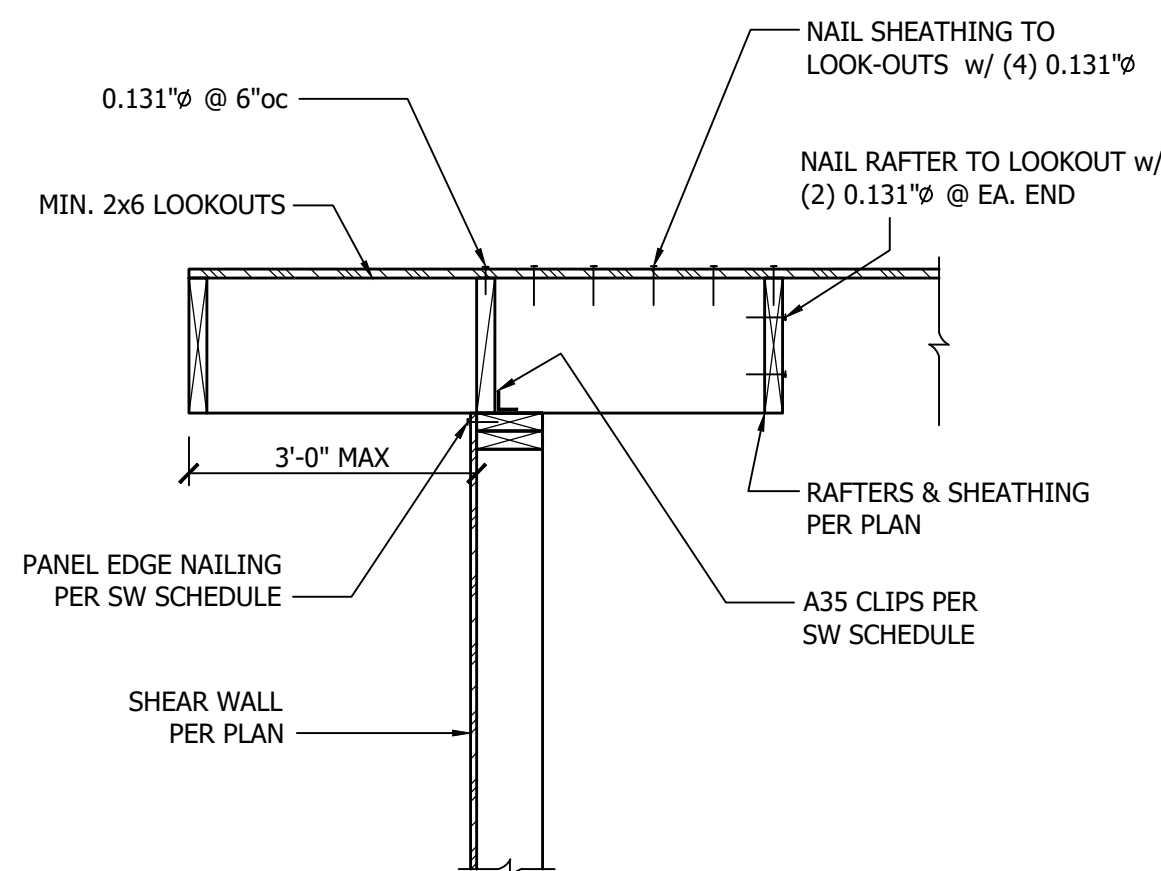
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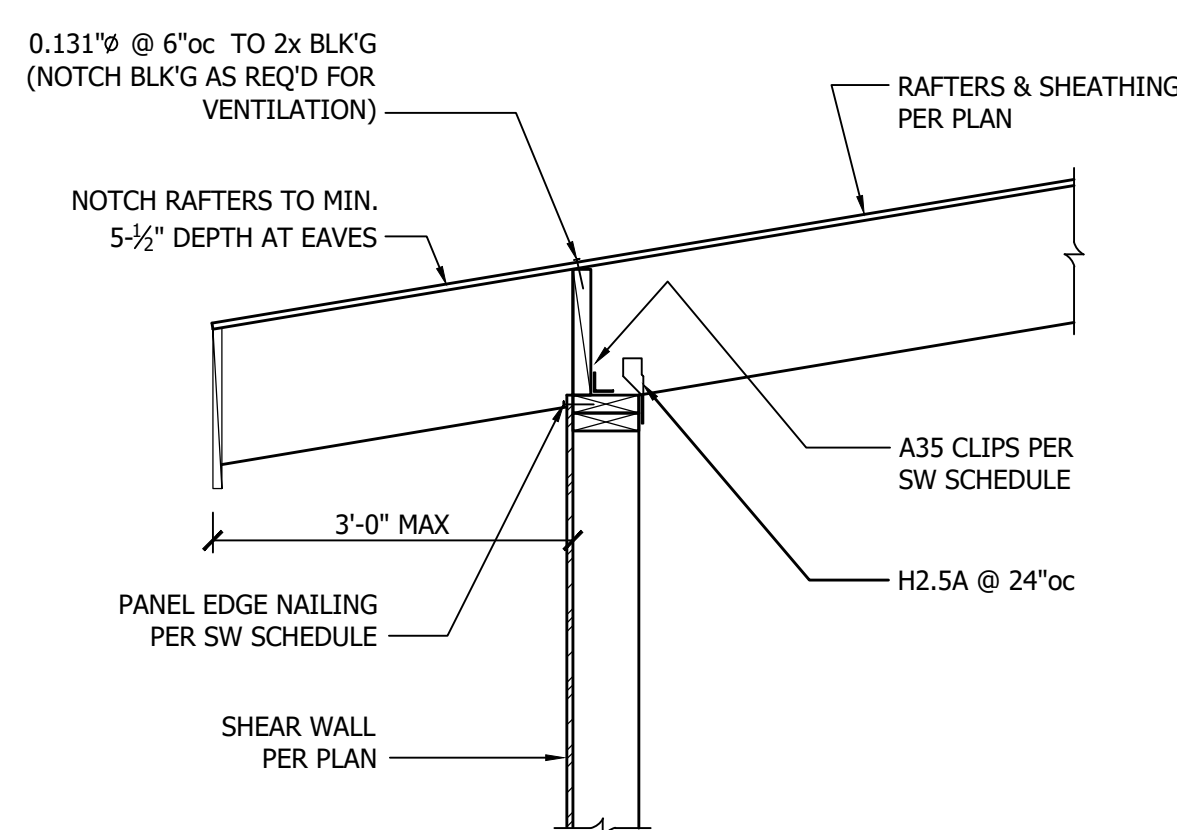
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Structural Details

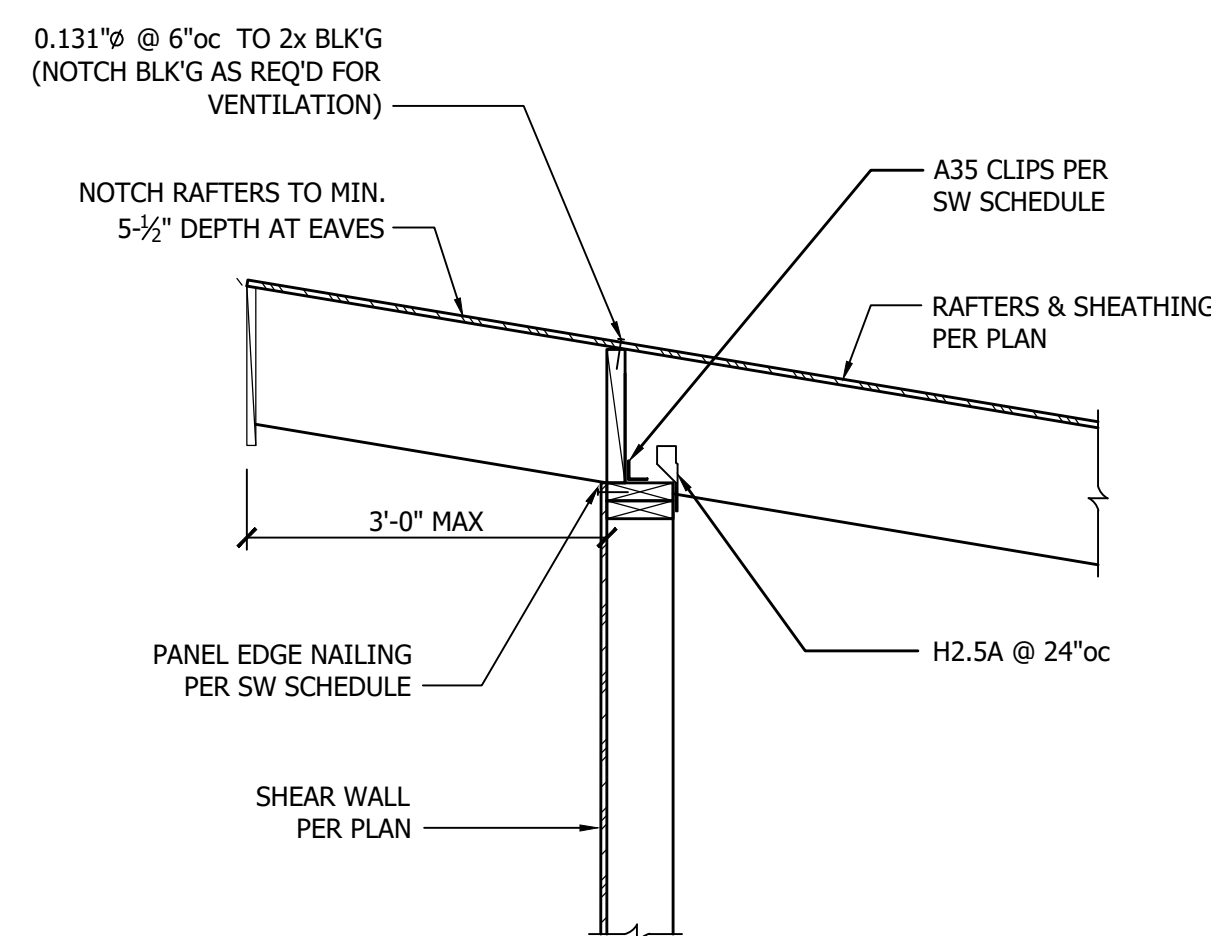
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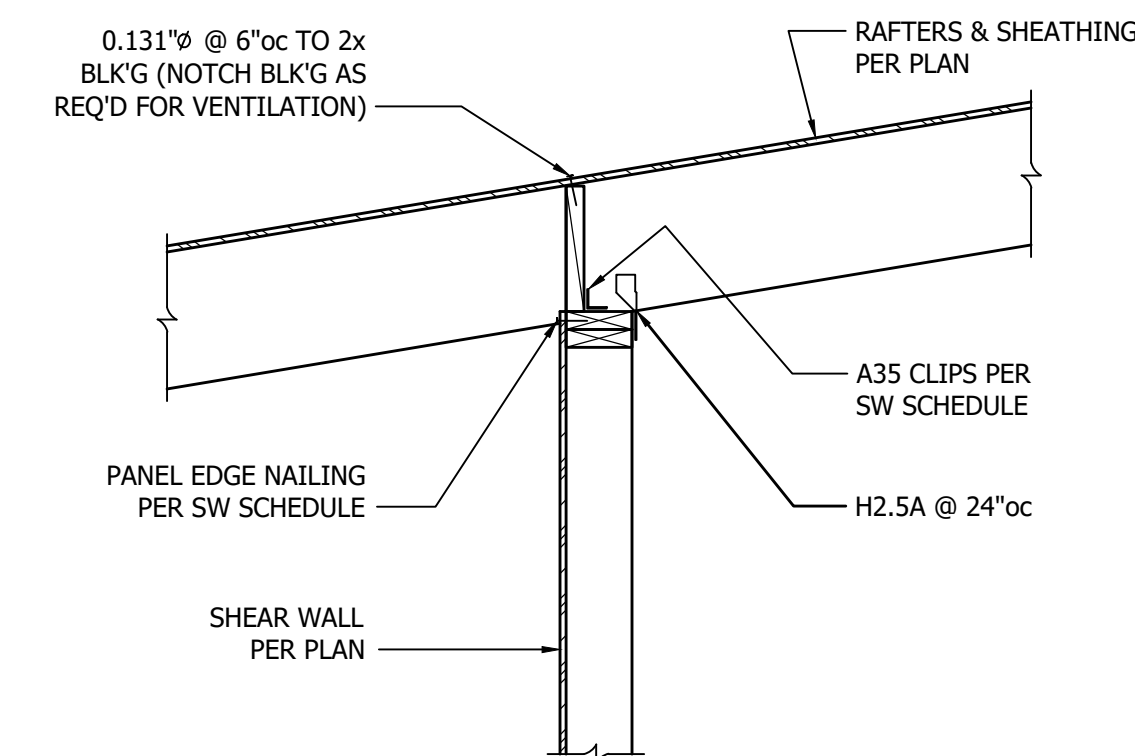
1 2x Rafter Parallel to Exterior Wall  
3/4" = 1'-0"



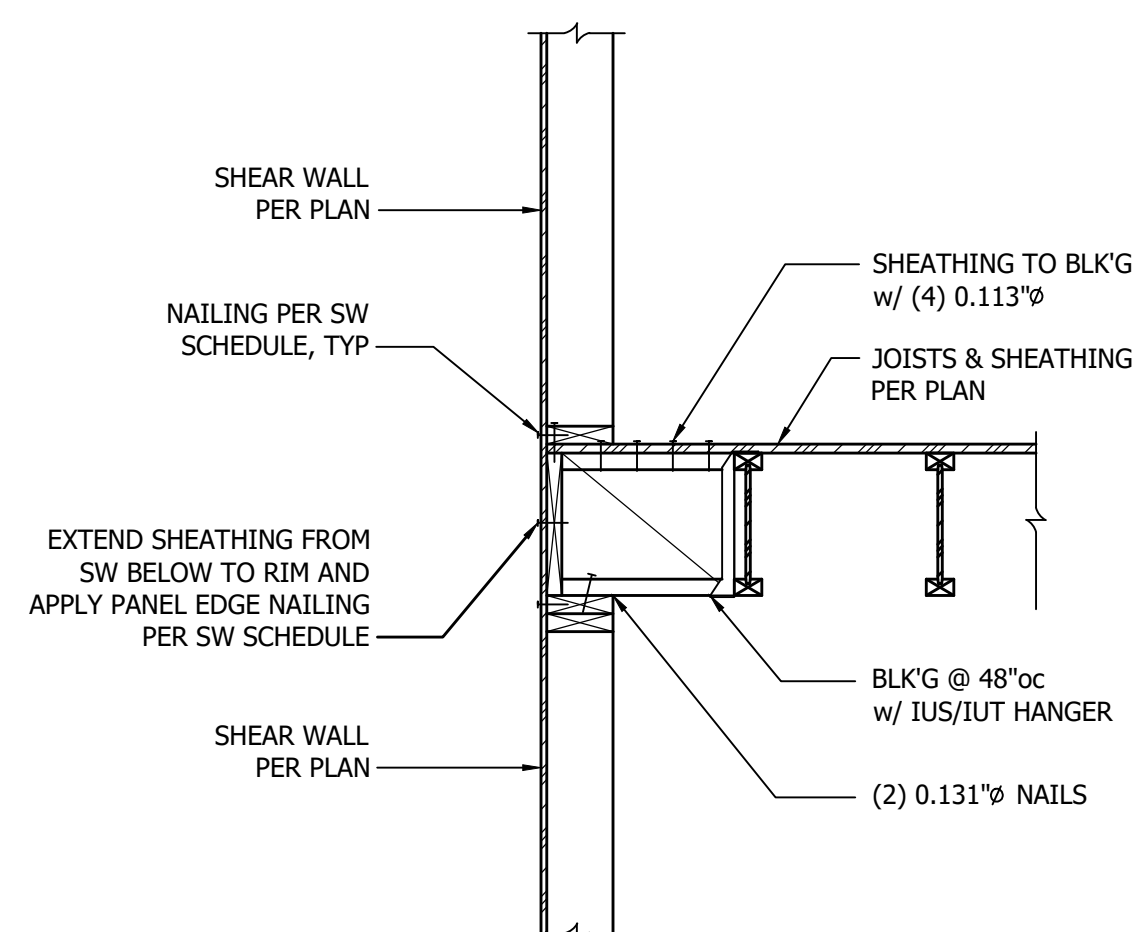
2 2x Rafters Perp. to Low Wall  
3/4" = 1'-0"



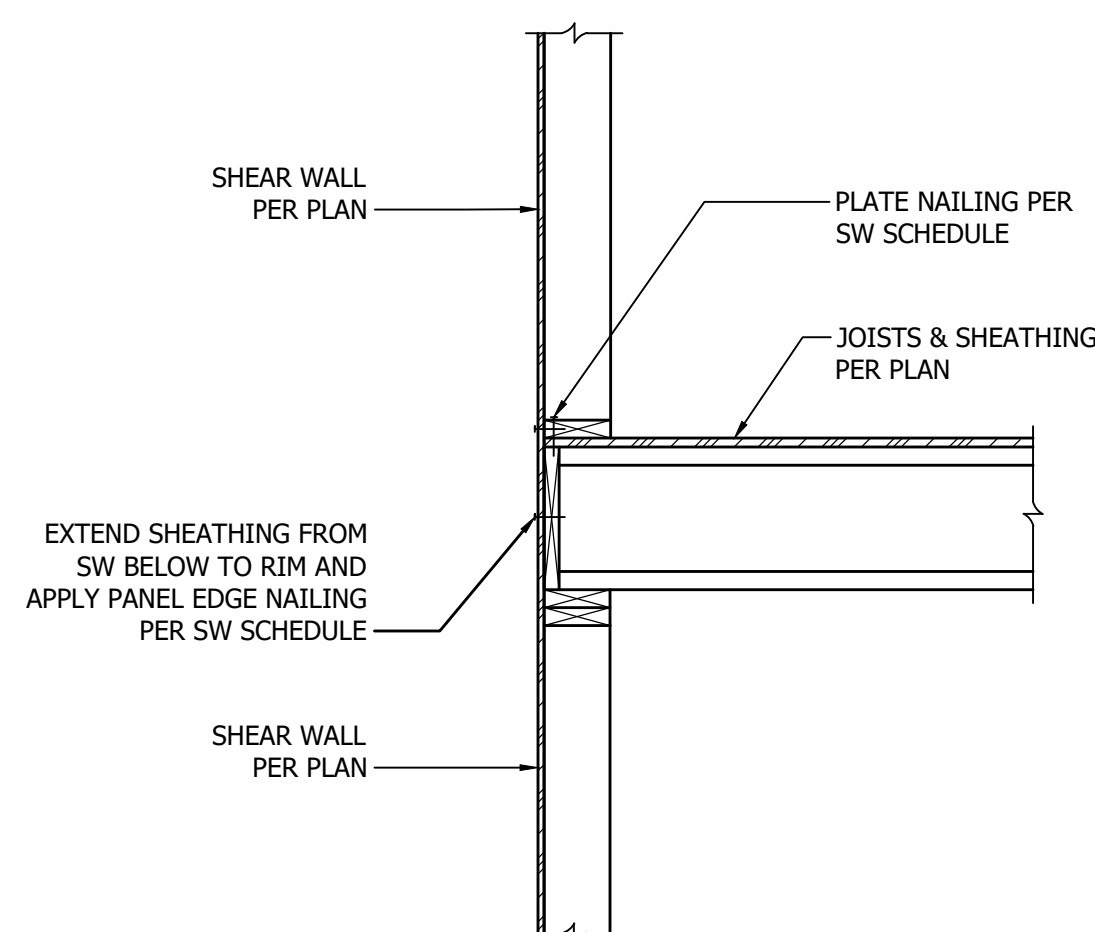
3 2x Rafters Perp. to High Wall  
3/4" = 1'-0"



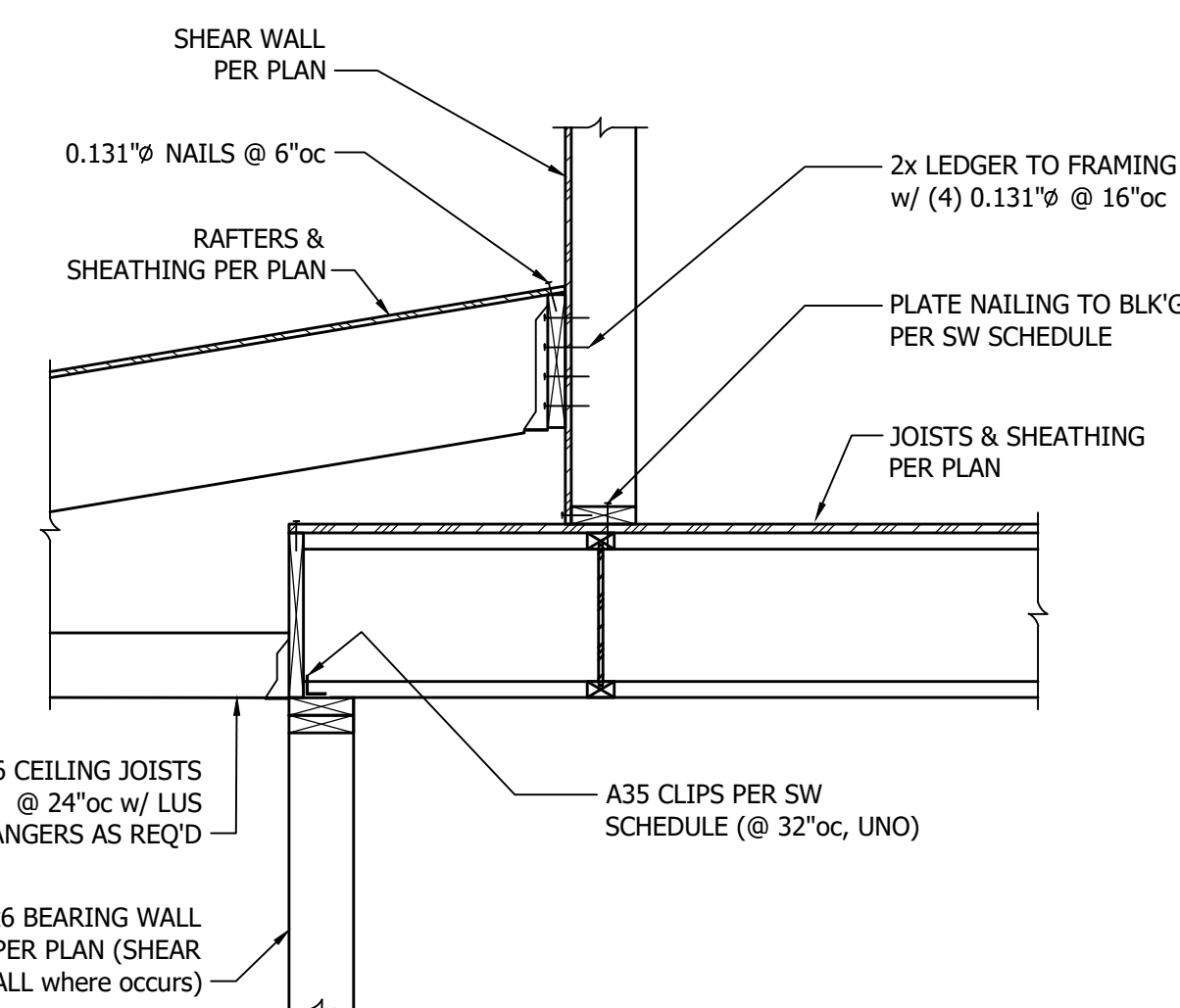
4 2x Rafters Perp. to Interior Wall  
3/4" = 1'-0"



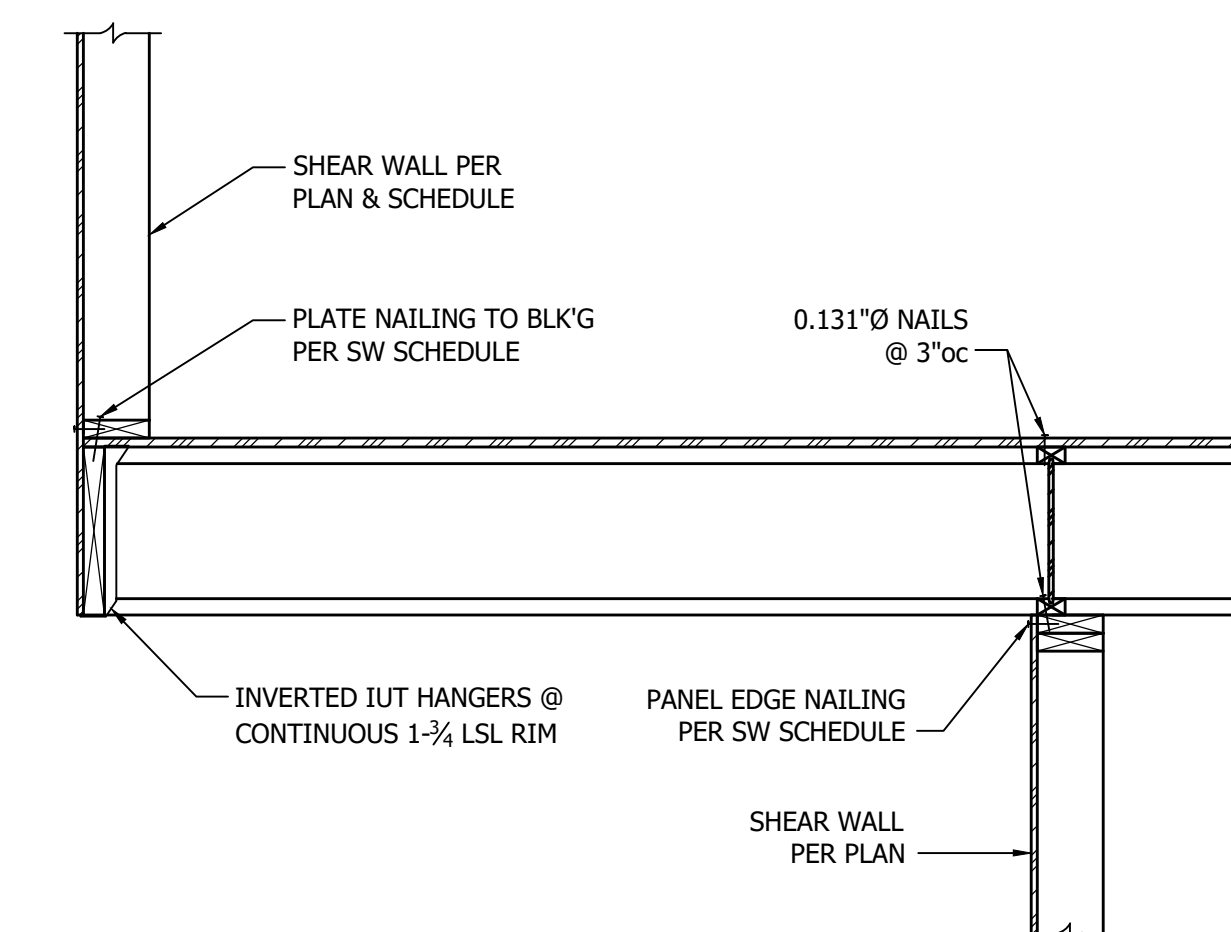
5 I-Joists Parallel to Exterior Wall  
3/4" = 1'-0"



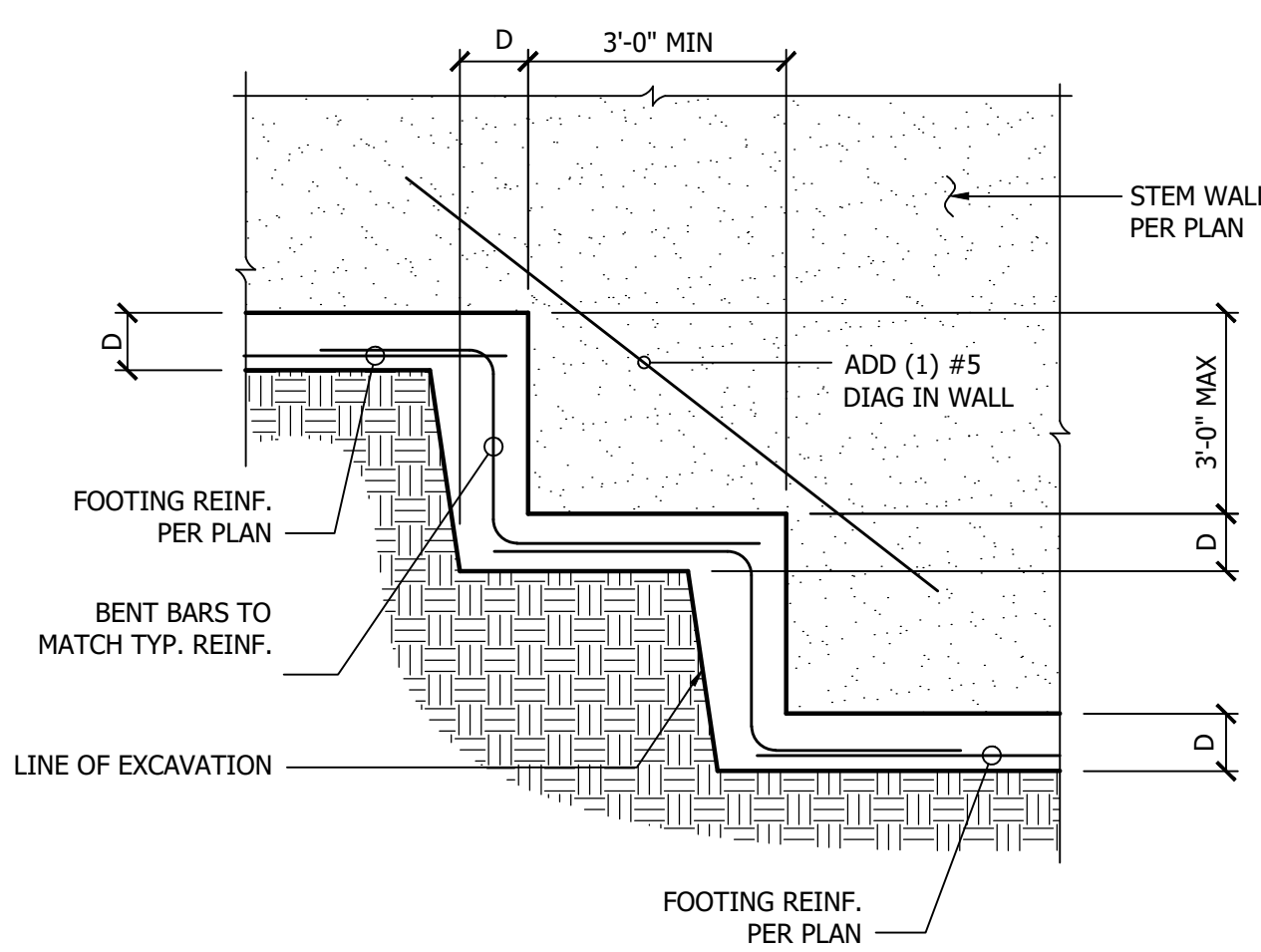
6 I-Joists Perpendicular to Exterior Wall  
3/4" = 1'-0"



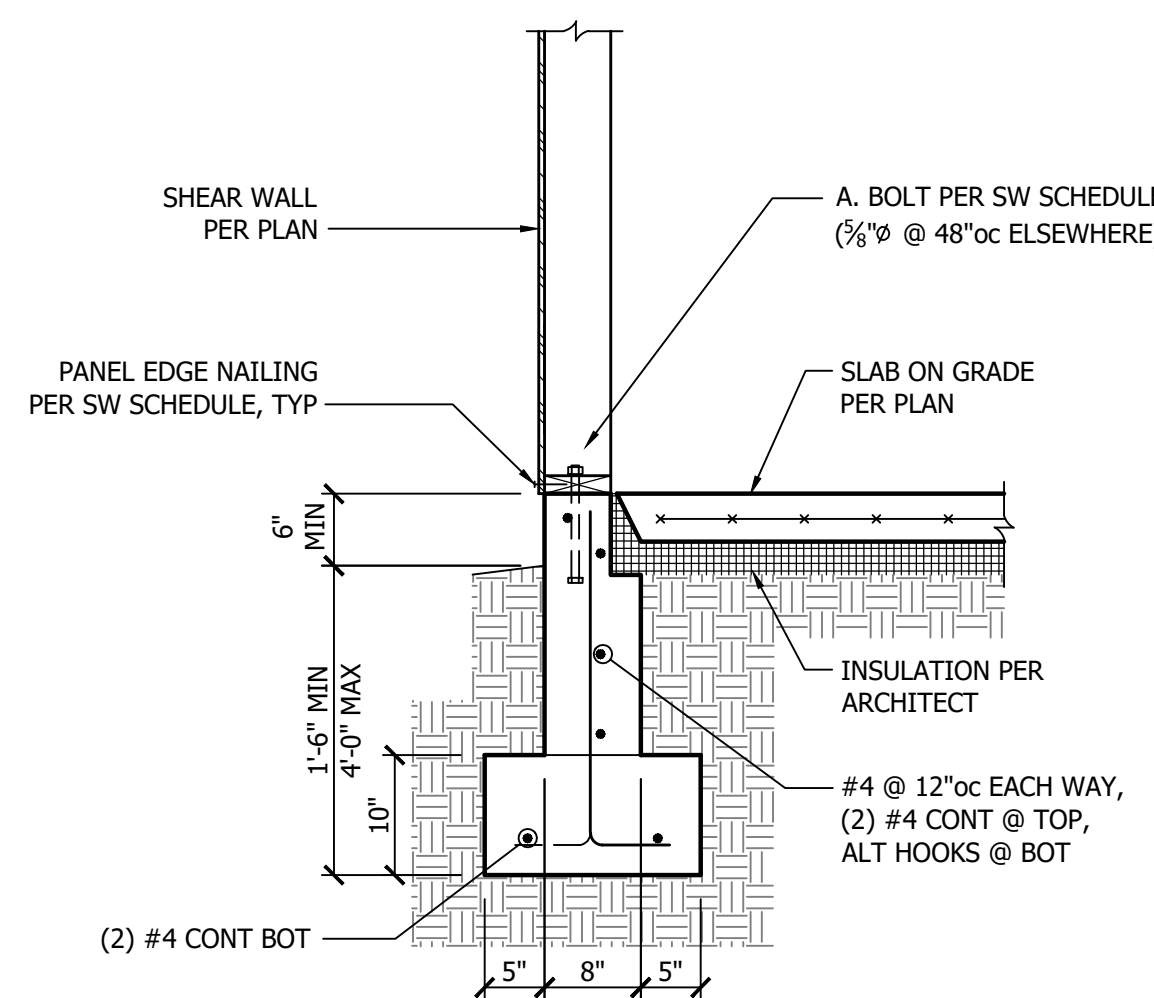
7 2x Low Roof at Parallel I-Joists  
3/4" = 1'-0"



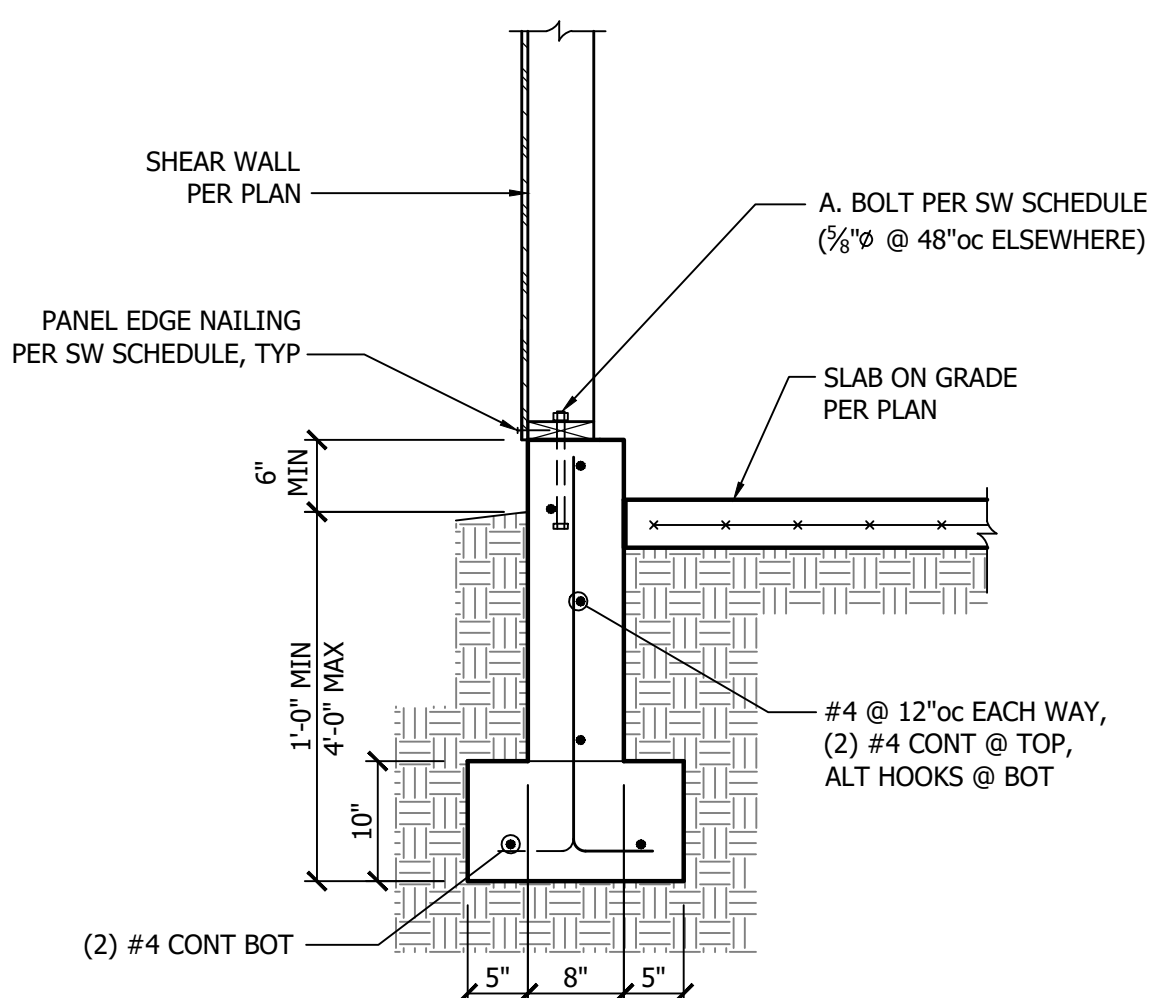
8 Shear Transfer @ Cantilevered I-Joists  
3/4" = 1'-0"



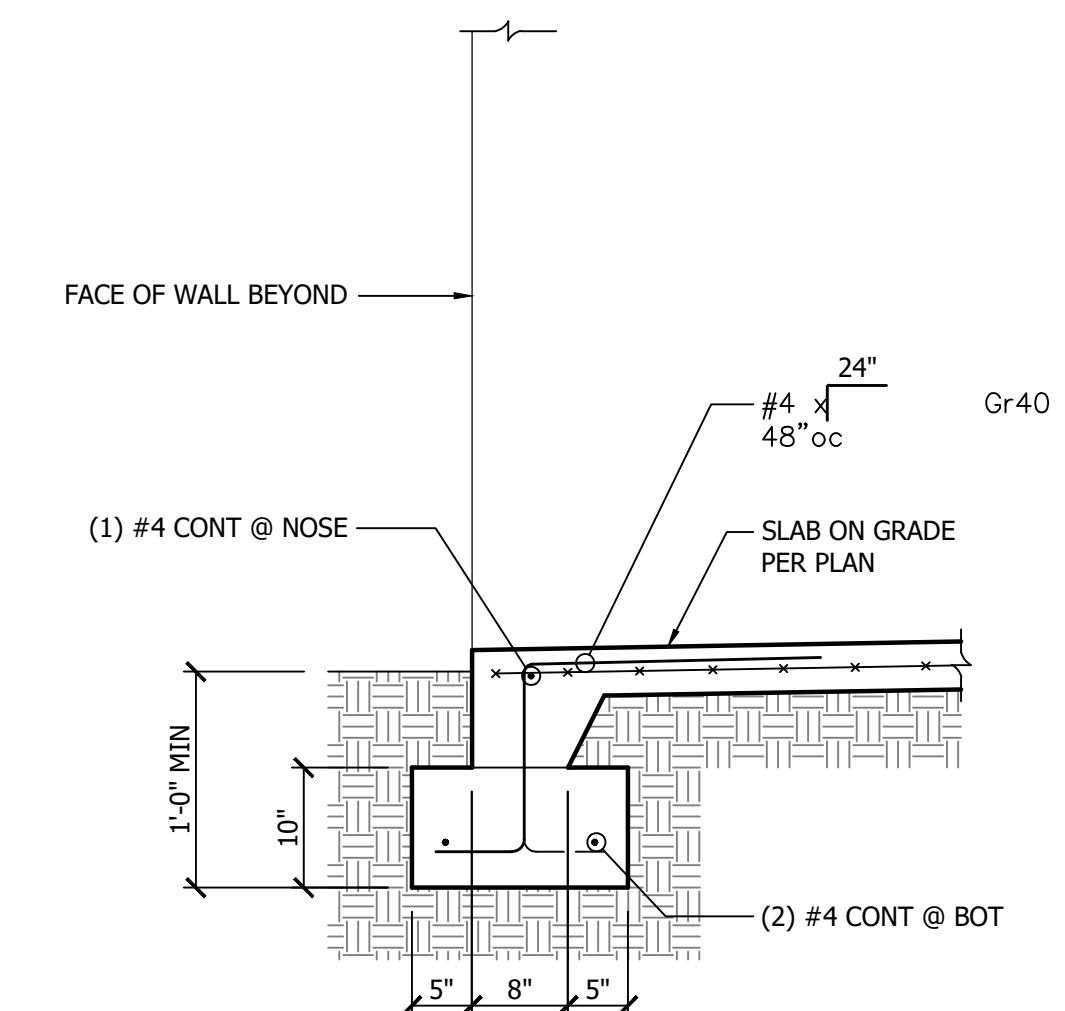
9 Stepped Footing, Typ.  
3/4" = 1'-0"



10 Stem Wall/Footing @ Exterior Wall  
3/4" = 1'-0"



11 Stem Wall/Footing @ Exterior Garage Wall  
3/4" = 1'-0"

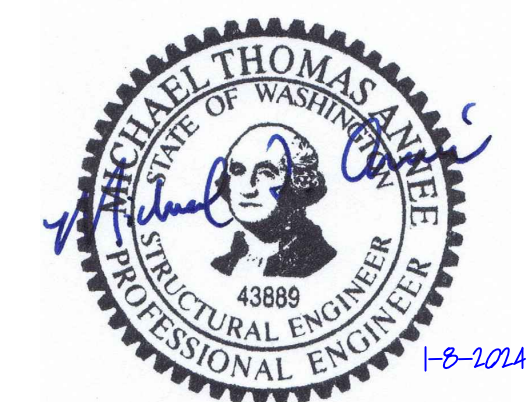


12 Footing @ Garage Opening  
3/4" = 1'-0"



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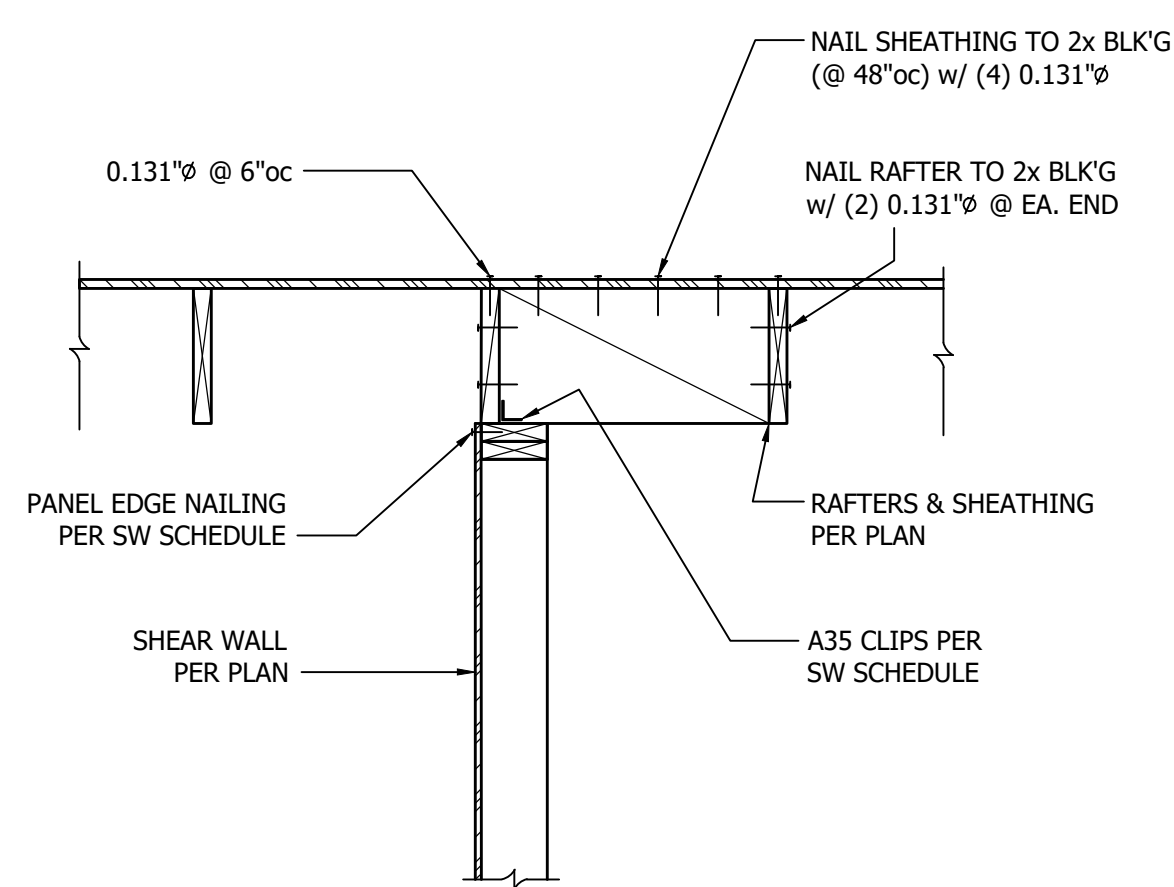
Rawson Remodel  
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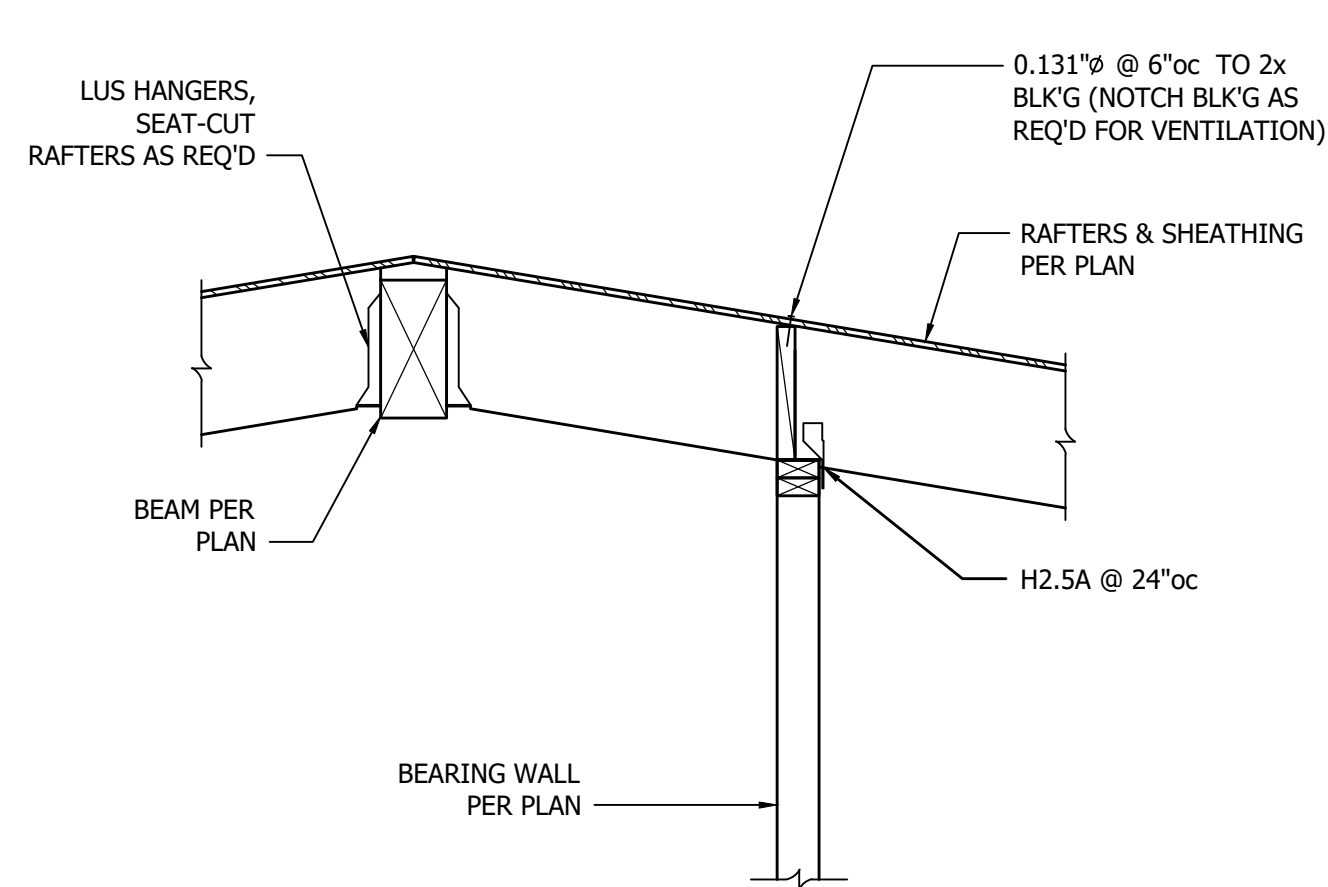
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Structural Details

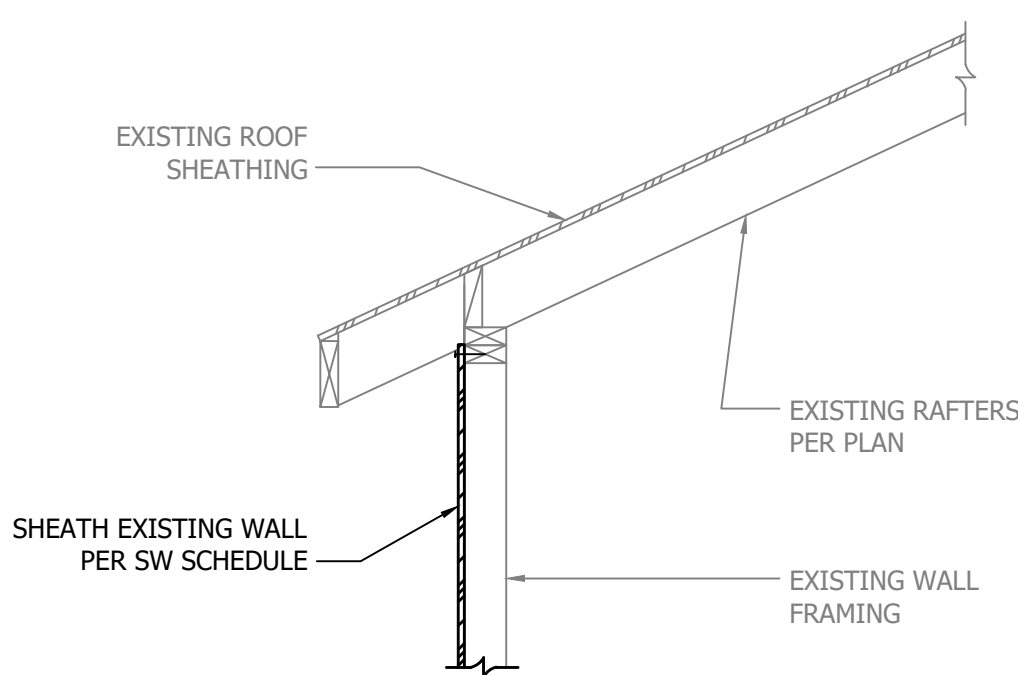
S3.1



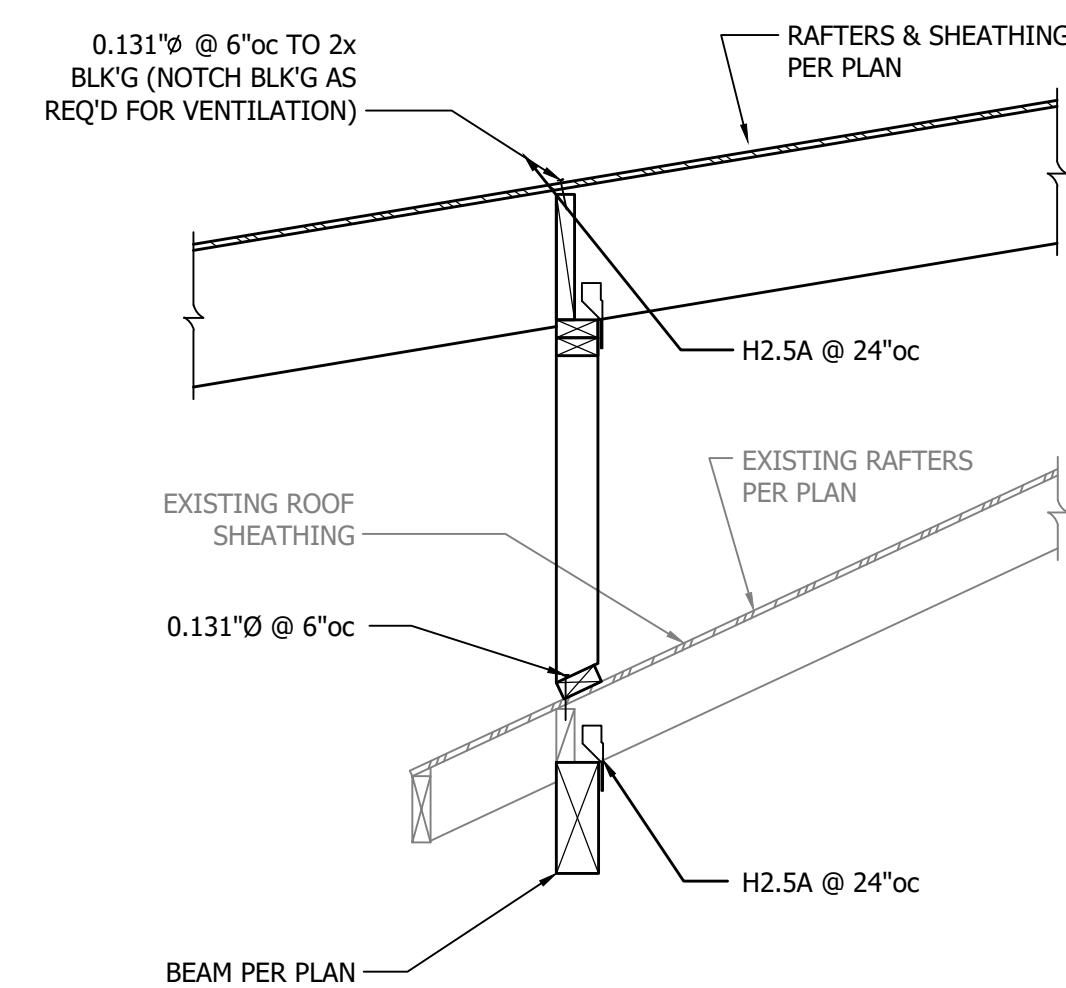
1 2x Rafter Parallel to Interior Shear Wall  
3/4" = 1'-0"



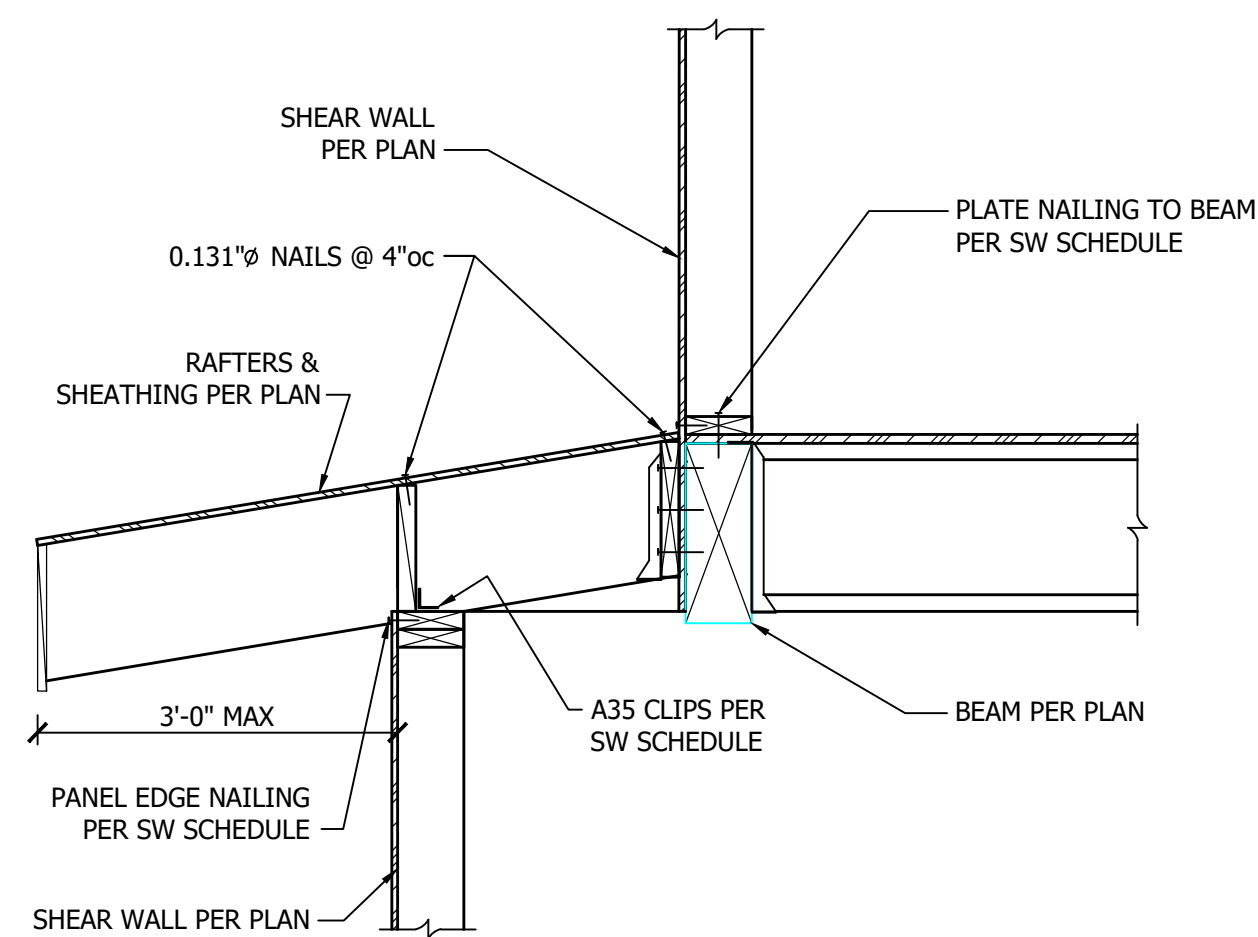
2 2x Rafters at Ridge & Interior Bearing Wall  
3/4" = 1'-0"



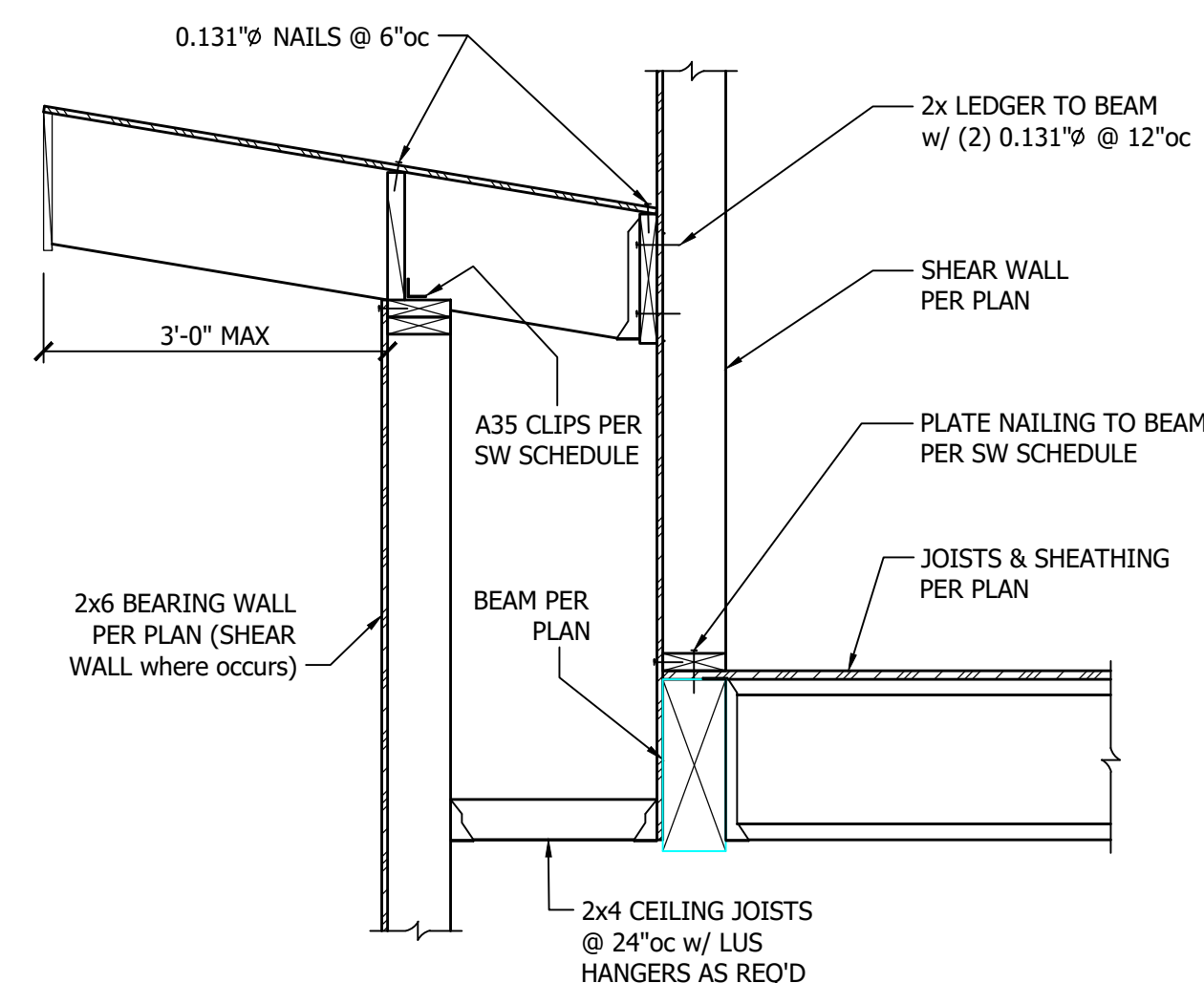
3 Shear Wall Sheathing at Existing, Exterior Wall  
3/4" = 1'-0"



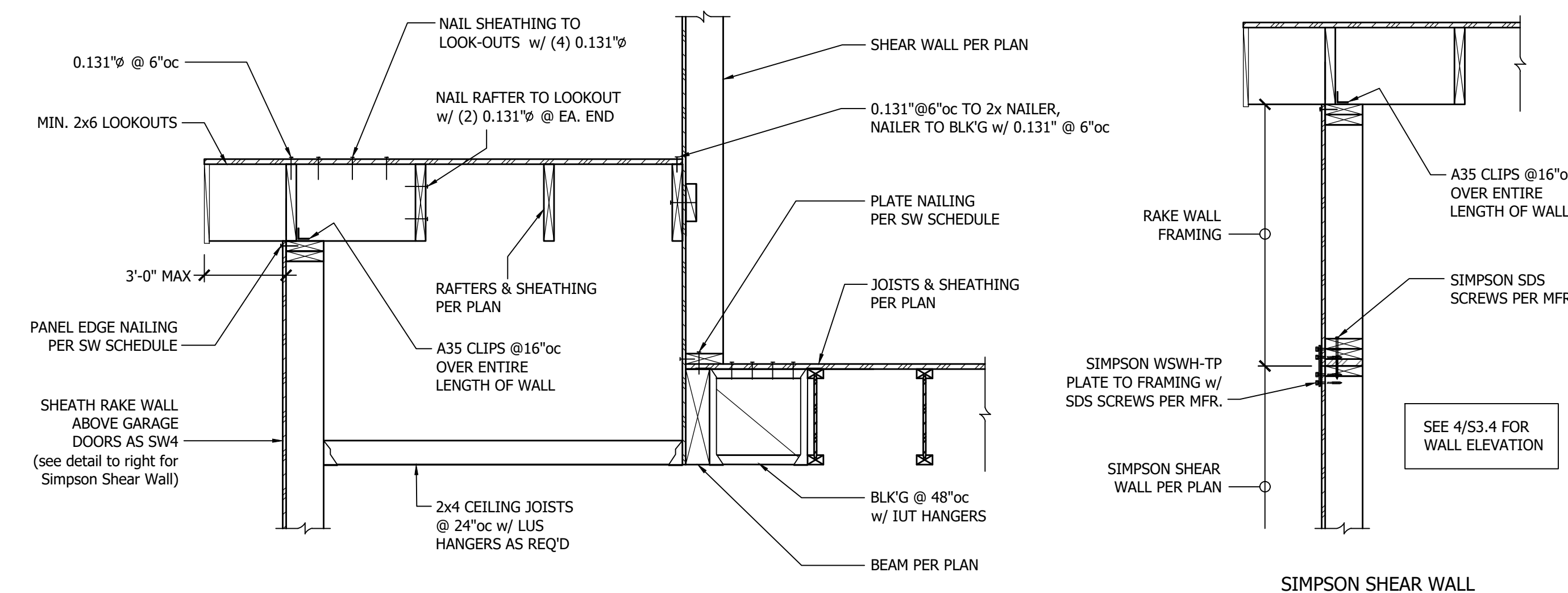
4 New Header Supporting Existing Rafters  
3/4" = 1'-0"



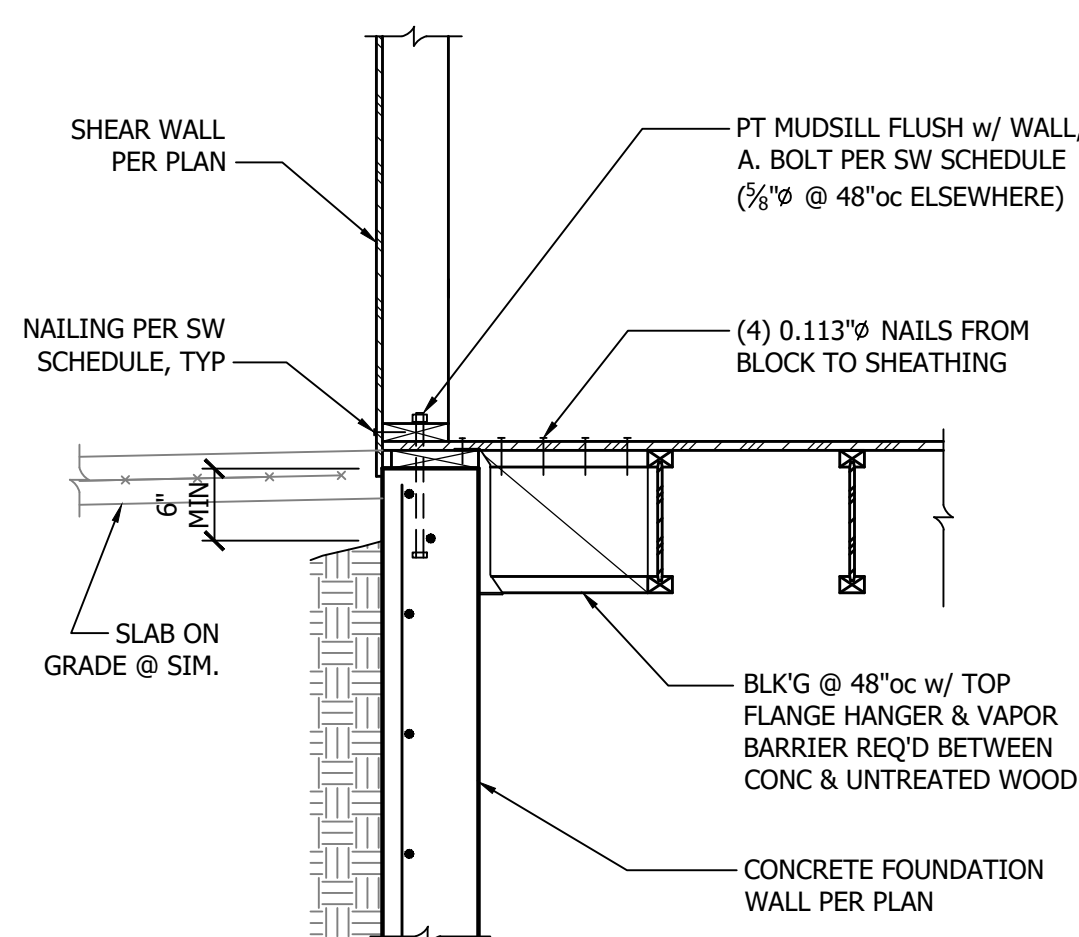
5 I-Joists Parallel to Exterior Wall  
3/4" = 1'-0"



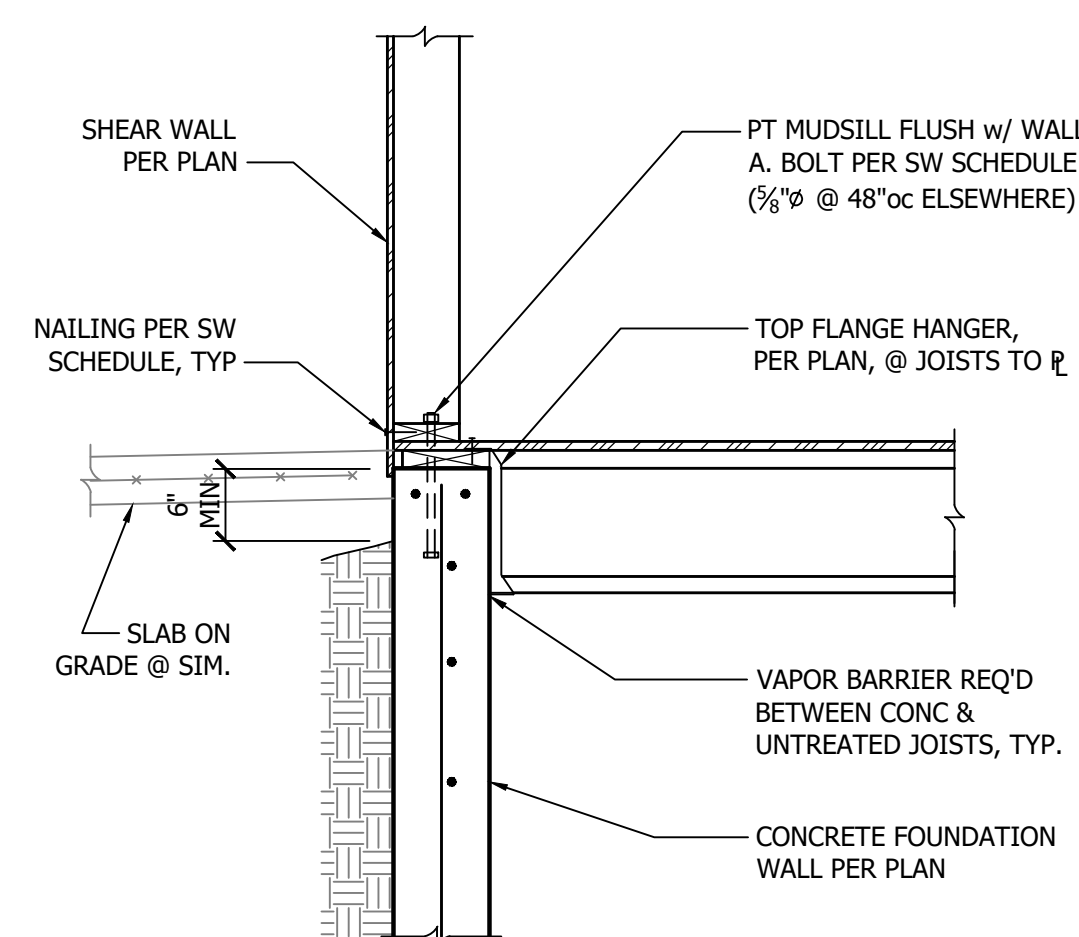
6 I-Joists Perpendicular to Exterior Wall  
3/4" = 1'-0"



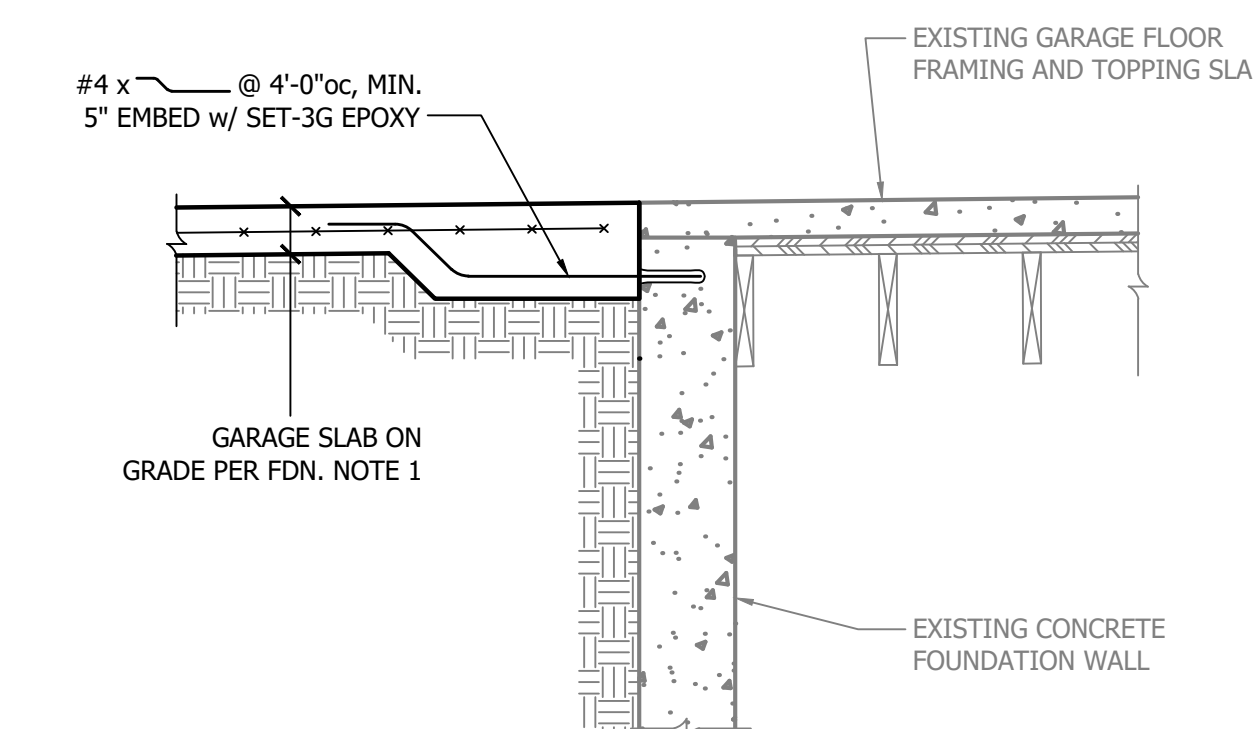
7 Low Roof Rafters Parallel to Floor Framing  
3/4" = 1'-0"



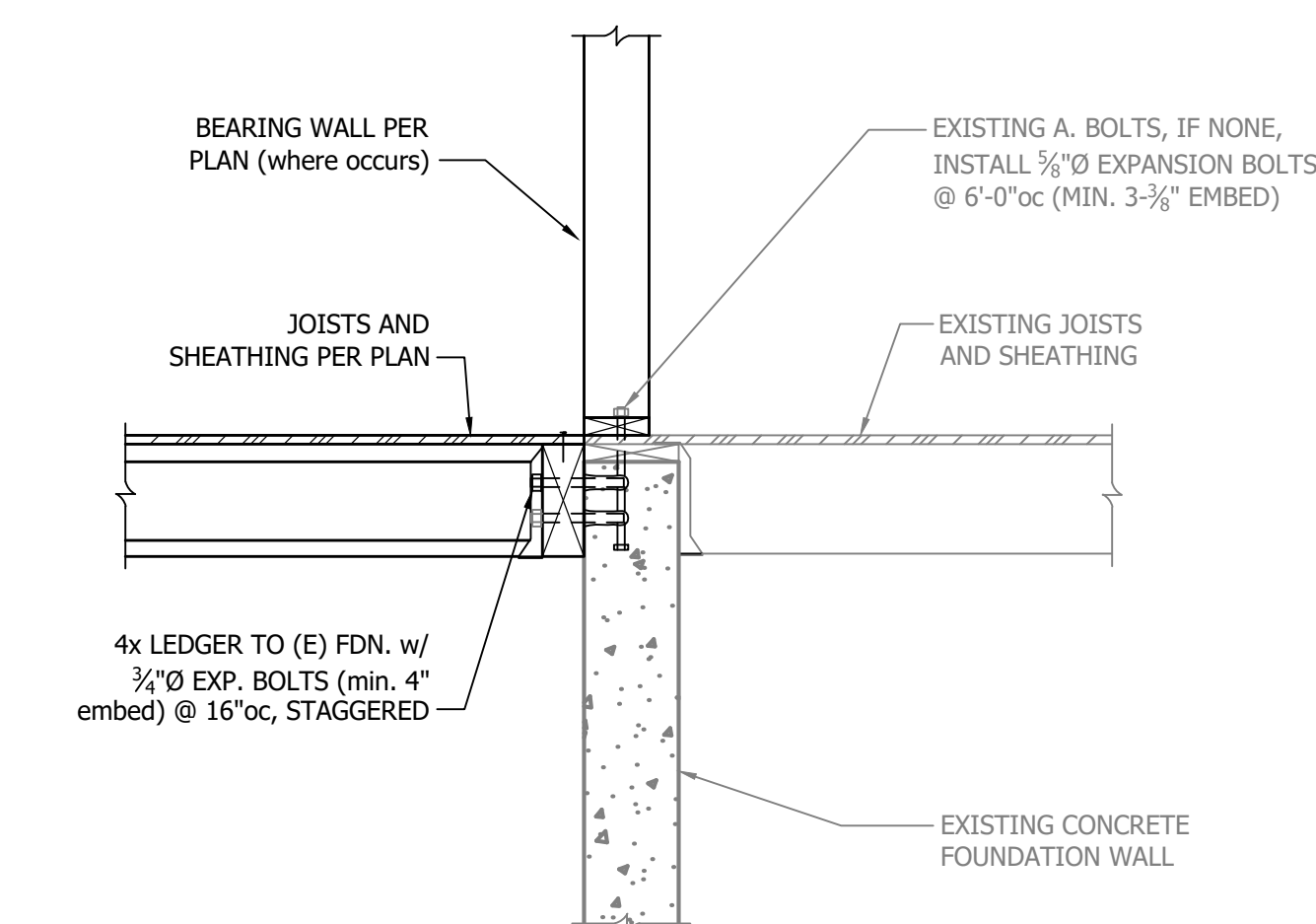
9 TJI Parallel to Flush Foundation Wall  
3/4" = 1'-0"



10 TJI Perp. to Flush Foundation Wall  
3/4" = 1'-0"



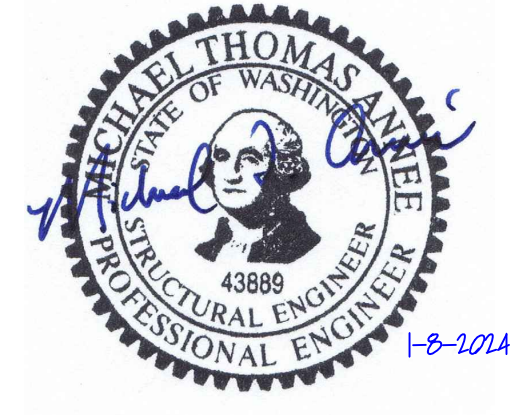
11 New Garage Slab Adjacent to Existing Garage  
3/4" = 1'-0"



12 I-Joists Perp. to Existing Foundation Wall  
3/4" = 1'-0"



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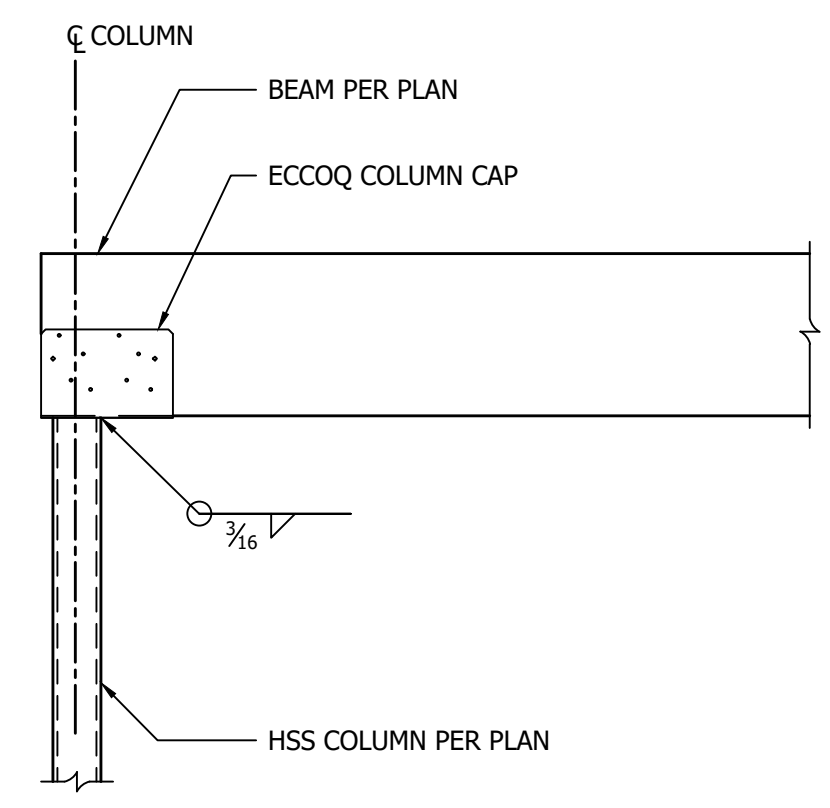
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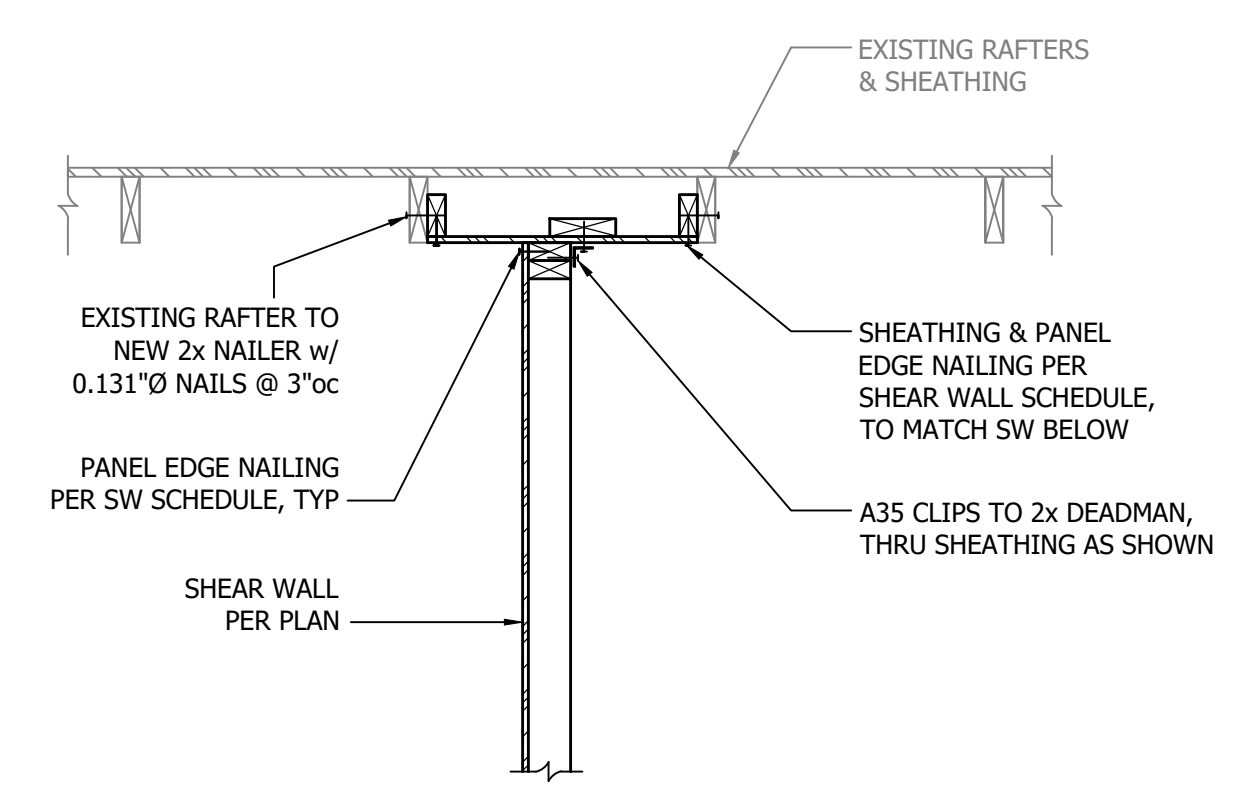
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Structural Details

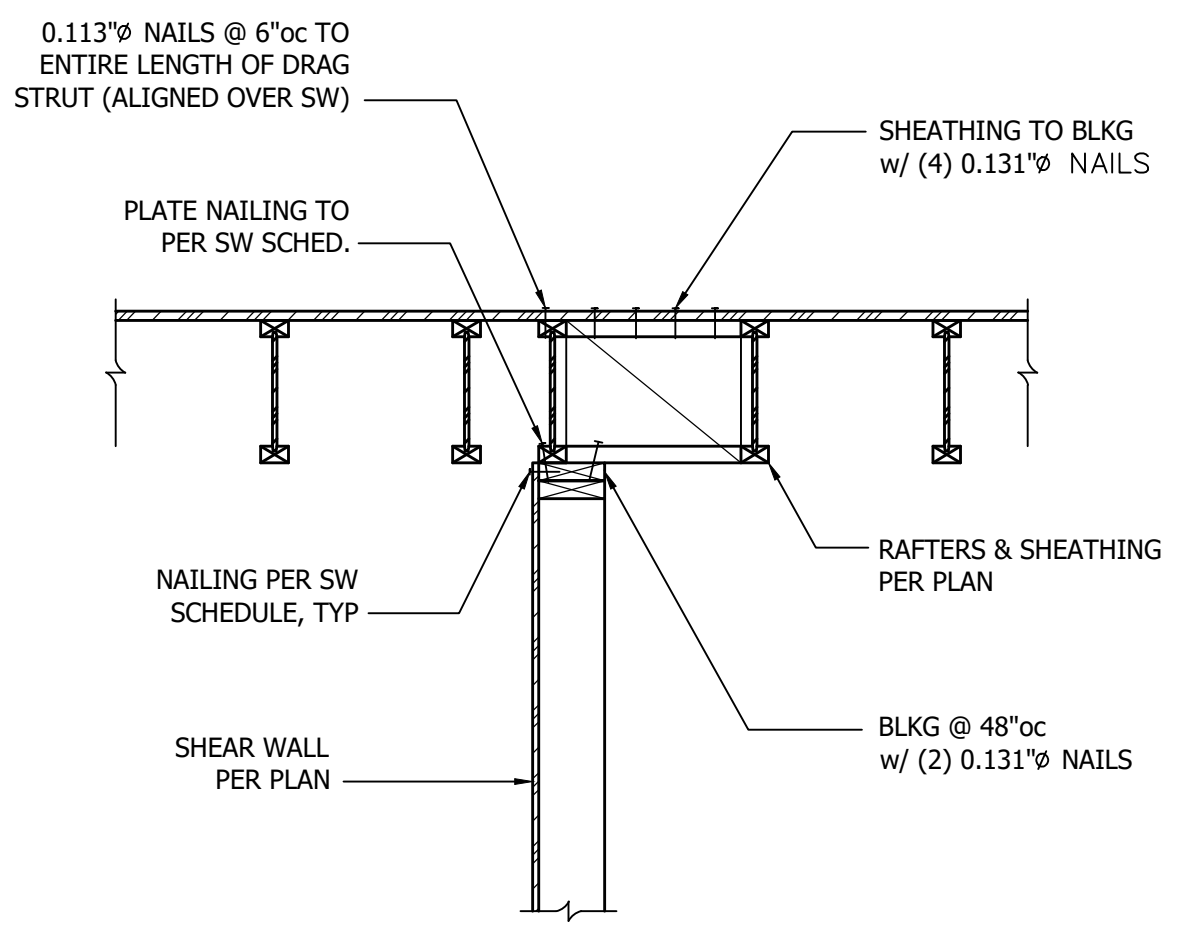
S3.2



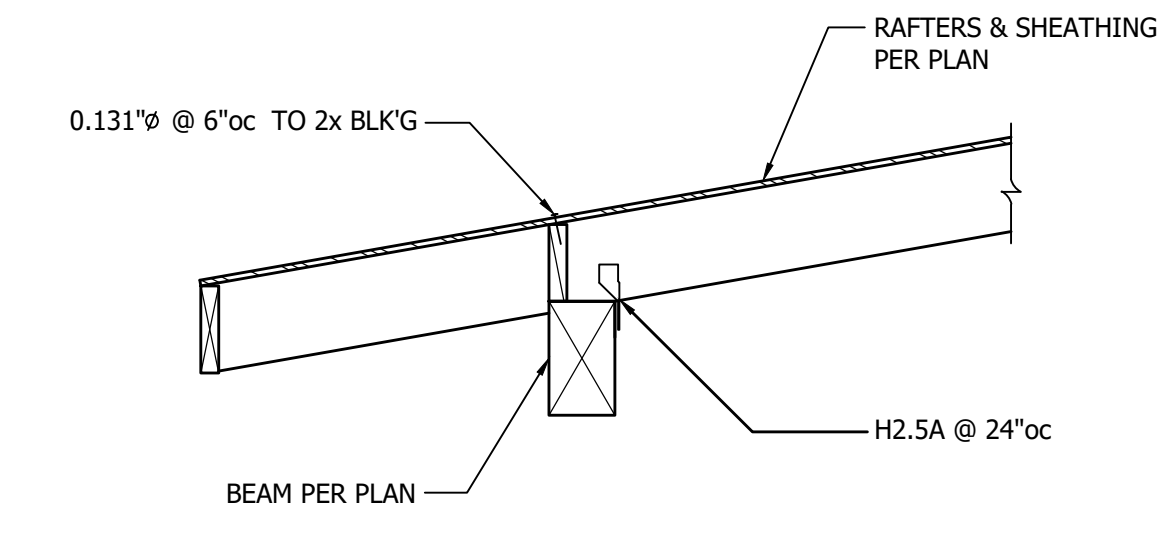
**1** Wood Beam to HSS Column, Typ.  
 3/4" = 1'-0"



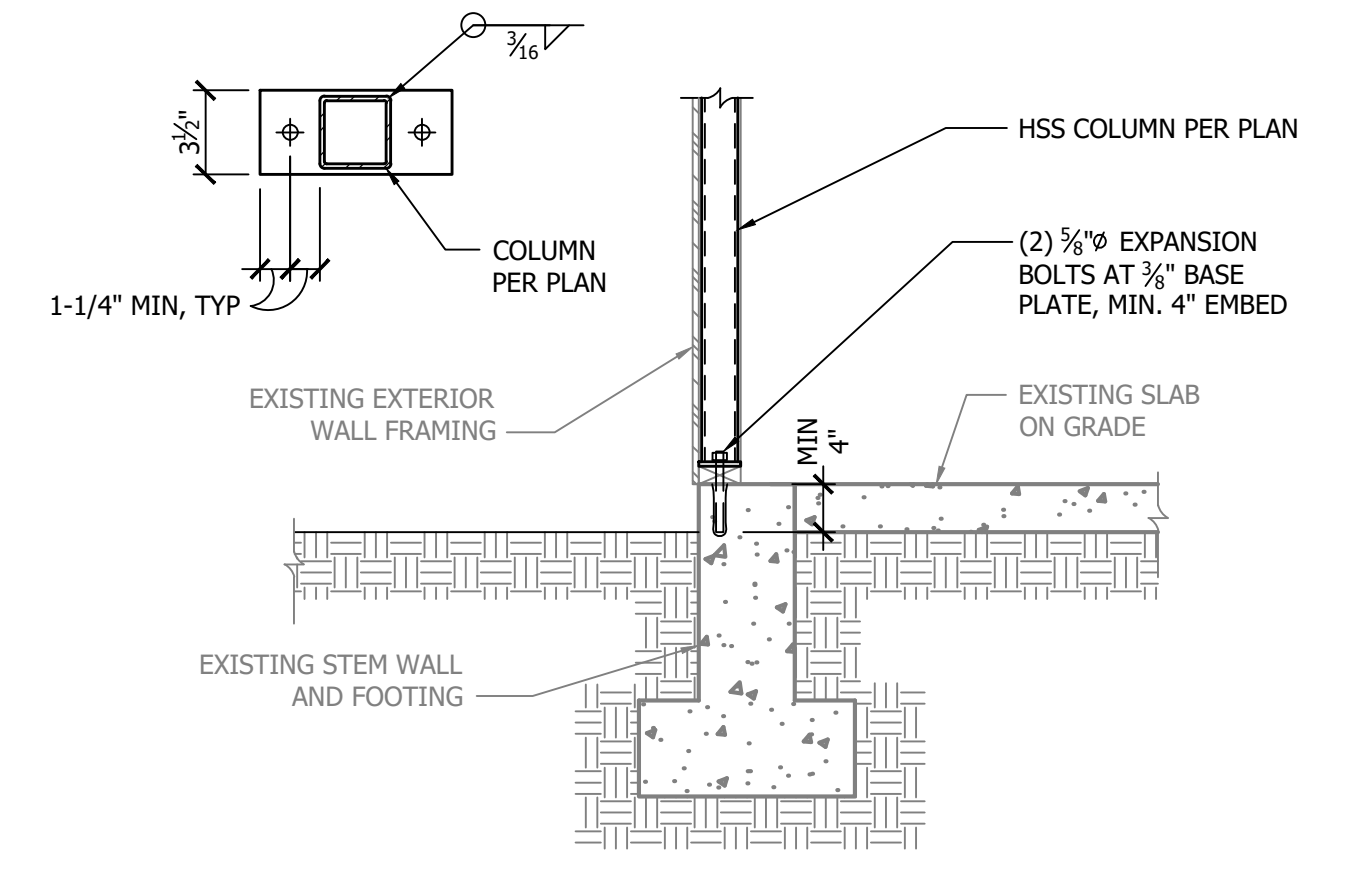
**2** New, Interior Shear Wall II to Existing Rafters  
 3/4" = 1'-0"



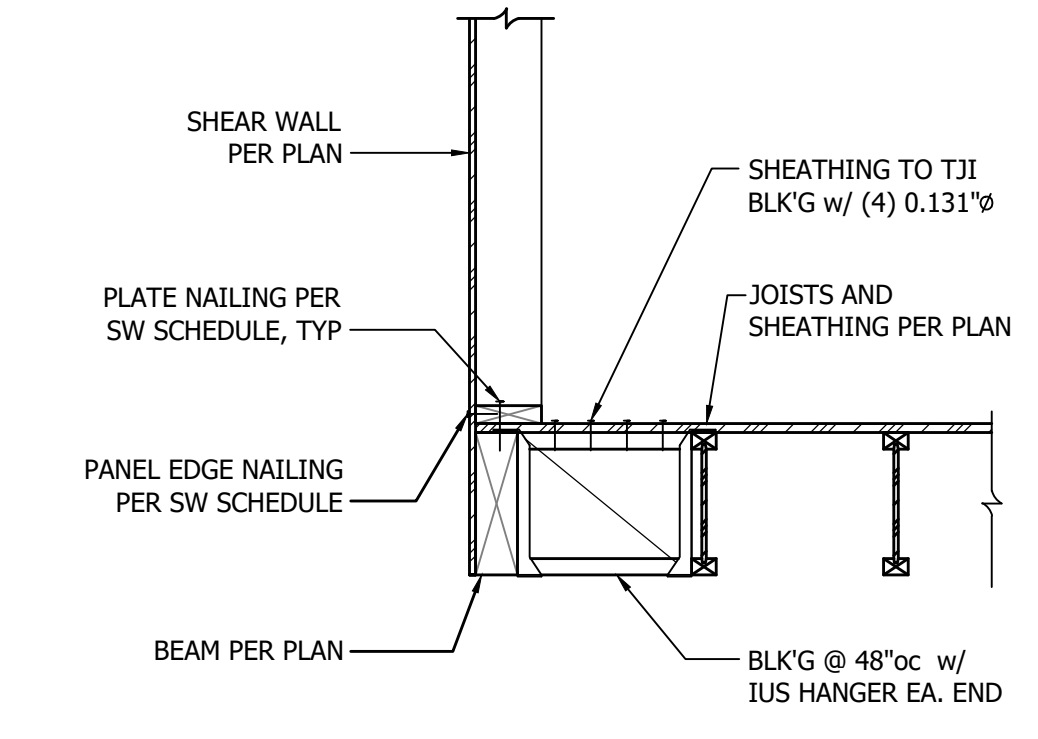
**3** Interior Shear Wall II to I-Joists Above  
 3/4" = 1'-0"



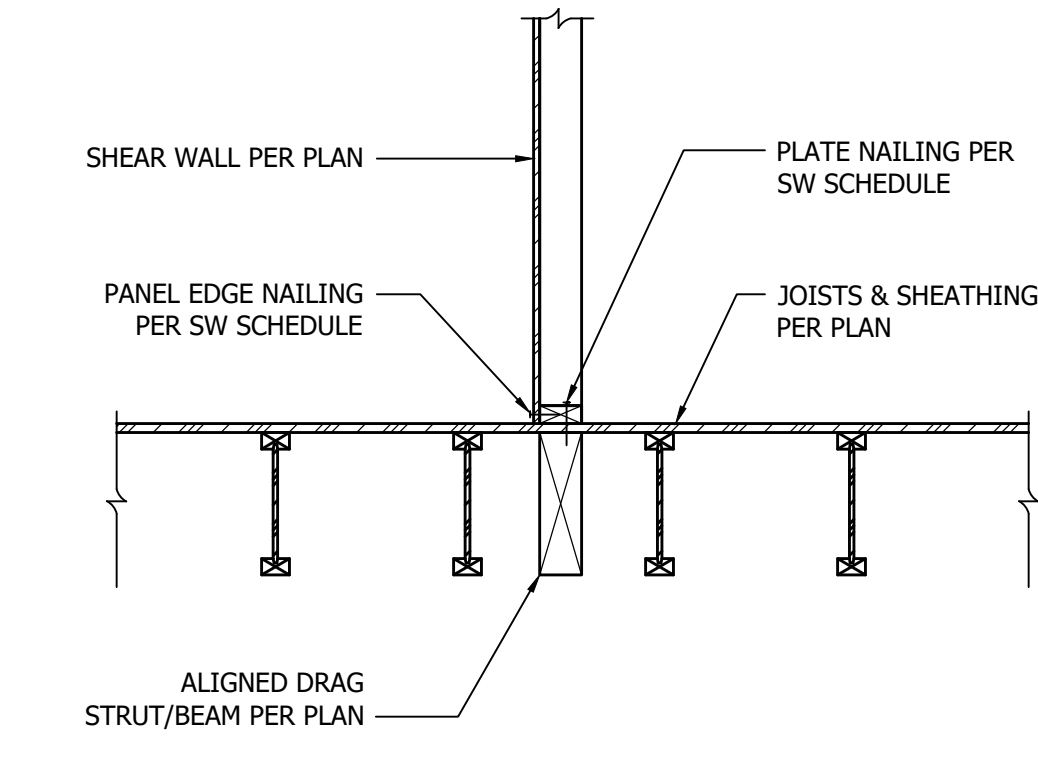
**4** 2x Rafters Perp. to Beam  
 3/4" = 1'-0"



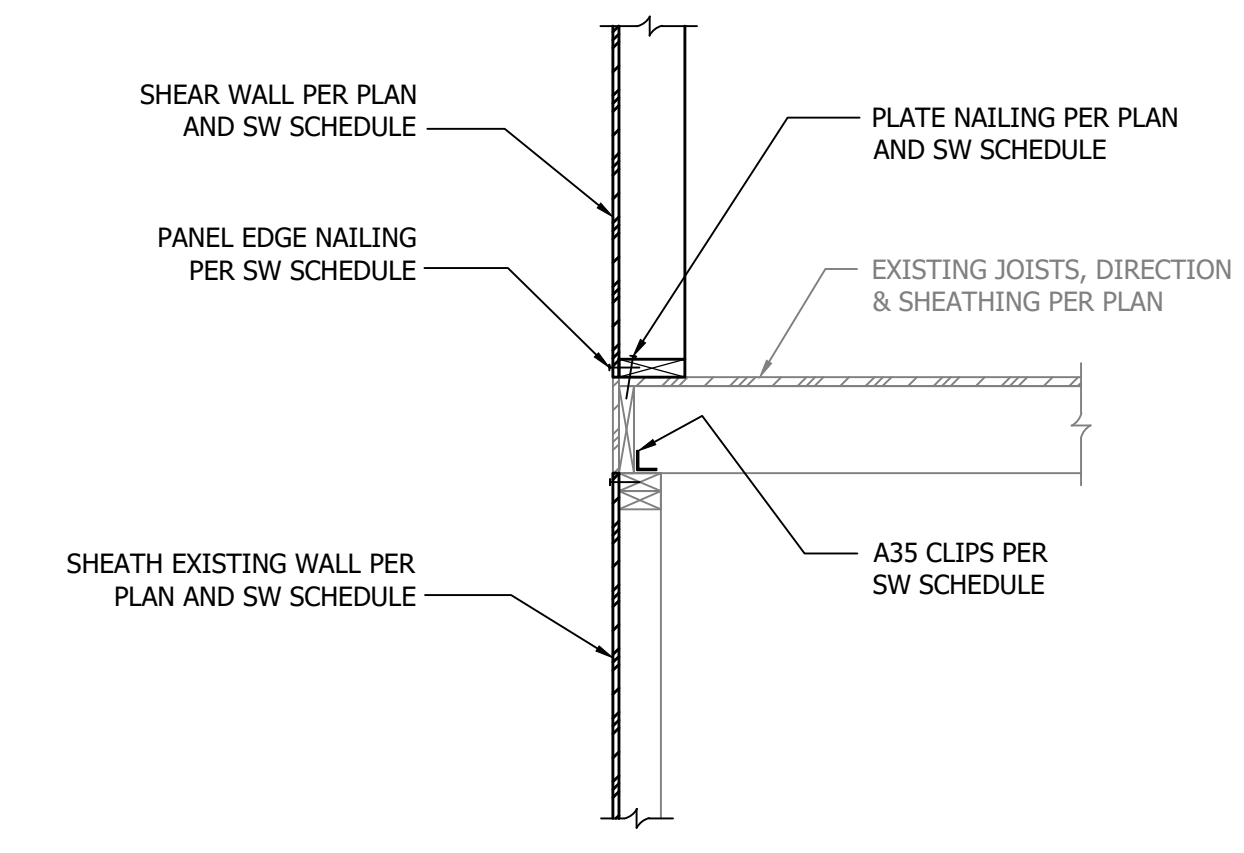
**5** HSS Column to Existing Fdn. Wall  
 3/4" = 1'-0"



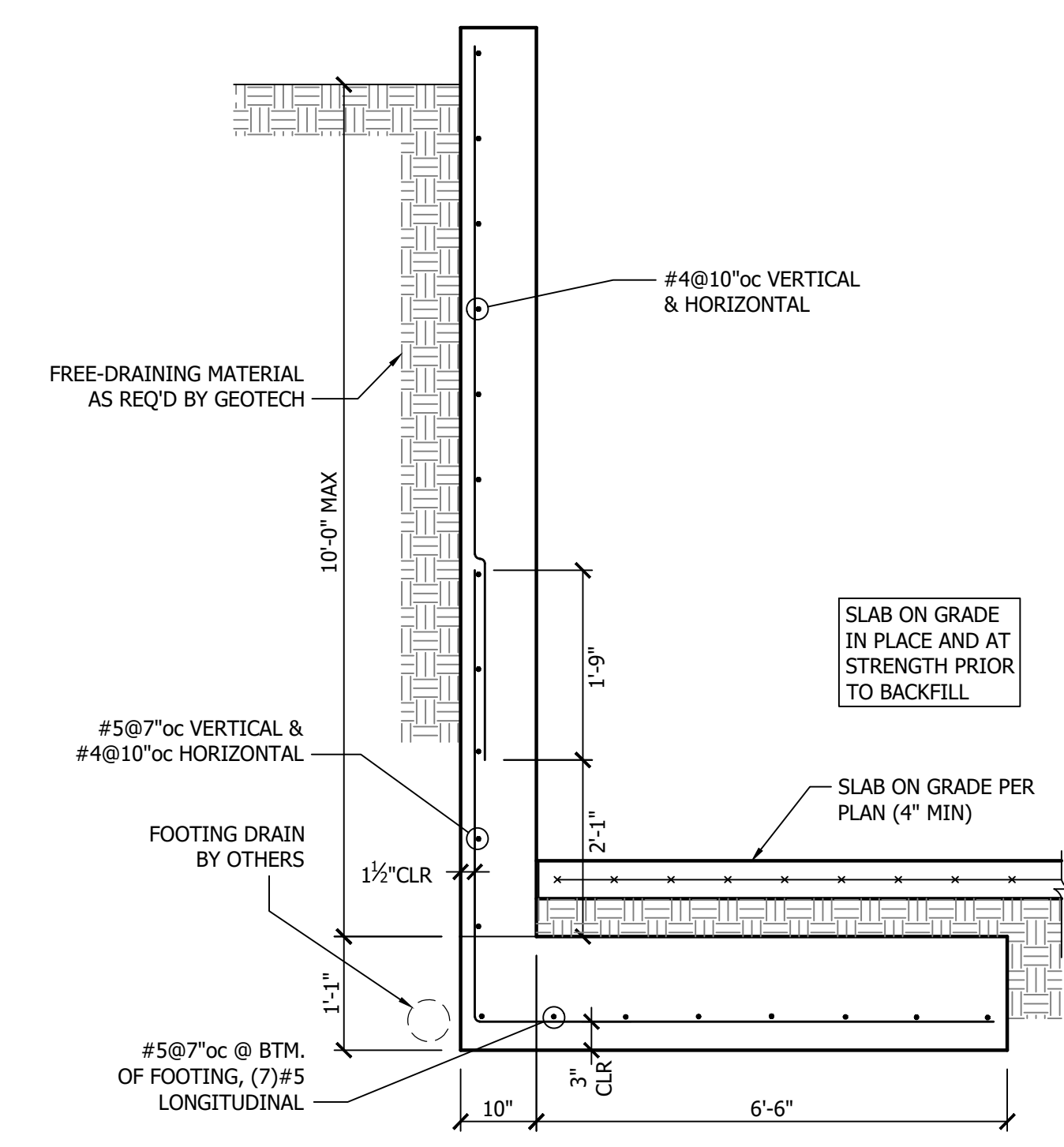
**6** Flush Beam Supporting Exterior Wall Above  
 3/4" = 1'-0"



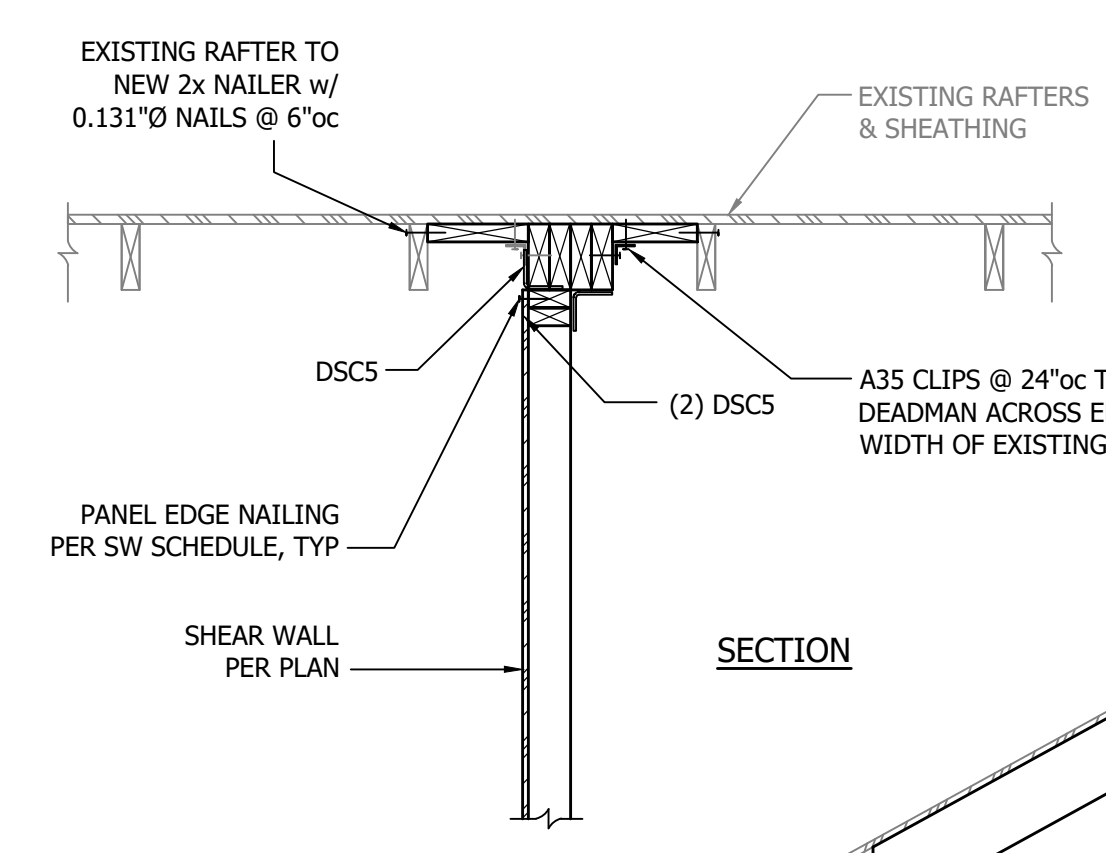
**7** Interior Shear Wall II to I-Joists Below  
 3/4" = 1'-0"



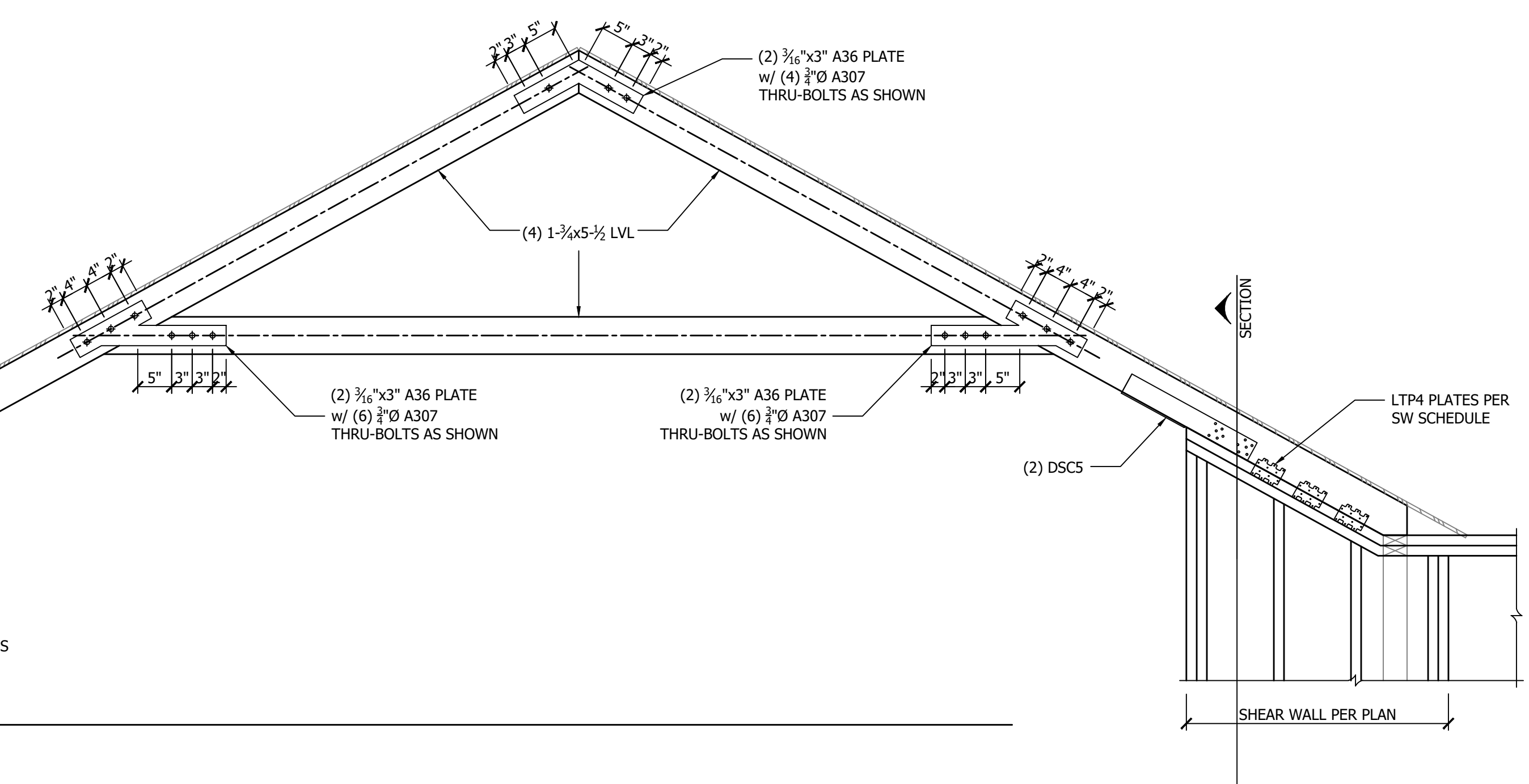
**8** Existing 2x Joist Perp. to Exterior Wall  
 3/4" = 1'-0"



**9** Retaining Wall at Basement  
 3/4" = 1'-0"



**10** Stick-Framed DragTruss  
 3/4" = 1'-0"



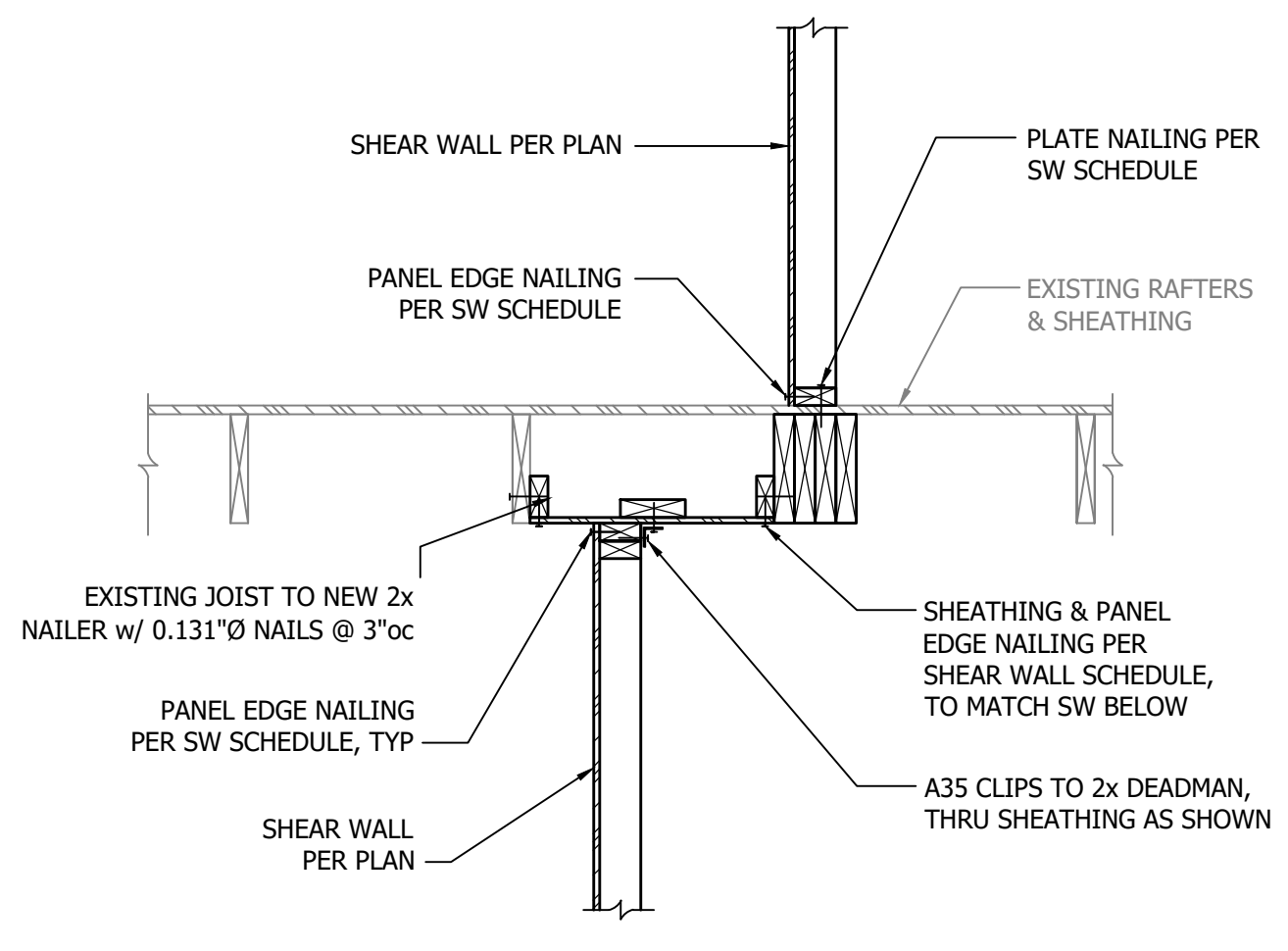
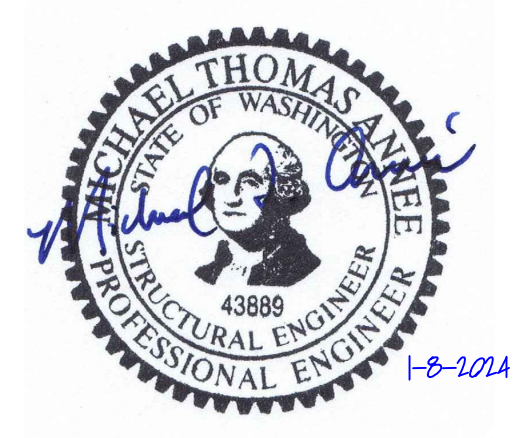
**11** Roof Truss Section  
 3/4" = 1'-0"



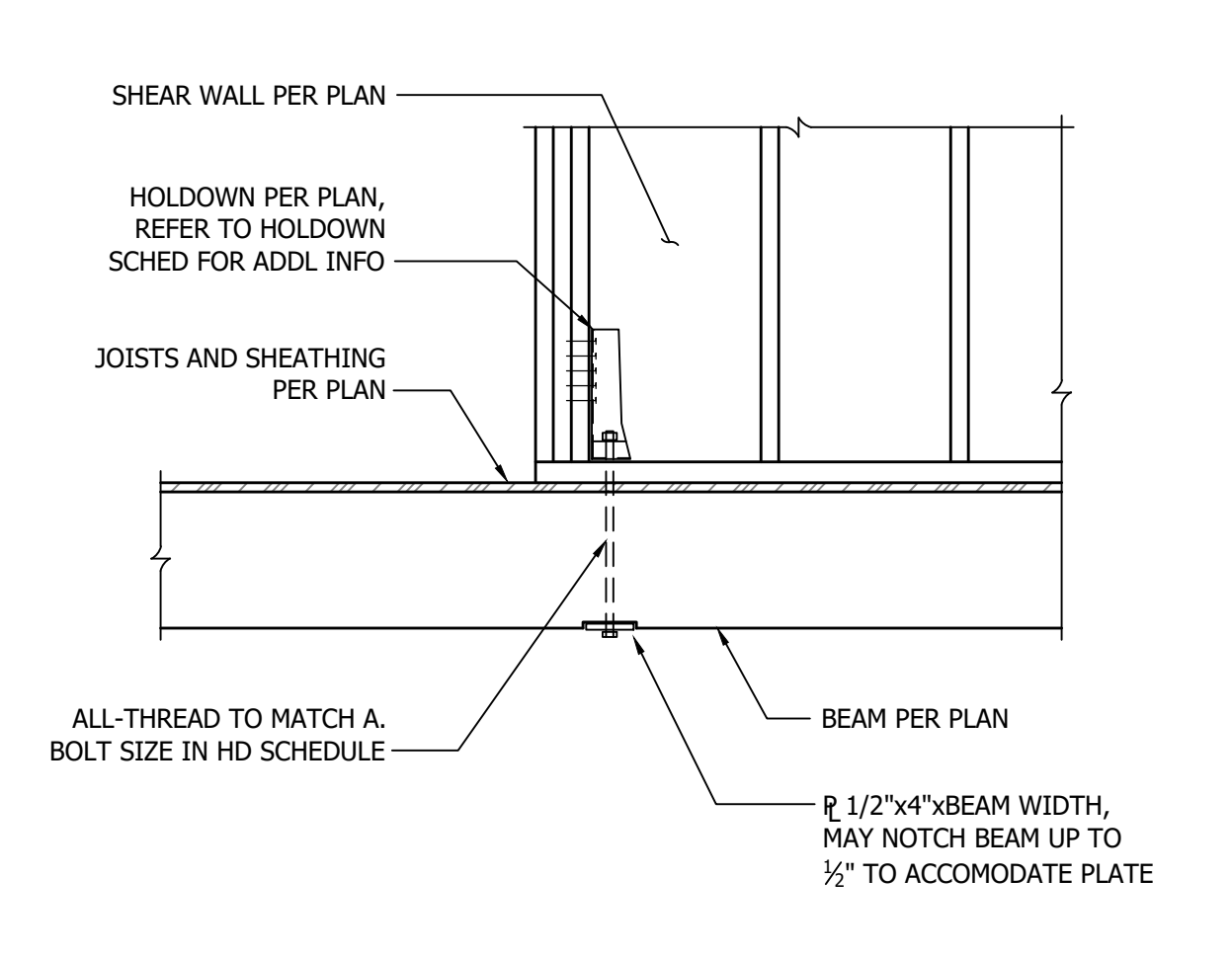


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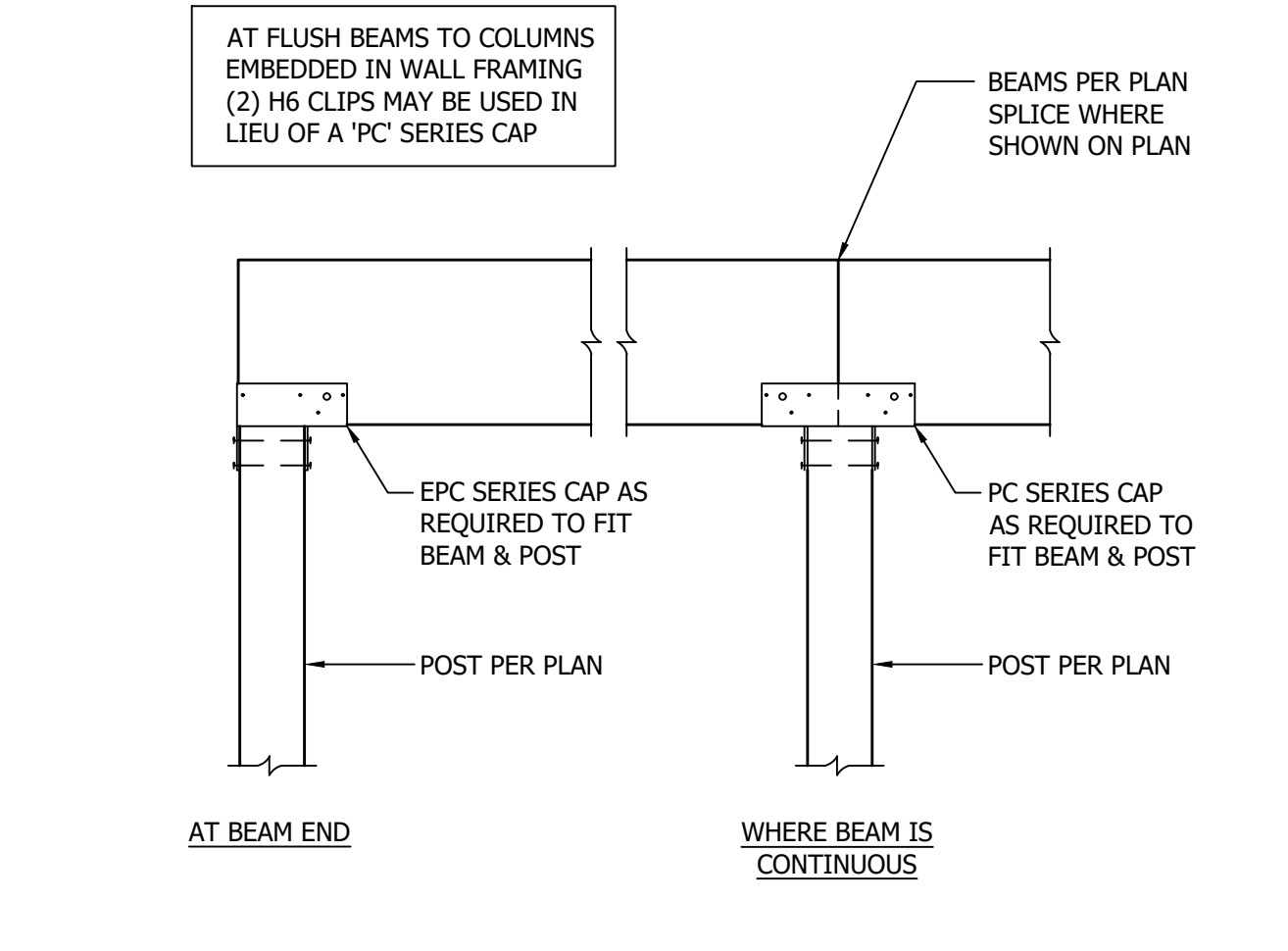
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Seattle, WA 98144  
phone: 206.658.5169  
mike@annestructural.com



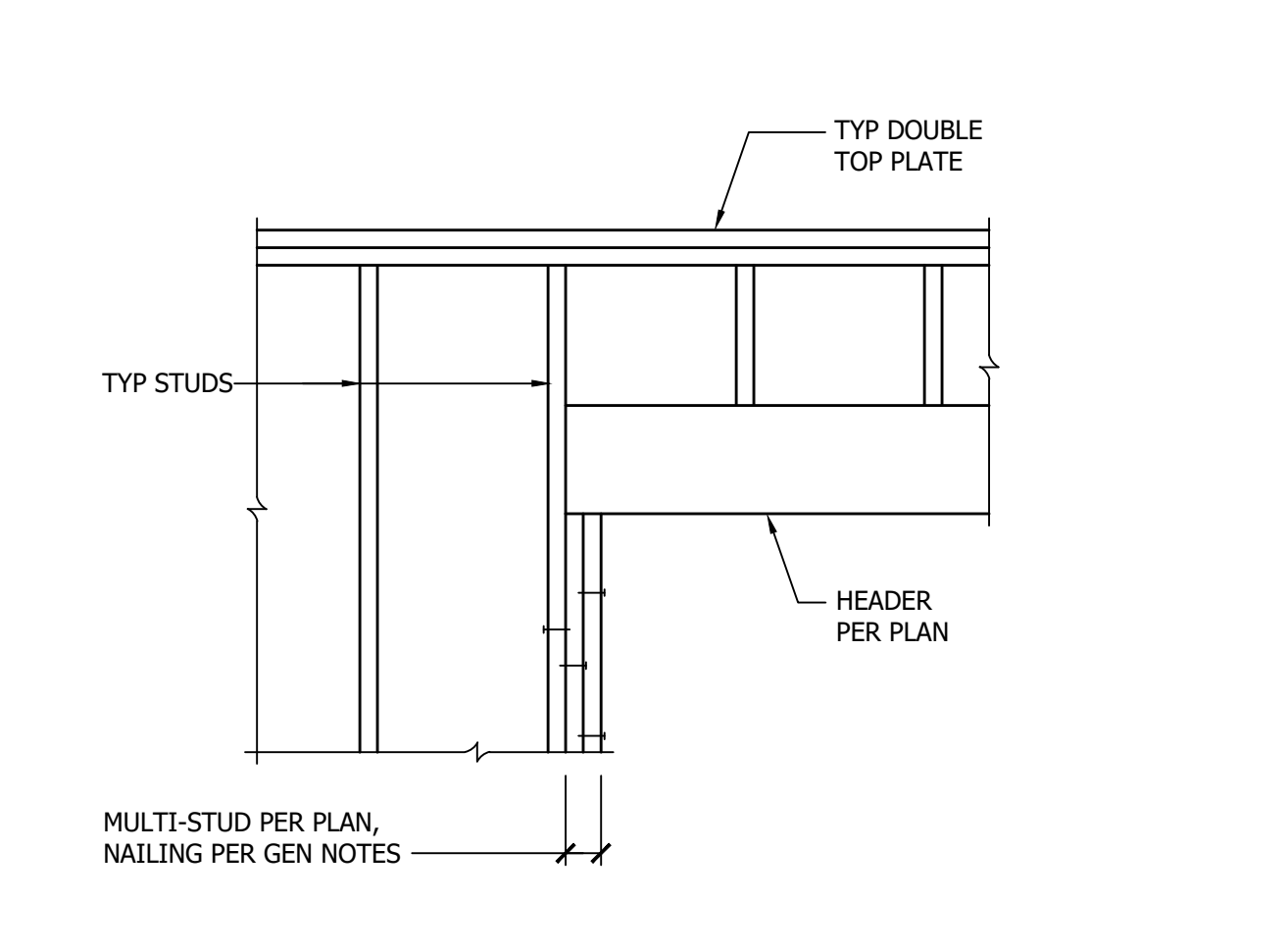
**1** New, Interior Shear Wall II to Existing Rafters  
3/4" = 1'-0"



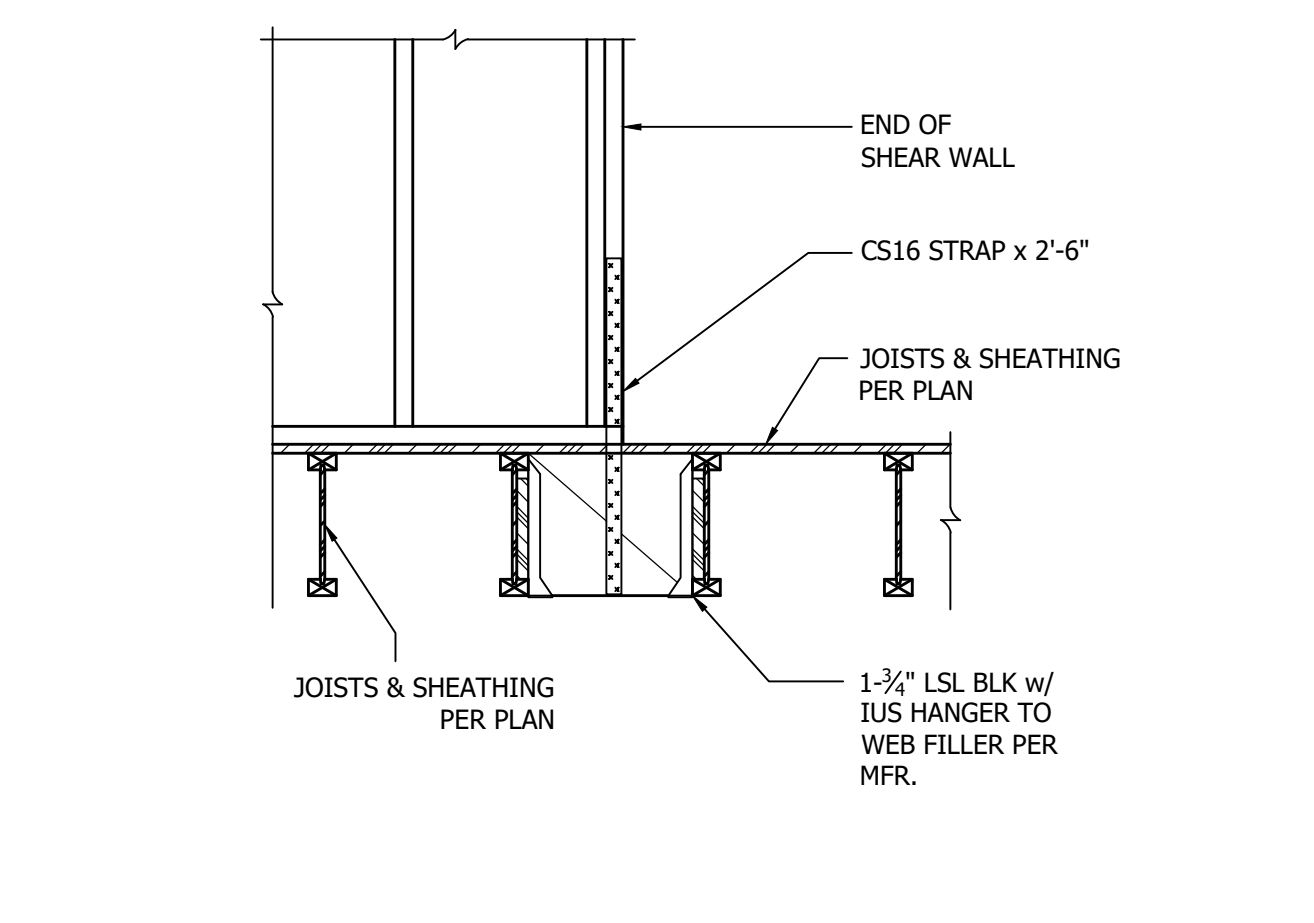
**2** HD-to-Beam  
3/4" = 1'-0"



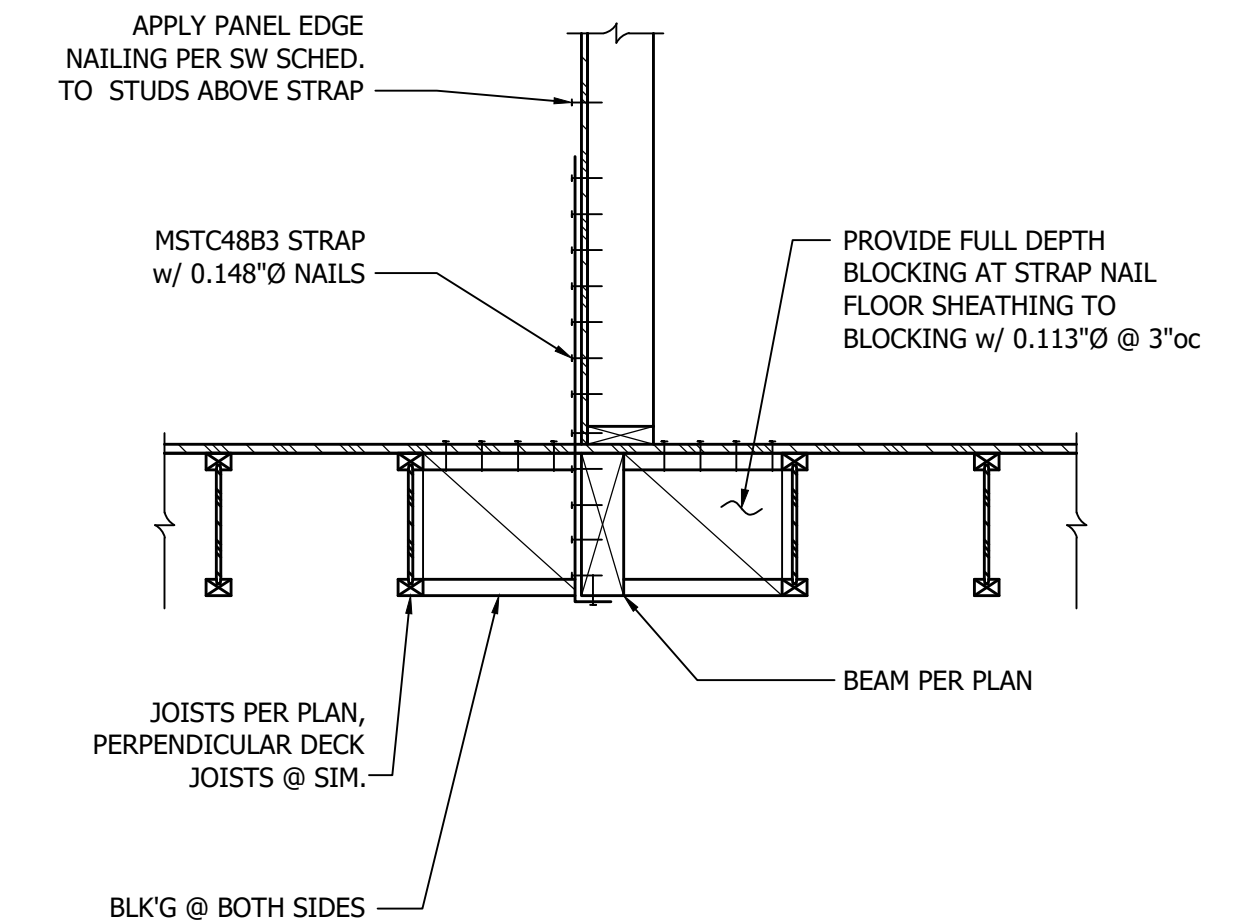
**3** Wood Beam to Wood Column, Typ.  
3/4" = 1'-0"



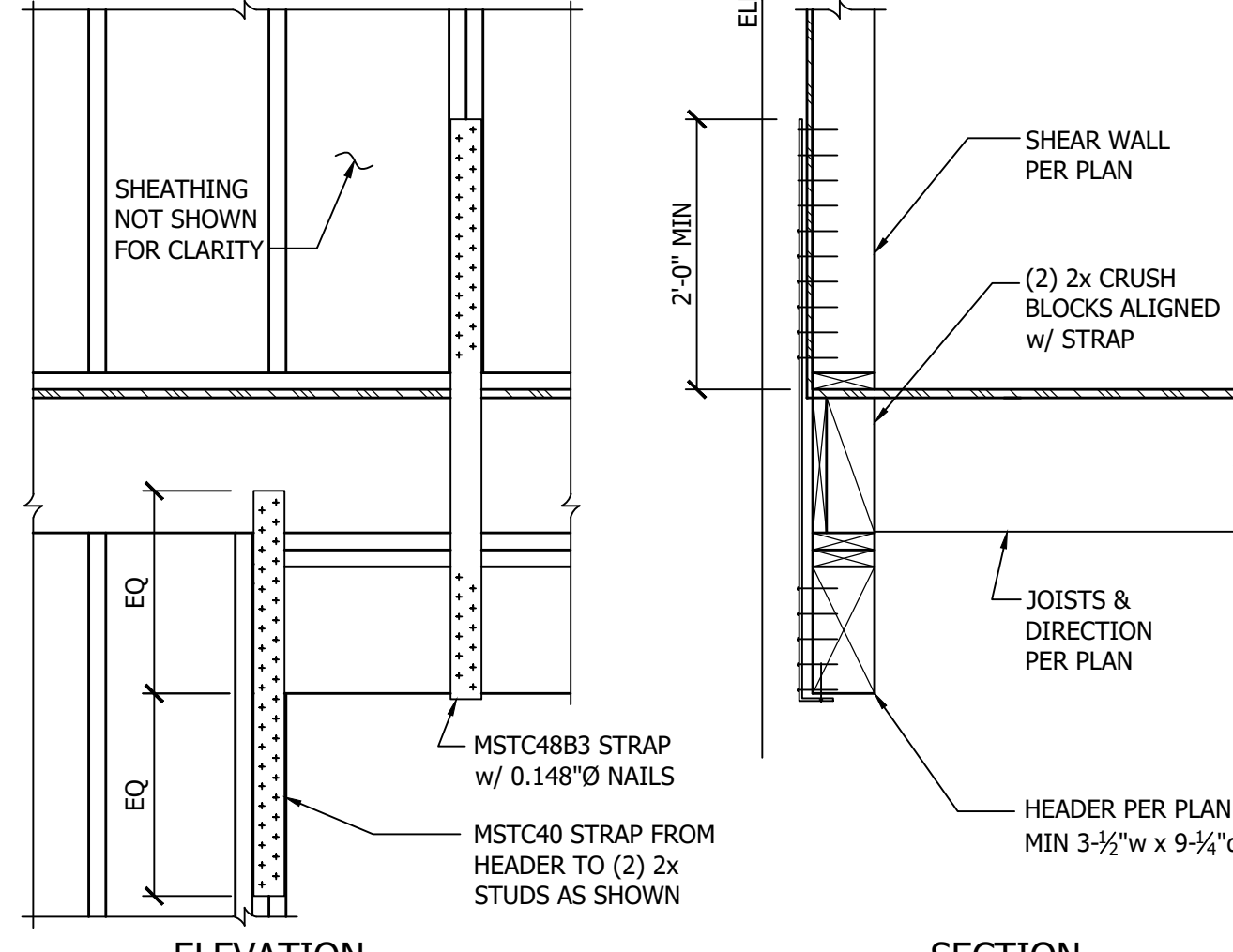
**4** Header Support, Typ.  
3/4" = 1'-0"



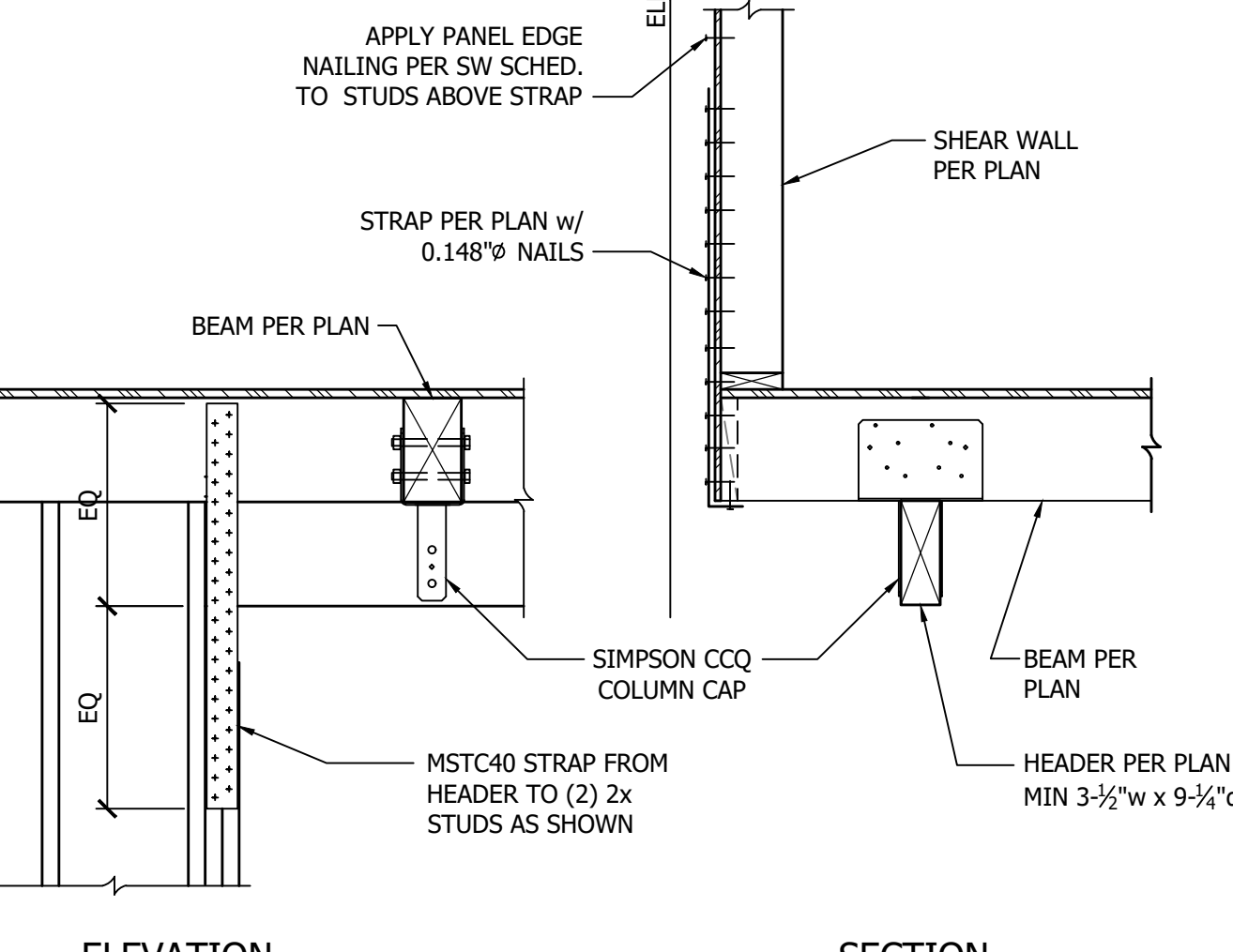
**5** Strap to Blocking Between Joists  
3/4" = 1'-0"



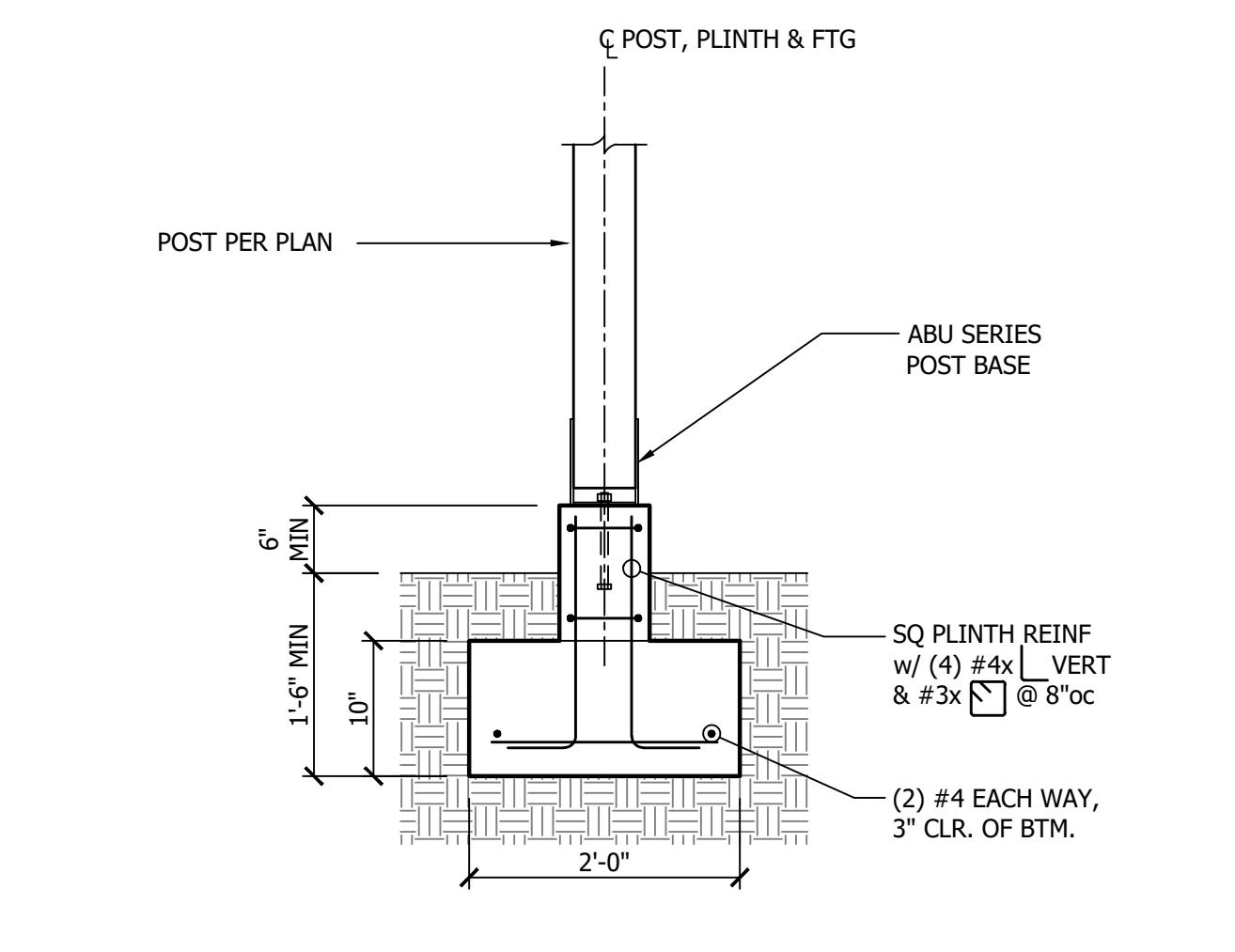
**6** Strap to Beam Below  
3/4" = 1'-0"



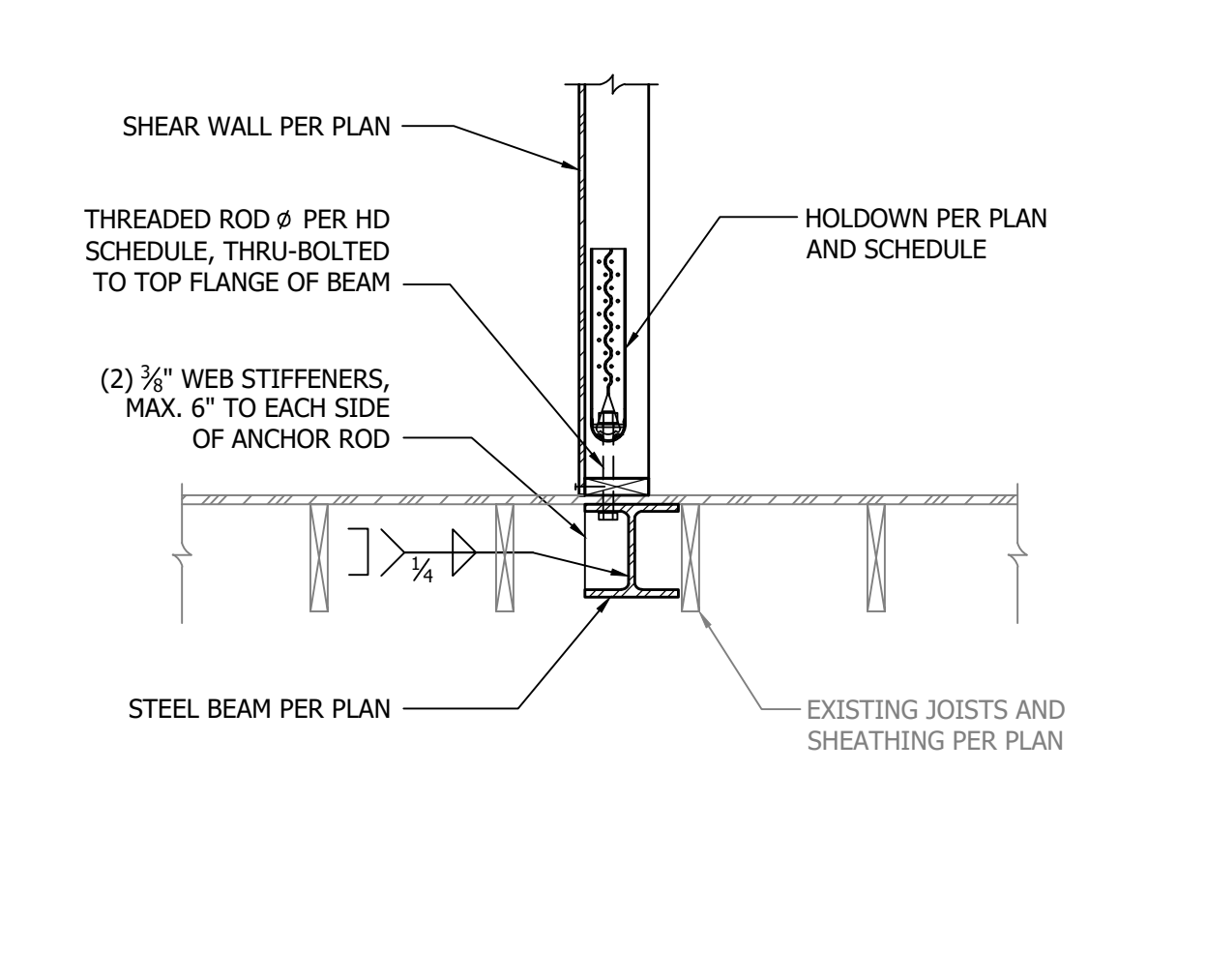
**7** Strap to Header, Typ.  
3/4" = 1'-0"



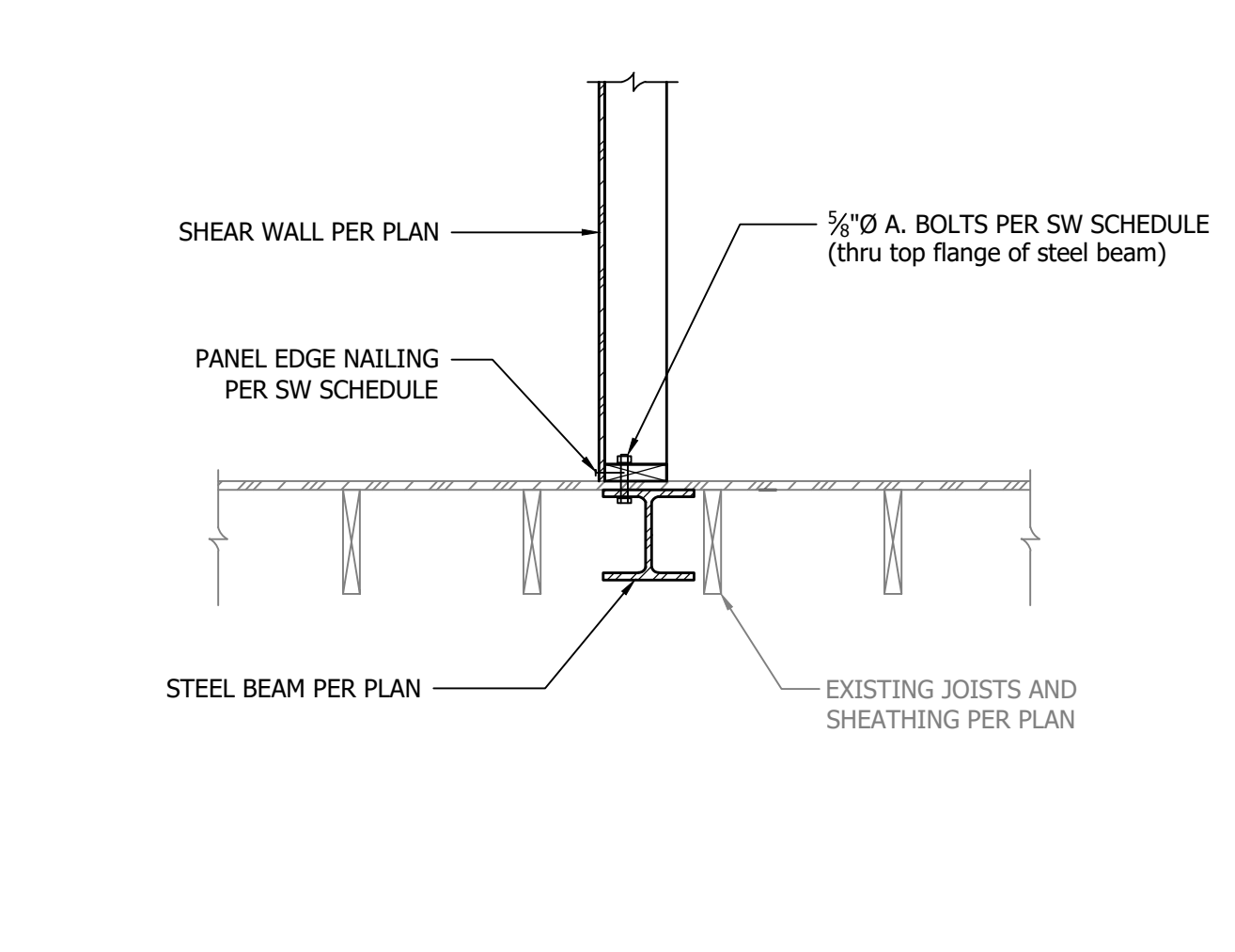
**8** Column Cap at Cantilevered Beam to Header  
3/4" = 1'-0"



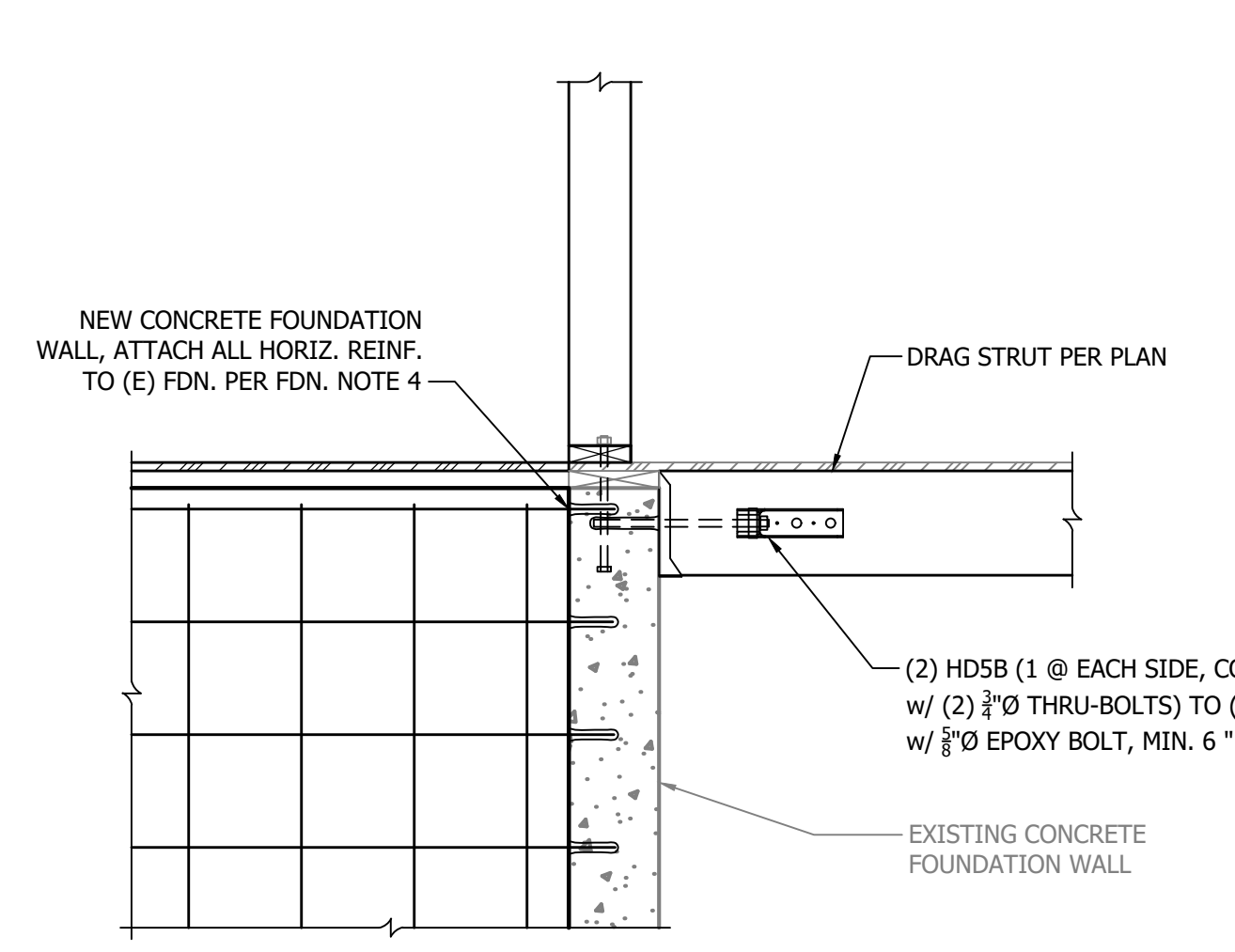
**9** Isolated Post Footing  
3/4" = 1'-0"



**10** Holdown to Steel Beam  
3/4" = 1'-0"



**11** New Steel Beam Under Shear Wall Above  
3/4" = 1'-0"



**12** Drag Strut to Existing Foundation Wall  
3/4" = 1'-0"

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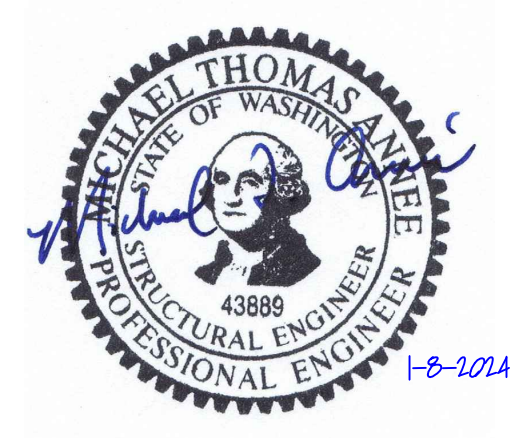
Structural Details

S3.3



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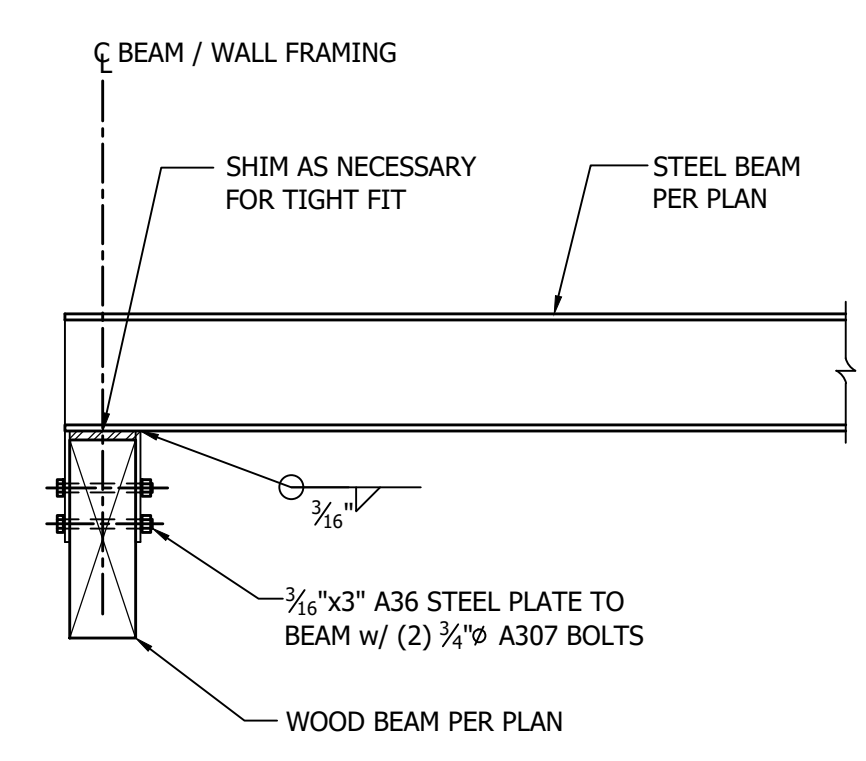
**Rawson Remodel**  
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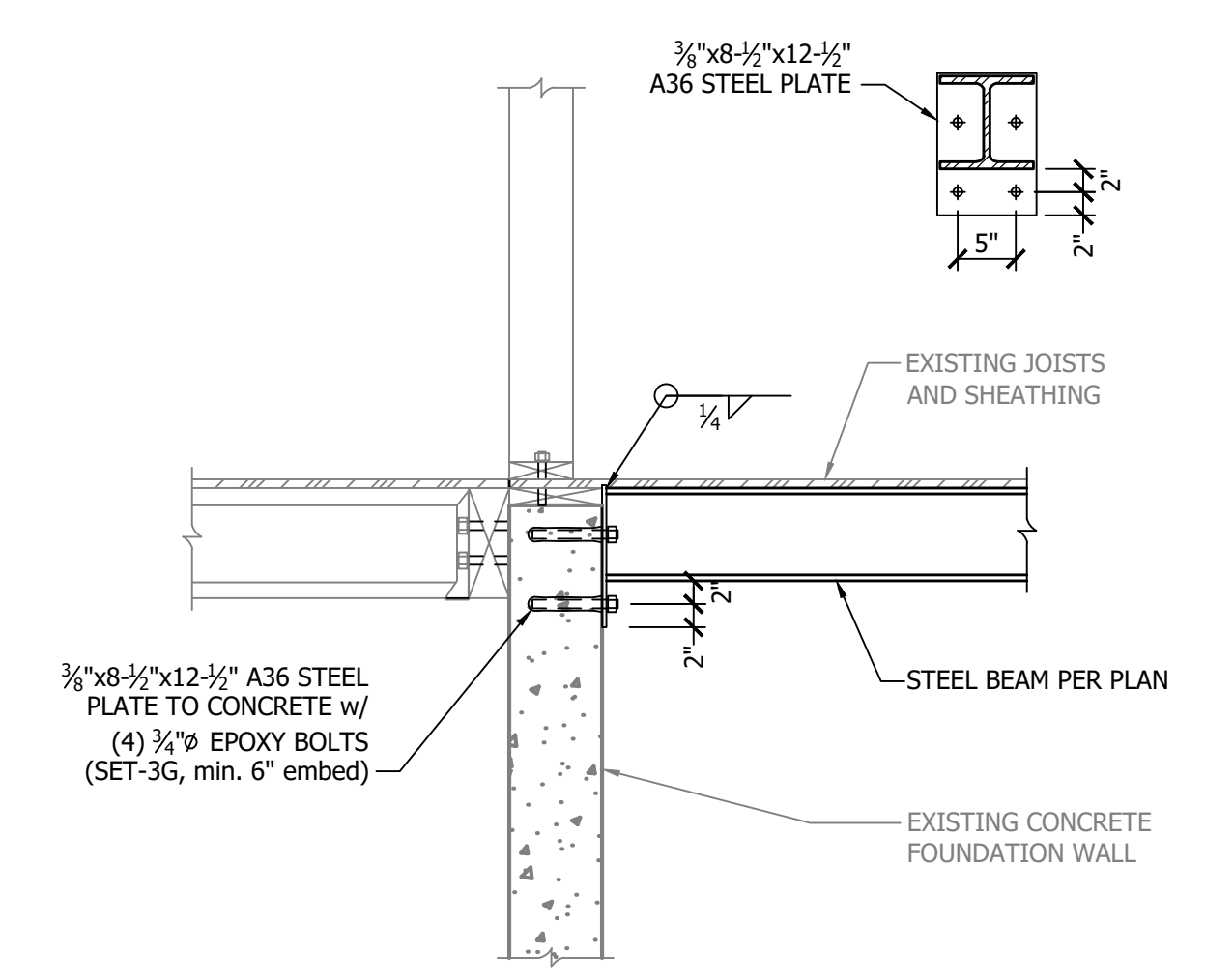
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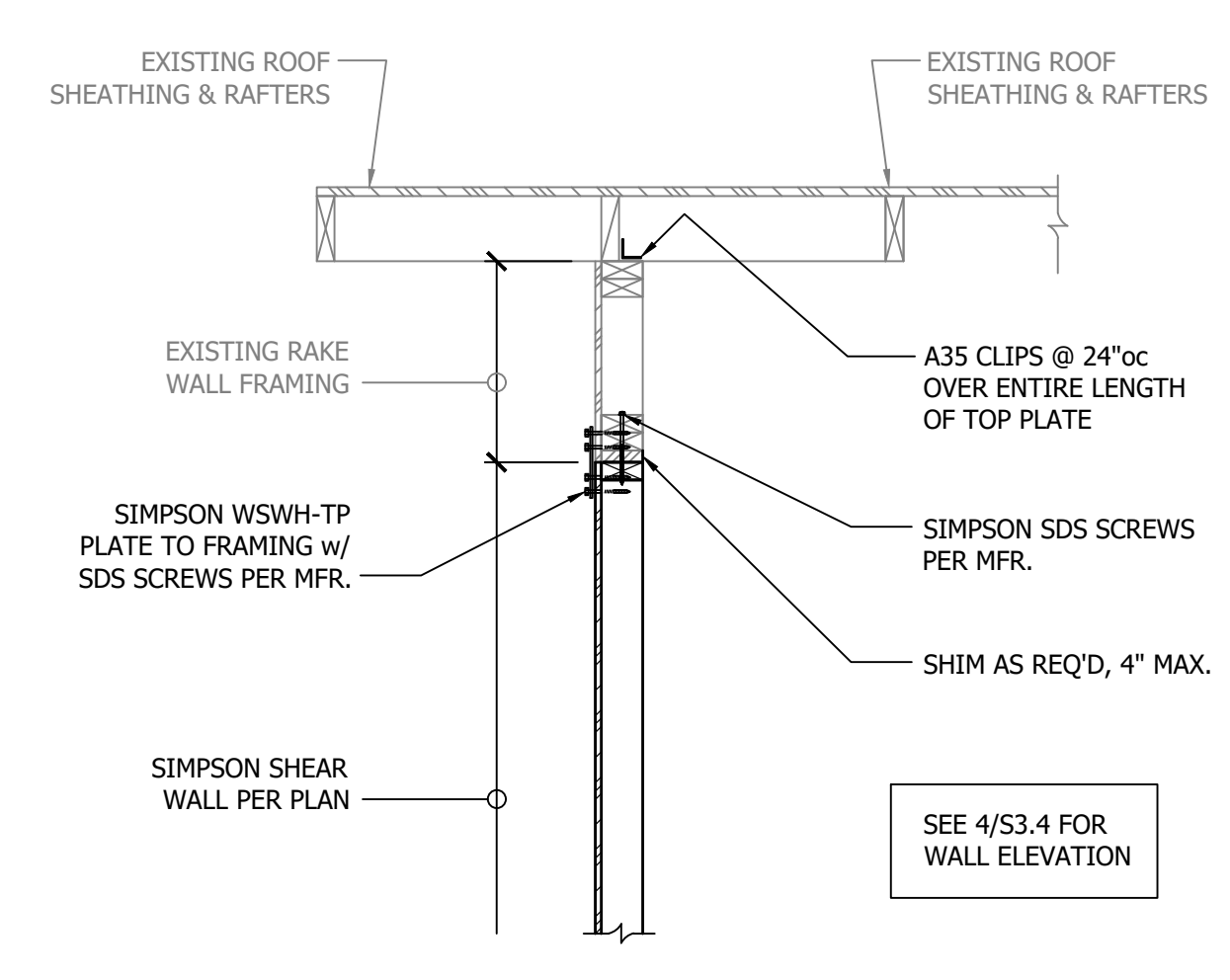
S3.4



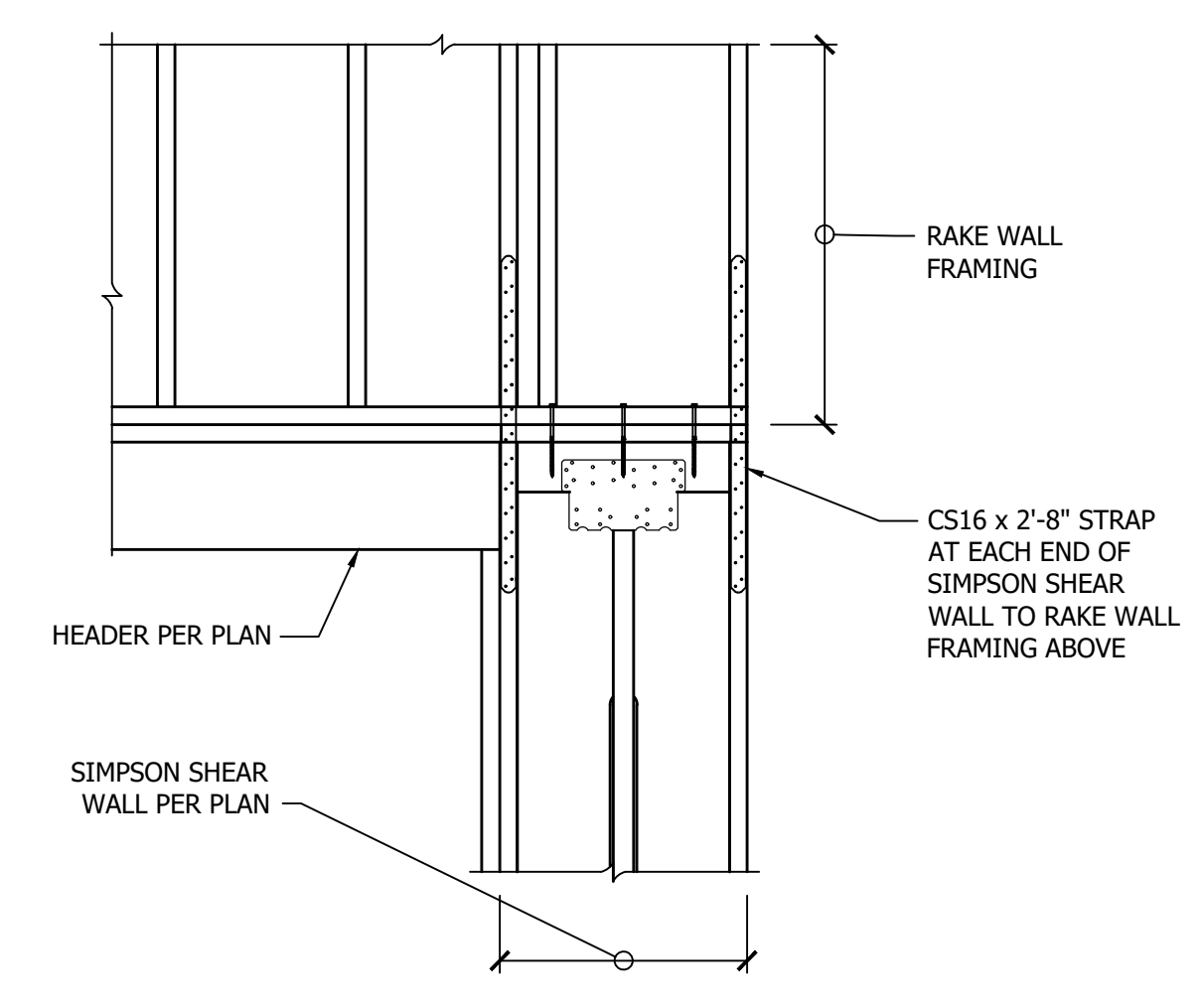
**1 Steel Beam to Wood Column, Typ.**  
3/4" = 1'-0"



**2 I-joists Perp. to Existing Foundation Wall**  
3/4" = 1'-0"



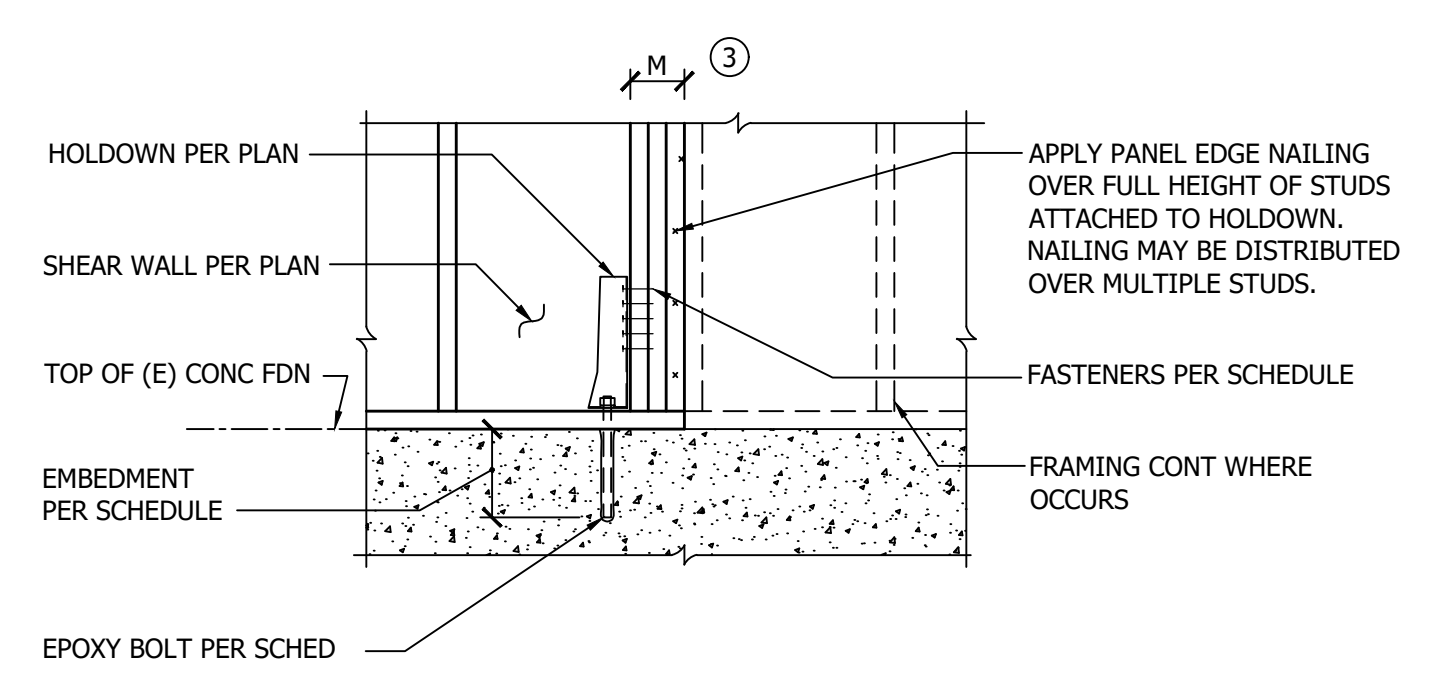
**3 Simpson Shear Wall at Existing Rake Wall**  
3/4" = 1'-0"



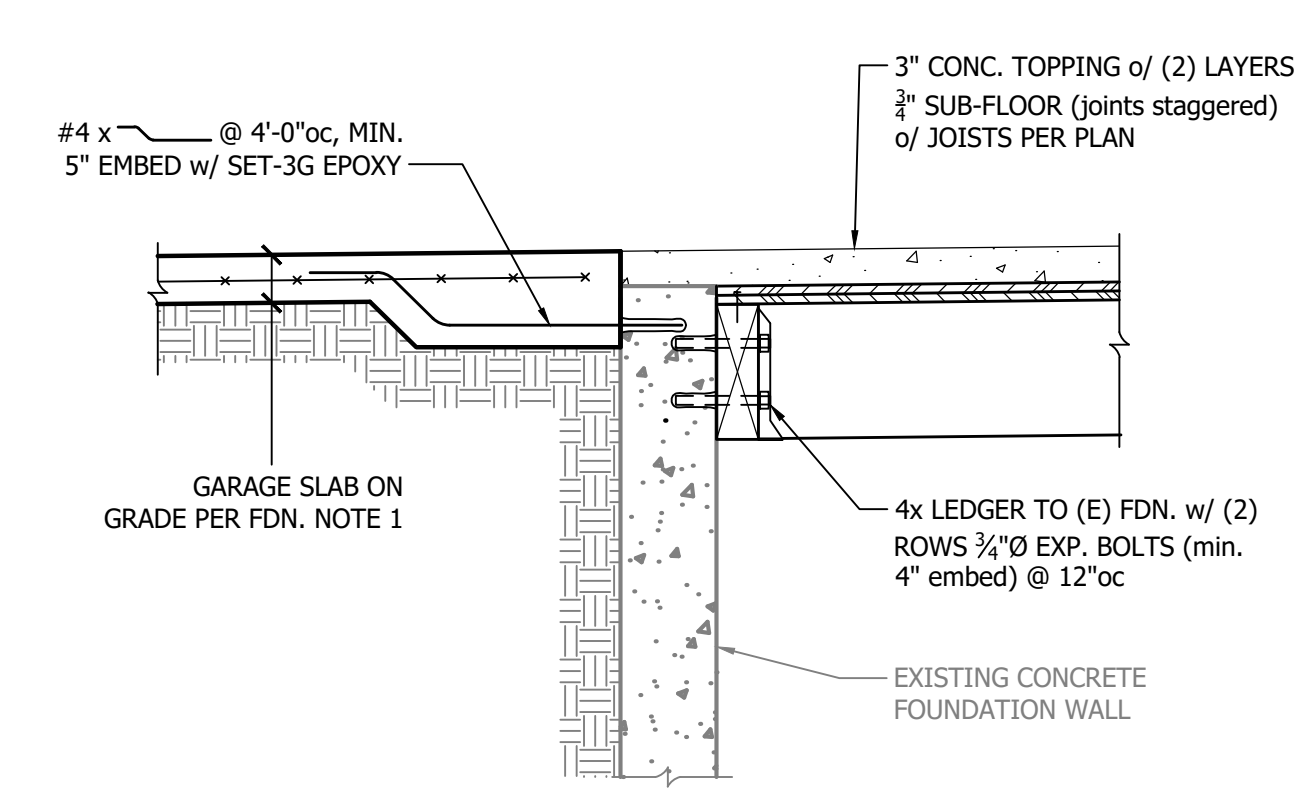
**4 Elevation at Rake Wall to Simpson Shear Wall**  
3/4" = 1'-0"

HOLDDOWN SCHEDULE ① ② ④						
MARK	FASTENERS	M ③	WALL			CAPACITY
			ANCHOR ROD	EMBEDMENT	EDGE DISTANCE	
HDU8	(20) SDS $\frac{1}{2}$ "x2 $\frac{1}{2}$ "	4x6	$\frac{7}{8}$ " $\varnothing$	16"	3"	7,870#
HDU14	(36) SDS $\frac{1}{2}$ "x2 $\frac{1}{2}$ "	4x8	1" $\varnothing$	20"	3"	12,375#

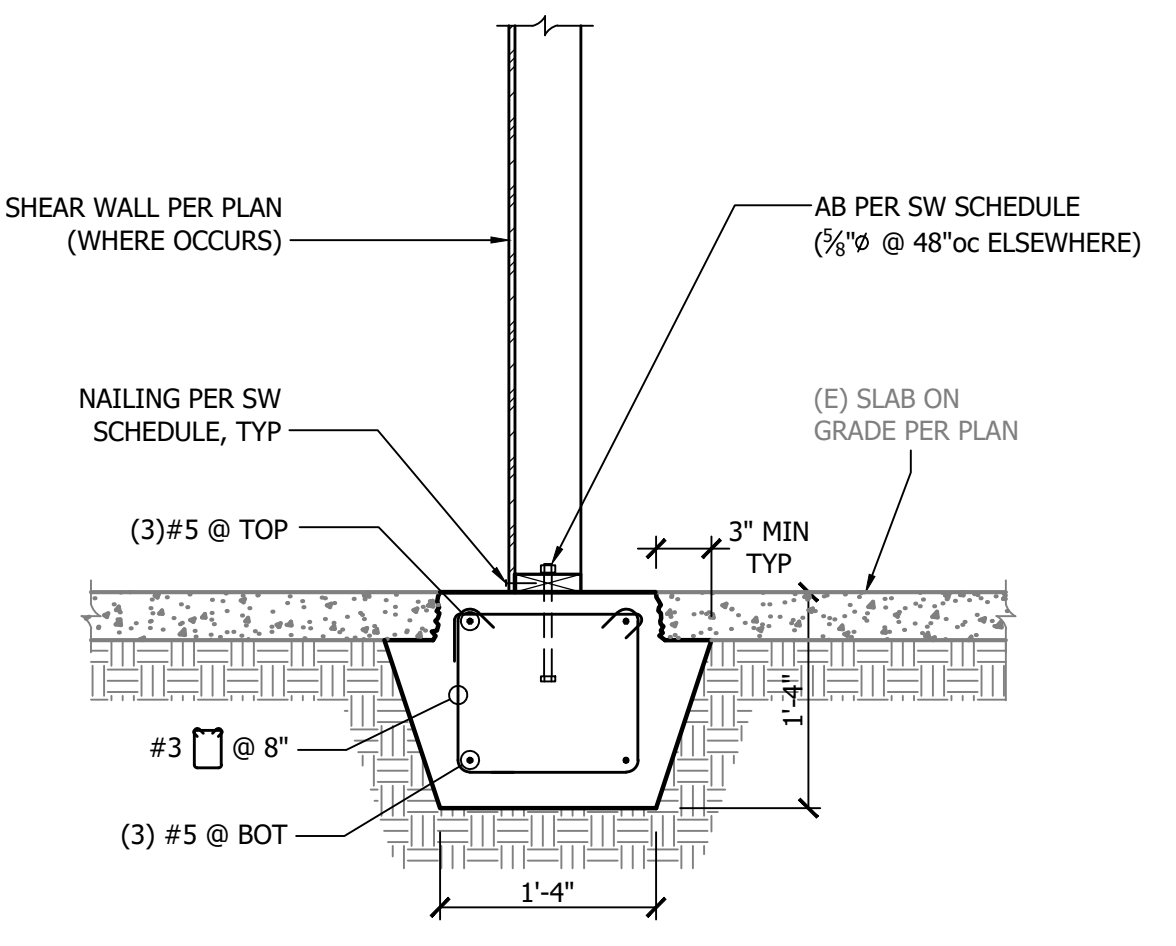
- ① PLACEMENT OF ANCHOR ROD IS BASED ON SIMPSON SET-XP EPOXY.
- ② INSTALL ALL HOLDDOWNS AND EPOXY PER MANUFACTURER'S INSTRUCTIONS.
- ③ DEPTH OF WOOD FRAMING MEMBER ATTACHED TO HOLDDOWN. MEMBERS SHALL BE HEM-FIR UNLESS NOTED OTHERWISE NOTED.
- ④ MIN 6" CONCRETE WALL THICKNESS REQUIRED.



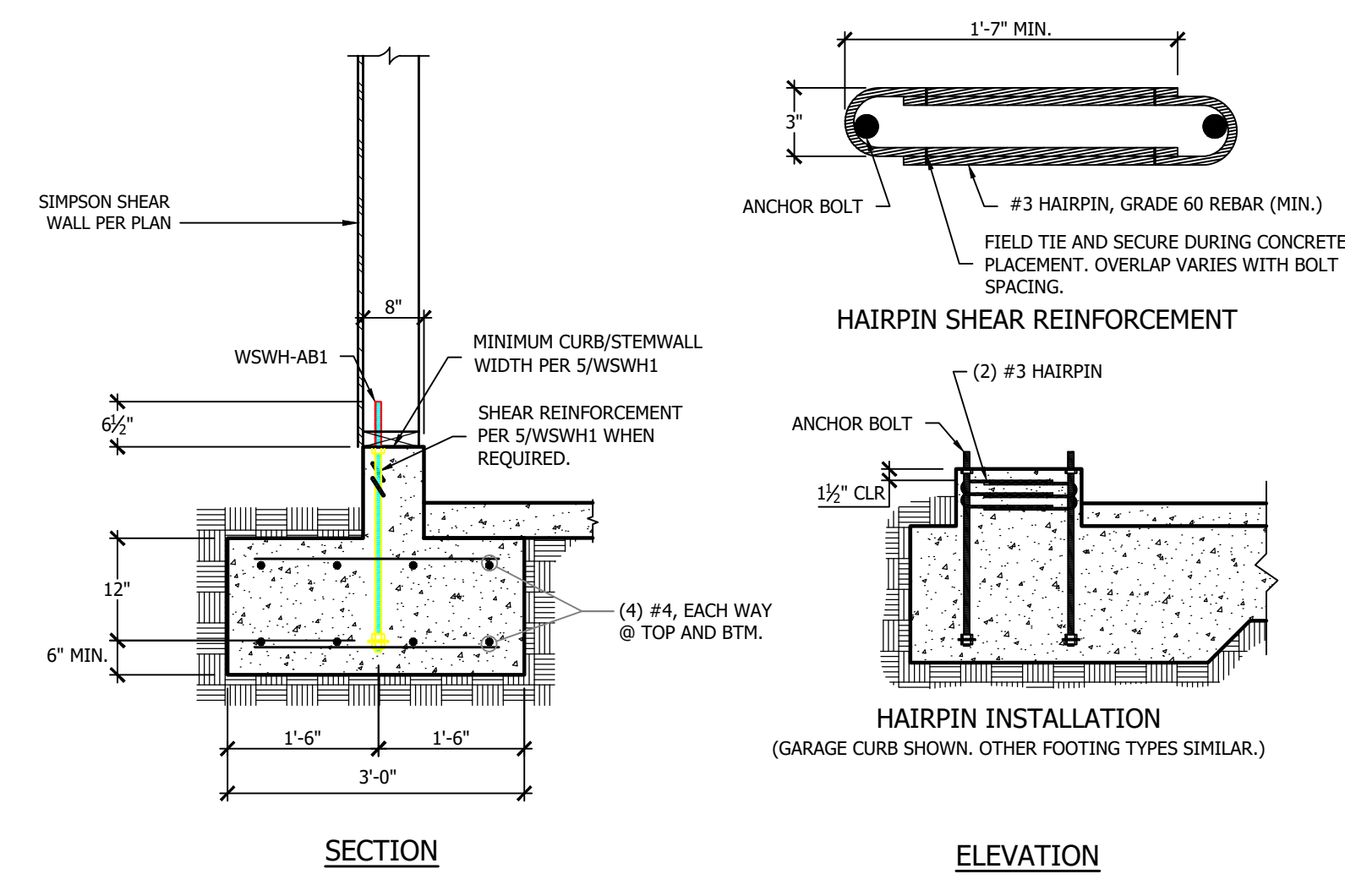
**5 Retrofit Holddown Schedule**  
3/4" = 1'-0"



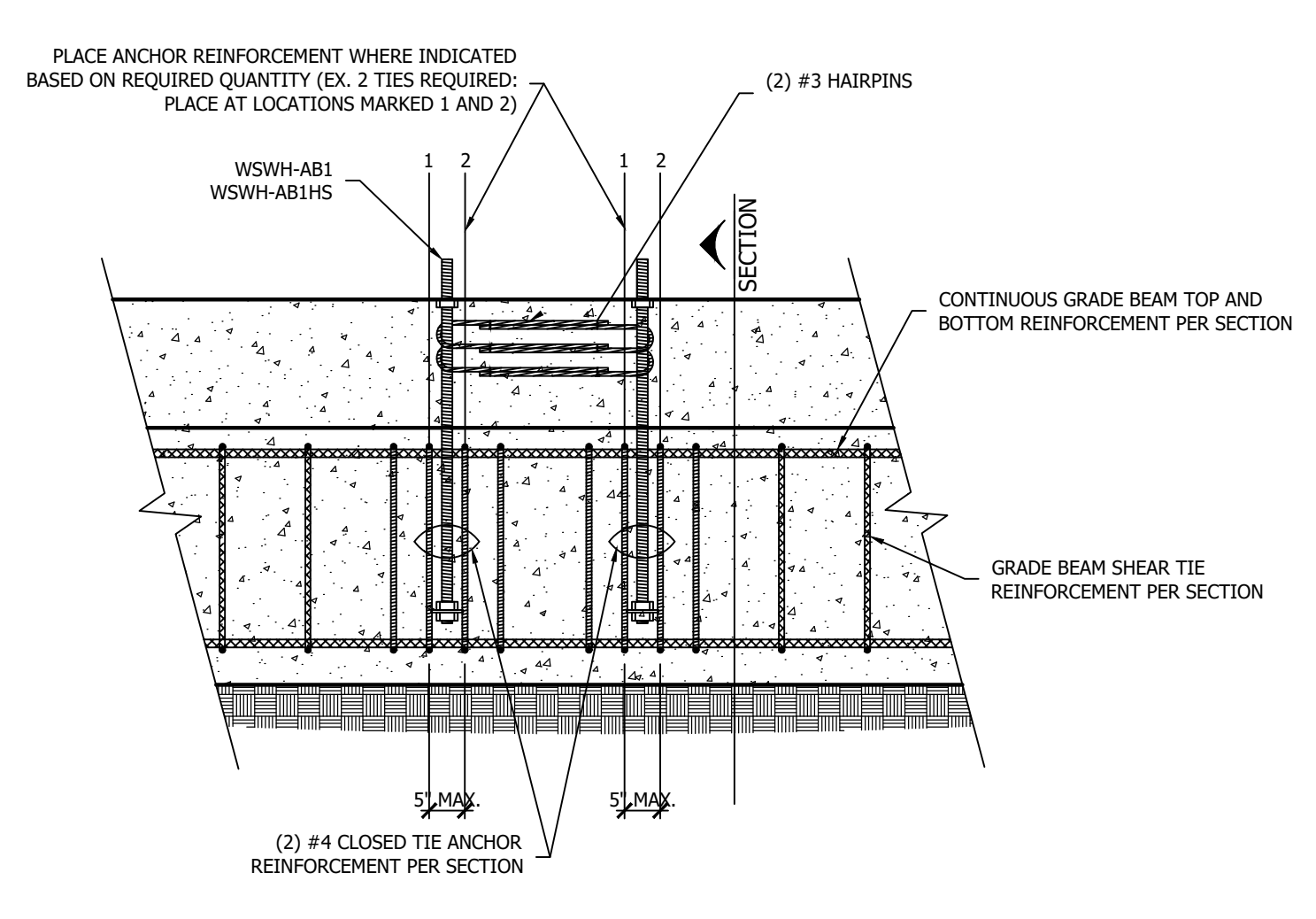
**7 Framed-to-SOG Garage Floor Transition**  
3/4" = 1'-0"



**8 Cont. Grade Beam at Existing Slab on Grade**  
3/4" = 1'-0"



**9 Simpson Shear Wall Anchorage and Reinforcement at Garage Ftg.**  
3/4" = 1'-0"



**11 Simpson Shear Wall Anchorage and Reinforcement at Grade Beam**  
3/4" = 1'-0"